



# EMPIRE BAY CATCHMENT FLOOD STUDY VOLUME 1 - REPORT



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## FOREWORD

The NSW Government's Flood Prone Lands Policy is directed towards providing solutions to existing flood problems in developed areas utilising ecologically positive methods wherever possible and ensuring that new development is compatible with the flood hazard and does not create additional flooding problems in other areas.

Under the policy, the management of flood prone land is the responsibility of Local Government. To achieve its primary objective, the policy provides for State Government financial assistance to Councils for flood mitigation works to alleviate existing flooding problems. The policy also provides for State Government technical assistance to Councils to ensure that the management of flood prone land is consistent with the flood hazard and that future development does not create or increase flooding problems in flood prone areas.

The Policy provides for technical and financial support by the State Government through the following sequential stages:

- |                                     |                                                                                                                                                                                         |
|-------------------------------------|-----------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| 1. Flood Study                      | Determines the nature and extent of the flood problem.                                                                                                                                  |
| 2. Floodplain Risk Management Study | Evaluates management options for the floodplain in respect of both existing and proposed development.                                                                                   |
| 3. Floodplain Risk Management Plan  | Involves formal adoption by Council of a plan of management for the floodplain.                                                                                                         |
| 4. Implementation of the Plan       | Construction of flood mitigation works to protect existing development.<br><br>Use of Environmental Planning Instruments to ensure new development is compatible with the flood hazard. |

The Empire Bay Catchment Flood Study is the first stage of the management process for the Empire Bay Catchment. The study, which has been prepared for Gosford City Council by Cardno Lawson Treloar Pty Ltd, defines flood behaviour for existing catchment conditions in the Empire Bay catchment floodplain.

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## EXECUTIVE SUMMARY

This Flood Study has been undertaken to define the nature and extent of flooding due to local rainfall only within the Empire Bay catchment. It lies wholly within the Gosford City Council (GCC) Local Government Area on the south-eastern side of Brisbane Water. It is located to the east of Woy Woy on the southern side of Cockle Channel and Cockle Bay.

The study area occupies an area about 554 ha covering the suburb of Empire Bay to the south-western section of Bensville. It is bounded by a ridgeline along the southern and western sides extending north to Cockle Channel and Cockle Bay. The Empire Bay residential area is relatively flat with an elevation down to about RL 1.0m AHD at the foreshore and the area around Cockle Bay Nature Reserve is also relatively flat.

Land-use in the catchment is primarily residential with significant areas of bushland / vegetated areas. The density of residential areas varies from low-density detached houses in the main part of Empire Bay and within Bensville, to larger bushland residential lots between these two areas. Several shops are located within the two main residential areas. Large areas of bushland are located on the higher elevations in the southern part of catchment and also along areas adjoining the estuary including Cockle Bay Nature Reserve.

Pit and piped drainage infrastructure convey stormwater runoff through the main residential areas of Empire Bay and Bensville to the foreshore area. Several drainage depressions and natural channels convey runoff from the bushland areas to piped systems crossings Empire Bay Drive.

A questionnaire was delivered to each residence in the study area to gauge resident's awareness of flooding in the catchment and to identify specific accounts of flood inundation. Fifty percent of respondents were aware of flooding in the catchment with some experiencing inundation in their house. A high percentage of respondents recalled details of inundation during the June 2007 event and less information was noted for flood events prior to this date. The draft report was placed on public exhibition from 18th September to 16th October 2009 inviting submissions for the Study.

Hydrologic and hydraulic modelling was completed to assess flood behaviour within the catchment. The SOBEK 1D/2D model from WL|Delft Hydraulics Laboratory was used to model the catchment and to hydraulically route overland flood flows and street flow. An area of about 887 ha was modelled which includes parts of the estuary. To facilitate the modelling, the catchment has been divided into three sectors comprising the Empire Bay residential area, Bensville residential area, and the bushland residential section which covers the largest area. The SOBEK modelling of the Empire Bay catchment utilises the rainfall-on-grid methodology for developing the hydrology. In the model, rainfall is applied directly to the 2D terrain, and the hydraulic model automatically routes the flow.

Data for the model set-up was collated from various sources including Gosford City Council, Johnson Partners surveyors, Bureau of Meteorology and Manly Hydraulics Laboratory. This data included aerial photos, aerial laser scanning (ALS), field survey of piped drainage systems, historical rainfall from previous storm events and historical water levels in the Brisbane Water estuary.

A terrain grid representing the topography of catchment was generated from the ALS and input to the SOBEK model. Also input to the model was rainfall data, soil loss-rates, drainage pipes and culverts, and parameters for hydraulic roughness to account for the varying land-uses. A 1% probability of exceedance estuary level was adopted from the Brisbane Water Foreshore Flood Study as the boundary condition at the foreshore areas. This analysis thus determines flood behaviour due to runoff from the local catchment and the Brisbane Water Foreshore Flood Study (2009) assesses flood impacts due to raised storm event levels in the estuary. The model was calibrated to flood levels and responses from the resident questionnaire for the June 2007 and April 1988 events.

Flood behaviour was modelled in SOBEK for a series of Annual Exceedance Probabilities (AEP). The events modelled were 0.5% AEP, 1%, 2%, 5%, 10%, 20%, 50%, and 100% AEP and Probable Maximum Flood (PMF). The sensitivity of the model was tested to demonstrate the range of uncertainty in the model results for changes in key parameters. Variations to the rainfall parameters, hydraulic roughness, downstream boundary, pipe blockage, and land-use were assessed.

In a 1% AEP event, the modelling shows that some properties and roads may be inundated up to 0.5m. This occurs notably around Gordon Road, Boongala Avenue, Rickard Road, and Greenfield Road in the main residential area of Empire Bay and around the main drainage channels at Pomona Road and Empire Bay Drive to Palmers Lane in the rural-residential area. In the 1% AEP event, the modelling showed that 22 houses are flooded above the floor level when the storm runoff is combined with a 1% probability of exceedance level in the estuary.

Provisional flood hazard (low and high hazard) and hydraulic categorisation (floodway, flood storage, and flood fringe) were also assessed for the flows within the catchment. In the 1% AEP event, no roadways are indicated as provisional high hazard though some properties have high hazard channels / creeklines in the vicinity.

Economic impacts of flooding were evaluated by completing a preliminary flood damage assessment. Costs were estimated for damages resulting to buildings due to local catchment runoff for the various storm events modelled. An average annual damage estimated for the modelled floodplain is about \$548,125.

Climate change is expected to result in increased sea levels and increased rainfall intensities. Potential impacts to flood behaviour in the Empire Bay catchment due to climate change have been analysed for:

- 10%, 20%, and 30% increase to rainfall intensity, and
- Estuary level rises of 0.2m and 0.91m.

Flood inundation in the low elevation areas of the catchment are particularly affected by increases in the sea level which influences the levels in the Brisbane Water estuary. Flooding in the higher elevation areas to the south is more influenced by the increases in rainfall intensities.

## 1. INTRODUCTION

The Empire Bay catchment lies within the Gosford City Council (GCC) Local Government Area on the south-eastern side of Brisbane Water. It is located to the east of Woy Woy on the southern side of Cockle Channel and Cockle Bay. The catchment is subject to flood inundation and GCC aims to undertake floodplain management in accordance with the Floodplain Management Process as set out in the New South Wales Government *Floodplain Development Manual* (2005).

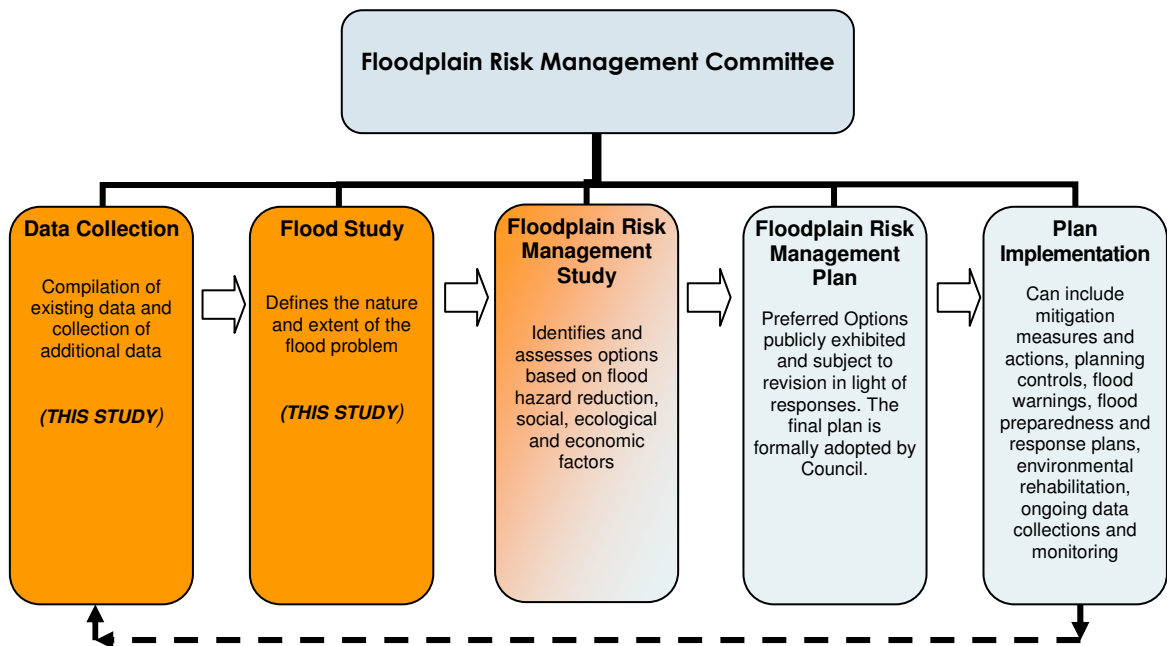
Cardno Lawson Treloar (CLT) was commissioned by GCC to undertake a flood study as part of the floodplain management process. This flood study has been undertaken to determine the flood behaviour in the catchment due to local storm runoff for the 0.5% Annual Exceedance Probability (AEP), 1% AEP, 2% AEP, 5% AEP, 10% AEP, 20% AEP, 50% AEP and 100% AEP flood events and the Probable Maximum Flood (PMF). In accordance with its objectives, the study has determined the nature and extent of flooding through the estimation of design flood flows, levels and velocities. Flood impacts due to storm events in the Brisbane Water estuary are detailed in the Brisbane Water Foreshore Flood Study (2009).

In undertaking the flood study, a hydrologic-hydraulic computer model of the major channels and floodplain within the catchment was established and verified against historical flood event observations. The hydraulic model was then used with design rainfall conditions to simulate design flood behaviour in the catchment. The study has defined Provisional Flood Hazard and Hydraulic Categories for the flood affected areas.

## 2. STUDY OBJECTIVES

### 2.1 Regulatory Context

The NSW Government Floodplain Development Manual (2005) sets out a process for floodplain risk management. A flowchart representation of this process is shown in Figure 2.1, which is adapted from the Floodplain Development Manual (2005).



**Figure 2.1 Floodplain Management Process**

The tasks being undertaken in this Flood Study Report include the compilation of data and definition of the flood behaviour and extent. Assessment of flood management and mitigation options would be undertaken in the next stage of the risk management process as part of the Floodplain Risk Management Study.

### 2.2 Objectives

The objective of this Study is to define the nature of the existing flood behaviour due to local runoff in the Empire Bay catchment.

To achieve the objectives, the following tasks were undertaken:

- Collate available flood-related data,
- Define existing catchment condition flood behaviour for mainstream flooding in the catchment,
- Define design flood levels, velocities and flow distributions for the catchment,
- Define the extent of flooding the nominated AEP events and PMF for the catchment,
- Define the hydraulic categories for the flood-affected areas,
- Define provisional flood hazard for the flood-affected areas,
- Assess flood damages for the flood-affected areas.

## 2.3 Methodology

This Study was carried out using computer-based hydrologic-hydraulic modelling. The SOBEK 1D/2D program is a purpose-built flood model developed by WL|Delft Hydraulics. In this model, rainfall is applied directly to the elevation grid and flow is routed according to the topography and hydraulic controls of the catchment. Stormwater drainage pits, pipes and channels are represented in the model as one-dimensional elements which are dynamically linked to the water conveyed across the elevation grid.

The study details are grouped together under the following sections of the report:

- Section 3 provides a general description of the catchment,
- Section 4 discusses data which was utilised for the study,
- Section 5 describes the modelling procedure,
- Section 6 details results for the design flood events,
- Section 7 reviews the sensitivity of the model to data used,
- Section 8 identifies the provisional flood hazard,
- Section 9 identifies the hydraulic categorisation,
- Section 10 describes the potential flood damages,
- Section 11 reviews the impacts of climate change.

### 3. CATCHMENT DESCRIPTION

The Empire Bay catchment is located within the Gosford City Council (GCC) Local Government Area. It is a sub-catchment of Brisbane Water, which connects to Broken Bay covering an area of about 554 ha. The suburb of Empire Bay and the south-western section of Bensville are included in the study area. Cockle Channel and Cockle Bay are the waterbodies situated on the northern side of the catchment.

Land-use in the catchment is primarily residential with significant areas of bushland / vegetated areas. The density of residential areas varies from low-density detached houses in the main part of Empire Bay and within Bensville, to larger bushland residential lots between these two areas. Several shops are located within the two main residential areas. Large areas of bushland are located on the higher elevations in the southern part of catchment and also along some areas adjoining the estuary including Cockle Bay Nature Reserve.

The Empire Bay catchment is bounded by a ridgeline which runs along the southern and western sides. The southern ridgeline has elevations varying from about RL 60m AHD in the west to a peak of about RL 150m AHD in the south east. Bensville is separated by a ridgeline to Cockle Bay with a highest elevation of about RL 36m AHD. The Empire Bay residential area is relatively flat with an elevation down to about RL 1.0m AHD at the foreshore and the area around Cockle Bay Nature Reserve is also relatively flat.

Pit and piped drainage infrastructure convey stormwater runoff through the main residential areas of Empire Bay and Bensville to the foreshore area. Several drainage depressions and natural channels convey runoff from the bushland areas to piped systems crossings Empire Bay Drive.

## 4. DATA

### 4.1 Community Consultation

A questionnaire was delivered to residences in the Empire Bay study area in October 2007, totalling about 1300. The aim of the questionnaire was to gauge resident's awareness of flooding in the catchment and to identify specific accounts of flood inundation to be used for the calibration of the computer model. One hundred and thirty-two responses were received from across the study area as shown in Figure 4.1. Appendix A includes a copy of the questionnaire and a summary of each response. Photos forwarded by residents are included in Appendix A.

Of the responses, 50% indicated that they have lived in the area for more than 10 years. Fifty percent of respondents were aware of flooding in the catchment, 25% had some awareness of flooding, and 23% were not aware of flooding in the catchment.

The extent of flooding noted by respondents included 7% indicating flooding inside the house, 44% indicating flooding in the yard, and 39% were inconvenienced by flooding events. Similarly, the following areas were nominated as flooded by respondents – backyard 29%, garage 17%, above-floor 5%, below-floor 9%, and frontyard 23%.

Different occasions of storm events recalled by respondents are listed in Table 4.1.

**Table 4.1 Historic Storm Events**

Event Date	Responses	Event Date	Responses
June 2007	41(31%)	October 1985	3
January 1996	17(13%)	November 1984	2
February 1992	7(7%)	February 1981	2
February 1990	4	January 1978	0
January 1989	7	March 1977	1
April 1988	8	May 1974	2

Twenty percent of respondents advised they had noticed bridges / culverts as blocked during storm events. Comments were also made noting that the drainage systems were undersized and debris blocked some pipelines. Debris such as dirt, branches, overgrown grass or weeds were identified as materials blocking pipes.

From 18th September to 16th October 2009, the draft report was placed on public exhibition at Council's administration centre, local libraries and on its website. Comments and submissions were invited for review for the final report. No submissions were received for this Study.

### 4.2 Rainfall

Owing to the small area of the catchment, uniform areal distribution of design storms has been assumed for the hydrologic component of the analysis. Design rainfall depths and temporal patterns for the modelling of 0.5% AEP, 1%, 2%, 5%, 10%, 20%, 50%, and 100% AEP were developed using standard techniques provided in Australian Rainfall and Runoff (1998). The design Intensity-Frequency-Duration (IFD) parameters are presented in Table 4.2

**Table 4.2 Design IFD Parameters**

Parameter	Value
2-Years ARI 1-hour Intensity	38.50 mm/hr
2-Years ARI 12-hours Intensity	8.50 mm/hr
2-Years ARI 72-hours Intensity	2.60 mm/hr
50-Years ARI 1-hours Intensity	77.00 mm/hr
50-Years ARI 12-hours Intensity	17.00 mm/hr
50-Years ARI 72-hours Intensity	5.90 mm/hr
Skew	0.0
F2	4.3
F50	15.9
Temporal Pattern Zone	1

The Probable Maximum Precipitation (PMP) was estimated using the publication “*The Estimation of Probable Maximum Precipitation in Australia: Generalised Short-Duration Method*” (Commonwealth Bureau of Meteorology, 2003). The spatial distribution ellipses of the method are not required due to the small size of the catchment. Table 4.3 shows the data for the PMP calculations.

**Table 4.3 PMP Calculation Values**

Parameter	Value
Moisture Adjustment Factor	0.71
Elevation Adjustment Factor	1.00
Percentage Rough	100%

Estimated average design storm rainfall intensities for the full range of storm events and durations are presented in Table 4.4.

**Table 4.4 Design Rainfall Intensities (mm/h)**

Duration	1 year ARI	2 year ARI	5 year ARI	10 year ARI	20 year ARI	50 year ARI	100 year ARI	200 year ARI	PMP
15 min	62	79	101	113	130	151	168	191	680
30 min	43.7	56	72	82	94	110	122	138	480
45 min	35.1	45.2	58	66	76	90	100	114	413
1 hour	29.8	38.5	50.0	57	65	77	86	98	350
1.5 hour	23.4	30.3	39.3	44.6	51	61	67	76	307
2 hour	19.7	25.5	33.0	37.5	43.3	51	57	65	265
3 hour	15.4	19.9	25.8	29.2	33.8	39.7	44.3	51	217
4 hour	N/A	N/A	N/A	N/A	N/A	N/A	N/A	43	185
4.5 hour	12.0	15.5	20.1	22.8	26.3	31.0	34.5	N/A	N/A
5 hour	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	164
6 hour	10.0	13.0	16.8	19.1	22.1	26.0	28.9	33	143
9 hour	7.84	10.1	13.1	14.9	17.2	20.3	22.6	N/A	N/A



### 4.3 Pit and Pipe Field Survey

Stormwater drainage pit and pipe details were supplied by Gosford City Council. Johnson Partners completed a detailed field survey of parts of the drainage system to supplement Council's information.

Site inspections and the field survey identified that some of the pipelines were below the standing water level at their outlet due to debris blocking the flow of water or due to the tide level at the time of inspection.

### 4.4 Soil Type and Loss Rates

The Department of Conservation and Land Management (NSW) soil map of the Gosford-Lake Macquarie area (1993) and Sydney (1989) identifies the soil types within the Empire Bay catchment. The residential area of Empire Bay is predominantly on Woy Woy soil type with Mangrove Creek soil type located on the western and eastern areas. The bushland / rural residential area between Empire Bay and Bensville has Mangrove Creek soil type adjacent to Cockle Bay, then a large area of Cockle Bay soil type, with Erina soils on the southern side to the higher elevations.

Woy Woy soils in the region are noted for their permanently high water table and seasonal waterlogging. High permeability is identified as a feature of the soil. Mangrove Creek soils are a mix of high and low permeability but generally low permeability where not regularly inundated. High run-on of water is also noted as a limitation of the soil. Characteristics of the Cockle Bay soils are seasonal waterlogging and localised permanent waterlogging. The underlying soils of the group are typically of low permeability. Erina soils have the limitation of high run-on and underlying groups are generally of low permeability.

Responses from the resident questionnaire noted that in some cases water can remain on the ground for a couple of hours, days or even up to a week. The influence of tide levels in the estuary on water ponding in the Empire Bay area was also noted.

### 4.5 Boundary Conditions

The 'Brisbane Water Foreshore Flood Study' (2009) completed by Cardno Lawson Treloar established the water levels and flood behaviour for various design events in Brisbane Water. Simulations for design ARI event conditions were undertaken for 5, 10, 20, 50, 100, 200-years ARI and a PMF event. Peak water level results at 119 foreshore locations in Brisbane Water are presented, with 11 of these sites located within the Empire Bay catchment study area.

Peak water levels are shown in Table 4.5 for the Empire Bay area with the highest level occurring in Cockle Channel in the west and reducing heading eastward to Cockle Bay. The joint probability of severe catchment flooding from the Empire Bay catchment occurring together with severe estuary flooding is low. Hence modelling for the case of a rare storm event, such as 1% AEP, in the catchment with a rare estuary level, such as from a 1% AEP event, as a downstream boundary may not be appropriate.

For the purpose of local creek studies, such as the Empire Bay Catchment Flood Study, the 1% probability of exceedance (PoE) level is to be used as the downstream boundary level in the estuary (Cardno Lawson Treloar, 2009). The 1% probability of exceedance is the level that one can be 99% confident will not be exceeded during any creek flood event. The 1% PoE level for Empire Bay is 0.64m AHD (Cardno Lawson Treloar, 2009).

The Empire Bay Catchment Flood Study assesses flood behaviour due to runoff from the local catchment for the various storm events. Flood behaviour due to elevated water levels in the estuary in storm events is described in the Brisbane Water Foreshore Flood Study (2009).

**Table 4.5 Empire Bay Foreshore Peak Water Level**

<b>Average Recurrence Interval (ARI)</b>	<b>Peak Water Level Range (m AHD)</b>
PMF	1.5 – 1.78
200y ARI	1.41 – 1.57
100y ARI	1.37 – 1.51
50y ARI	1.32 – 1.44
20y ARI	1.27 – 1.37
10y ARI	1.23 – 1.31
5y ARI	1.18 – 1.25

## 5. FLOOD MODELLING

The SOBEK 1D/2D model from WL|Delft Hydraulics Laboratory was used to model the catchment and to hydraulically route overland flood flows and street flow. This modelling system dynamically couples the one-dimensional and two-dimensional flow in the floodplain.

An area of about 887 ha is modelled as shown in Figure 5.1 which includes parts of the estuary. To facilitate the modelling, the catchment has been divided into three sectors comprising the Empire Bay residential area, Bensville residential area, and the bushland residential section which covers the largest area.

### 5.1 Hydrology

The SOBEK modelling of the Empire Bay catchment utilises the rainfall-on-grid methodology for developing the hydrology. In the model, rainfall is applied directly to the 2D terrain, and the hydraulic model automatically routes the flow. The rainfall patterns are described in Section 4.2 and two different loss rates shown in Table 5.1 are applied to the model. Higher loss values are applied to the bushland sector compared to the residential sectors based on the soil types described in Section 4.4 and the larger amount of pervious area in the bushland sector.

**Table 5.1 Hydrology Loss Rates**

Description	Empire Bay Residential and Bensville Residential Values	Bushland Residential Sector Values
Initial Loss	5 mm	20 mm
Continuing Loss	1 mm/h	2.5 mm/h

### 5.2 Piped Drainage Systems

Piped drainage systems are incorporated into the SOBEK model as distinct 1D elements connected to the terrain grid. Detailed field survey by Johnson Partners supplemented the pipe information supplied by Gosford City Council.

The different size of inlet pit openings was included in the model as orifice-links of the same size to represent the restriction of flow into the piped system. An orifice-link was included between pipeline reaches to model the energy losses at pits and between conduits. Generally the open channel sections and drainage depression are represented in the terrain grid but specific lines, such as at Sorrento Road / Kendall Road and Wards Hill Road / Empire Bay Drive, are included as distinct 1D elements.

Figure 5.2 shows the pipe and channel systems incorporated in the model. About 11.6 km of pipeline and 0.2 km of channel systems are modelled. The roughness values adopted for the piped drainage systems are listed in Table 5.2.

**Table 5.2 1D Element Roughness Values**

Component	Roughness Value
Pipe	0.018
Culvert	0.025
Open Channel	0.03

### 5.3 Topography

A terrain grid was developed to represent ground elevations based on aerial laser scanning data from Gosford City Council supplemented by detailed field survey completed by Johnson Partners. Figure 5.3 shows the elevations of the Empire Bay catchment in the model. The ridgeline along the southern part of Empire Bay is at about RL 100 to 150 m AHD and a ridgeline through Bensville has a peak elevation of about RL 37 m AHD. The main residential area of Empire Bay grades from a peak of RL 15m AHD at Empire Bay Drive to about RL 1.0m AHD at the estuary foreshore areas. House footprints were retained at ground elevation to account for some potential storage of floodwaters at these locations (eg under-floor voids, verandah areas, and above-floor inundation).

Details of the elevation grids for the three sectors are listed in Table 5.3.

**Table 5.3 Elevation Grids**

Sector	Area	Grid Resolution	Number of Grid Cells
Empire Bay Residential	175 ha	3m x 3m	389,000
Bensville Residential	46 ha	3m x 3m	102,000
Bushland Residential	667 ha	9m x 9m	230,000

### 5.4 Hydraulic Roughness

Each cell of the elevation grids also has a roughness value to model the influence to flow behaviour of the particular land-use. The adopted roughness layout, shown in Figure 5.4, was based on aerial photographs, site inspections, and Council's land-use zonings. The roughness value applied for each land-use is listed in Table 5.4.

**Table 5.4 Roughness Values**

Land-use	Roughness Value
Channel	0.03
Bushland	0.06
Open Space	0.03, 0.04 or 0.05
Residential	0.09
Vegetated Marsh	0.06 or 0.07
Road	0.02
Estuary	0.02

### 5.5 Model Calibration

The resident questionnaire detailed in Section 4.1 indicated most respondents recalled the June 2007 event. Recollection of storm events prior to June 2007 was less but some details were noted for the April 1988 event.

The data required to calibrate the SOBEM model to particular events includes water levels, event rainfall data, and event water level data. Residents recalled particular flood details for the June 2007 event and the April 1988 event, but insufficient water levels were available for potential calibration to other events. Rainfall and water level data was

available for the June 2007 event and April 1988 event. Thus the model was run for both these events as calibration storms.

### 5.5.1 June 2007 Storm Event

A significant storm event occurred between 7 and 12 June 2007 in the Central Coast area. A pluviograph at Kincumber operated by Manly Hydraulics Laboratory is the nearest rainfall record to the Empire Bay catchment. It is about 3 km from the shops at Empire Bay (Sorrento Road and Kendall Road) at an elevation of about 20m AHD and records data in 2 minute timesteps. Manly Hydraulics Laboratory also operates a pluviograph at Koolewong, about 5.6 km from Empire Bay shops. The location of these sites is shown in Figure 5.5. Rainfall across Cockle Channel from Empire Bay in the suburb of Davistown is recorded by Mr. B. Evans on a daily basis.

Table 5.5 lists the daily rainfall depths over the period of Wednesday 6 June 2007 to Tuesday 12 June 2007. Kincumber rainfall data was obtained from Manly Hydraulics Laboratory and is equivalent to a storm event of about 20% AEP. Koolewong data was obtained from the report "New South Wales Central Coast June 2007 Flood Summary" by NSW Department of Commerce and Manly Hydraulics Laboratory (2007).

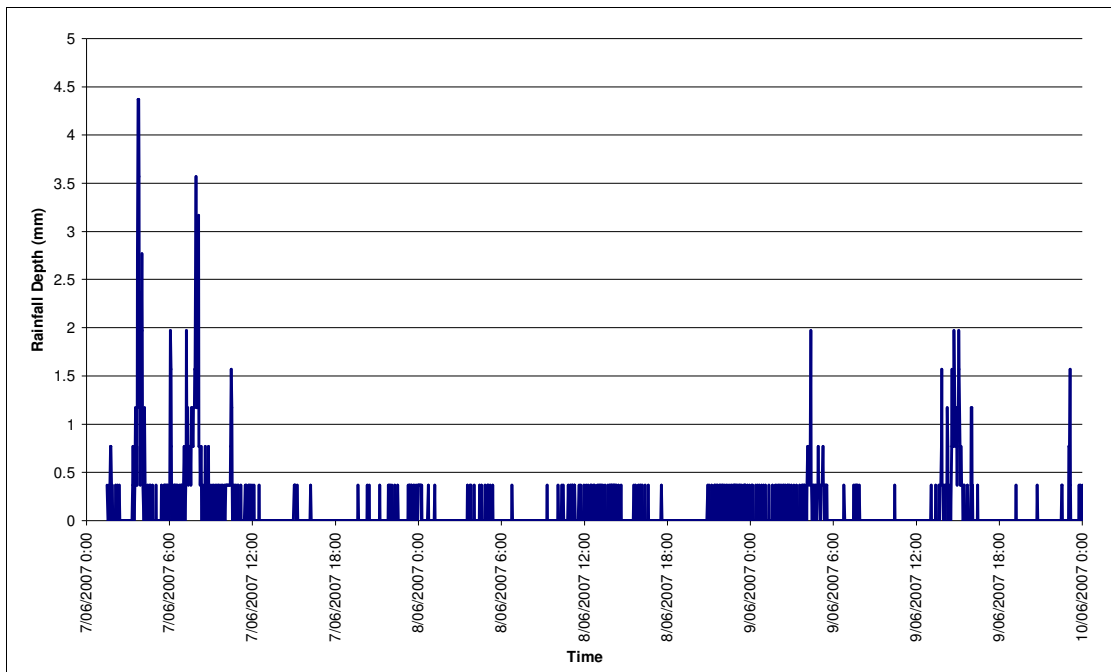
**Table 5.5 Rainfall Depths (mm/d)**

Date	Davistown (from Mr. B. Evans)	Koolewong	Kincumber (depth to 9am)	Ratio of Davistown to Kincumber
6/6/07	0	n/a	0 (in 9 hours from 6/6/07 00:00)	
7/6/07	54	60.5	129	0.4
8/6/07	38	71	45	0.8
9/6/07	63	92.5	82.5	0.8
10/6/07	49	4.5	63.5	0.8
11/6/07	2	0.0	1	2.0
12/6/07	0	n/a	0.5	0.0
<b>TOTAL</b>	<b>206</b>	<b>228.5</b>	<b>321.5</b>	<b>0.6</b>

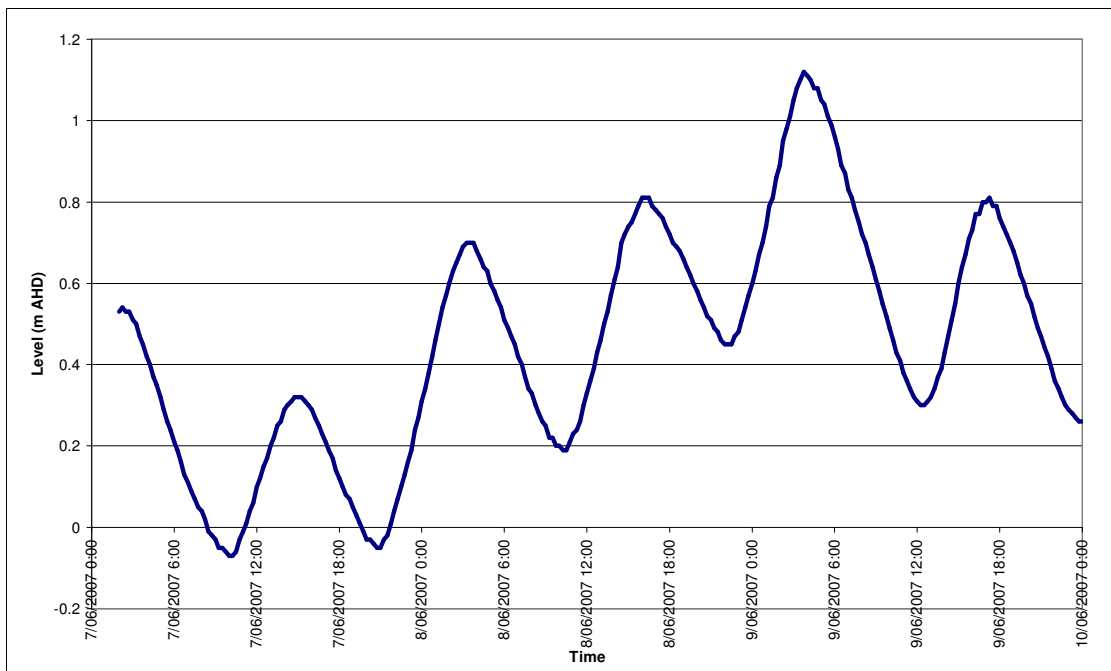
The Kincumber daily rainfall is significantly higher than the Davistown and Koolewong depths. For the calibration model, the Kincumber pluviograph rainfall will be used as the rainfall per day reflects the pattern of the nearby Davistown data better and is located closer to the catchment. The Kincumber rainfall is multiplied by 0.8 to reflect the rainfall recorded in the catchment and a continuing loss of 1 mm/hr is applied to the data. Figure 5.6 shows the adjusted rainfall depths at two minute intervals from the Kincumber pluviograph data.

Manly Hydraulics Laboratory operates a water level recorder at Koolewong (shown in Figure 5.5) which is the closest to the Empire Bay catchment. Figure 5.7 shows the water level time series for the June 2007 storm event. The peak water level of 1.12m AHD occurs at around 06:00 on Saturday 9<sup>th</sup> June, compared to the peak rainfall burst on Thursday 7<sup>th</sup> June though rainfall continued for periods up to and beyond the peak tide time.

The SOBEK model incorporating the adjusted Kincumber rainfall pattern and the Koolewong water level pattern was run for the time period of 02:00 on Thursday 7/6/07 to 00:00 on Sunday 10/6/07.



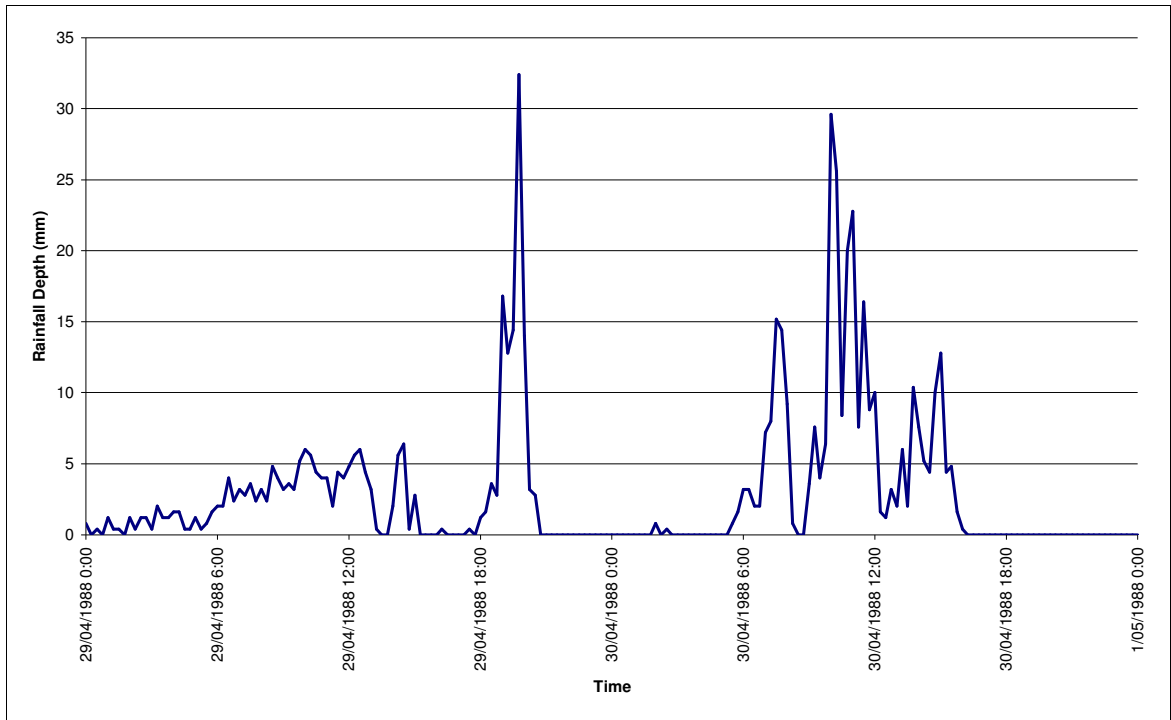
**Figure 5.6 Kincumber Rainfall Depth per two-minutes (adjusted by 0.8) (June 2007)**



**Figure 5.7 Koolewang Water Level Time Series (June 2007)**

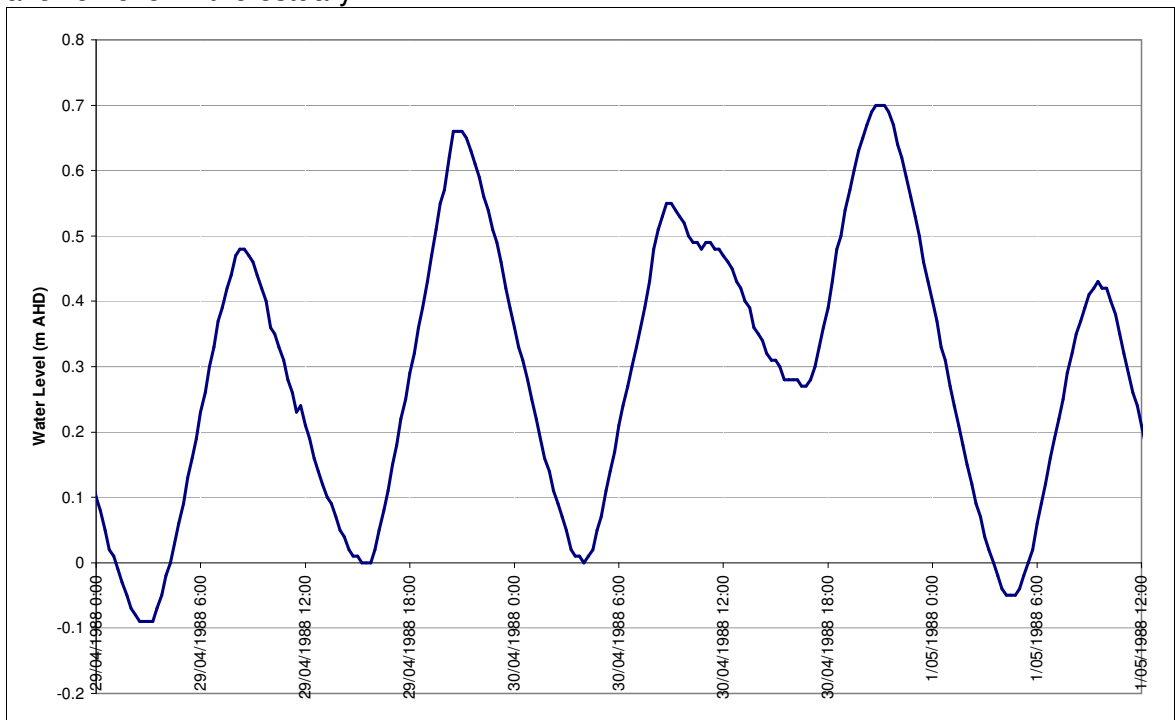
### 5.5.2 April 1988 Event

A storm with a rainfall depth equivalent to the 1% AEP event was experienced on the 29<sup>th</sup> and 30<sup>th</sup> April 1988. A depth of 318 mm was recorded at the Kincumber rainfall gauge on the 29<sup>th</sup> and 396 mm on the 30<sup>th</sup>. Similarly to the June 2007 data, the Kincumber data was adjusted by 0.8 to better represent the rainfall to the Empire Bay catchment. Figure 5.8 shows the rainfall depth in 15 minute timesteps adjusted by 0.8 for the flood modelling period of 29/4/88 00:00 to 1/5/88 00:00.



**Figure 5.8 Kincumber Rainfall Depth per 15-minutes (adjusted by 0.8) (April 1988)**

Figure 5.9 shows the water level recorded at the Koolewong site during the April 1988 storm event. A peak water level of 0.7m was recorded at 21:00 on 30<sup>th</sup> April 1988. In comparison to the June 2007 event, the April 1988 event received more rainfall but had a lower level in the estuary.



**Figure 5.9 Koolewong Water Level Time Series (April 1988)**

### 5.5.3 Results

Table 5.6 shows the results from SOBEK for flood levels satisfactorily model the flood descriptions noted in the questionnaire responses. The surveyed levels were obtained by Johnson Partners. Table 5.7 shows anecdotal flood inundation noted by respondents in the questionnaire and similar results modelled by SOBEK. The peak depths for the June 2007 event and April 1988 event are shown in Figure 5.10 and Figure 5.11 respectively.

**Table 5.6 Model Results for June 2007 and April 1988 event – Surveyed (m AHD)**

Location	Description	Surveyed Level (m AHD)	SOBEK Model June 2007	SOBEK Model April 1988	Comment
10 Pomona Road	~1988-89 within 25mm of floor	Dwelling floor level = 20.32	Peak WL 20.67 @ 0.05m depth	Peak WL 20.69 @ 0.08m depth	Model ground elevation is higher than floor thus ground level detail not sufficient for compare
8 Pomona Road	1988 & 1989 house flooded; about 300mm deep across yard	Dwelling floor level = 20.105	Peak WL 20.37 @ depth 0.06m; yard peak ~ 0.16m	Peak WL 20.40 @ depth 0.09m; yard peak ~ 0.22m	Model ground elevation is higher than floor thus ground level detail not sufficient for compare
8 Boongala Avenue	April 1988 water to bottom step and flooded garage	RL bottom step = 1.23; RL garage floor = 1.23	Peak WL 1.29 @ depth 0.08m	Peak WL 1.40 @ depth 0.19m	Model ground elevation close to levels thus potential ground level detail not sufficient for compare
1 Boongala Avenue	Not flooded above floor in 1988 or 2007 as house was raised post-1974	Dwelling floor level = 1.43	Peak WL 1.15 @ depth 0.12	Peak WL 1.37, @ depth 0.34	Satisfactory
11 Valencia Street	June 2007 water came up to house	RL flood level = 2.34	Peak WL 2.40 @ depth 0.05 (northside); Peak WL 2.22 @ depth 0.09 (southside)	Peak WL 2.42 @ depth 0.07 (northside); Peak WL 2.25 @ depth 0.12 (southside)	Satisfactory – location of level not specified
12 Valencia Street	2007 basement flooded	Dwelling floor level = 6.19	Peak WL 6.37 @ depth 0.02	Peak WL 6.38 @ depth 0.03	Model ground elevation is higher than floor thus ground level detail not sufficient for compare
12 Allawa Close	Inside house flooded - ~1988	Dwelling floor level = 5.80	Depth < 0.01m	Depth < 0.01m	Model depth < 0.01m thus detail not sufficient to compare



**Table 5.7 Model Results for June 2007 – Anecdotal Reports**

Location	Description	SOBEK Model June 2007
6 Gordon Rd	~200mm deep across Gordon Road in Jun07, Jan96, Apr88. Apr88 water at steps below back door	~0.3m across Gordon Rd; ~0.2m in back yard
9 Gordon Road	Jun07 water across Gordon Rd but ok to drive through	~0.25m deep
2 Boongala Ave	Jun07 property has never flooded	<0.06m deep
52A Sorrento Road	Not flooded. In heavy rainfall Gordon Road connecting to Boongala Road covered by water	Peak ~0.11m deep on property. Gordon / Boongala intersection up to ~0.4m deep.
	Boongala Avenue has been like a river. Boongala always floods with heavy downpour	Depths across Boongala ~0.05 to 0.50m
14 Gordon Rd	Gordon Road about 1 foot deep	Depths on road near property ~0.05 to 0.40m
22 Myrtle Road	Jun07 backyard completely underwater, almost to backdoor	Yard mostly inundated to depths ~0.01 to 0.17m
8 Allawa Close	Jun07 rose to ~0.3m covering backyard and partial front yard	Part front yard to peak 0.07m, flood across backyard peak ~0.08m
11 Echuca Road	Jun07 ~3 inches in backyard	Peak ~0.10m
5 Echuca Road	No property flooding. Water covers intersection of Echuca Road and Rickard Road in downpours	Isolated locations on property peak~0.08m. Intersection ~0.15 to 0.45m deep
70 Shelley Beach Road	Jun07 property access blocked. Front yard ~8 inches deep	~0.30m deep across road. Front yard ~0.18m deep
55 Shelley Beach Road	Jun07 bottom end of rear yard ~0.2m	Peak ~ 0.2m
4 Shelley Beach Road	Jun07 road completely cut	Peak depth on road ~0.2m
41 Hillcrest Ave	Never affected by flooding	Peak depth in yard ~0.04m
9 Allawa Close	Jun07 paddock under 5-10cms water	Depth across paddock ~0.05 to 0.15m
Killdare Street	Jun07 no reported flooding	Isolated locations of water of peak depth up to 0.09m
10 Emma Street	Jun07 Backyard up to 0.20m deep	Backyard depth up to 0.17m
6 Emma Street	Jun07 up to 60mm from side neighbours properties	Peak about 0.06m runoff from neighbours not road
Empire Bay Drive (between Wards Hill Rd and Awinya Cl)	Several reports of no flooding	Road isolated inundation up to ~0.15m, respondent properties up to ~0.07m

## 6. DESIGN FLOOD ESTIMATION

Flood behaviour was modelled in SOBEK for a series of Annual Exceedance Probabilities (AEP). The 0.5% AEP, 1%, 2%, 5%, 10%, 20%, 50%, and 100% AEP and Probable Maximum Flood (PMF) events were modelled for local catchment runoff with a 1% Probability of Exceedance estuary level.

### 6.1 Critical Duration

The critical duration for the Empire Bay catchment was evaluated by reviewing the peak water level results for a range of durations for the 20% AEP, 1% AEP and PMF events. Durations of 15 minutes, 30, 60, 90, 120, and 180 minutes were modelled for the 1% AEP event. PMF durations of 15 minutes, 30, 45, 60 and 90 minutes were modelled. Reference locations across the catchment, shown in Figure 6.1, are listed in Table 6.1 for evaluating the results.

**Table 6.1 Sensitivity Analysis Reference Locations**

Number	Location
1	Intersection Gordon Rd & Boongala Ave
2	Intersection Sorrento Rd & Gordon Rd
3	Sorrento Rd near Kendall Rd
4	Intersection Greenfield Rd and Rickard R
5	Intersection Shelly Beach Rd & Sher Cl
6	Greenfield Rd
7	Empire Bay Dr (near Awinya Cl)
8	Palmers Lane
9	Empire Bay Dr
10	Empire Bay Dr
11	Wards Hill Rd
12	21 Pomona Rd
13	Pomona Rd
14	Valencia St
15	Emma St
16	Kildare St

The critical durations for the PMF, 1% AEP, and 20% AEP are shown in Figure 6.2, Figure 6.3 and Figure 6.4 respectively.

For the 1% AEP and 20% AEP, the higher elevations in the catchment show a shorter critical duration (about 15 minutes) compared to the lower elevations and less steep areas (between 90 and 180 minutes). Table 6.2 shows peak water levels at the reference locations for the 1% AEP durations modelled. Comparison of the separate durations for the 1% AEP and 20% AEP shows that the 2 hour duration storm is the critical event for the catchment as:

- there is no difference to peak water levels (in excess of +/- 0.01m) between the 2 hour and 15 minute storm,

- peak water levels for the 90 minute storm are lower than the 2 hour storm in some locations, but is generally only slightly higher (<0.02m) in other locations.

**Table 6.2 1% AEP Critical Duration**

Point	Peak Flood Level (m AHD) for Duration						Critical Duration
	15 min	30 min	60 min	90 min	120 min	180 min	
1	1.21	1.30	1.36	1.36	1.37	1.36	120 min
2	1.13	1.17	1.22	1.24	1.24	1.24	120 min
3	1.31	1.31	1.31	1.31	1.31	1.31	90 min
4	1.36	1.41	1.46	1.46	1.47	1.44	120 min
5	1.25	1.25	1.25	1.25	1.25	1.25	120 min
6	1.76	1.76	1.78	1.77	1.79	1.76	120 min
7	9.34	9.37	9.39	9.38	9.40	9.37	120 min
8	6.49	6.59	6.66	6.64	6.69	6.63	120 min
9	7.66	8.54	8.78	8.78	8.82	8.66	120 min
10	8.92	8.98	9.03	9.03	9.04	9.00	120 min
11	12.08	12.09	12.12	12.10	12.12	12.11	120 min
12	16.09	16.52	16.81	16.81	16.93	16.63	120 min
13	17.69	17.72	17.73	17.75	17.76	17.73	120 min
14	1.29	1.30	1.29	1.31	1.30	1.28	90 min
15	1.79	1.79	1.80	1.80	1.80	1.78	120 min
16	7.86	7.89	7.85	7.89	7.87	7.83	30 min

## 6.2 Model Scenarios

Catchment models were therefore run in SOBEK for the durations shown in Table 6.3.

**Table 6.3 Model Scenarios**

AEP	Rainfall Durations [min]	Estuary Level [m AHD]
PMF	15, 30, 45, 60, 90	0.64 (1% PoE)
0.5%	120	0.64 (1% PoE)
1%	15, 30, 60, 90, 120, 180	0.64 (1% PoE)
2%	120	0.64 (1% PoE)
5%	120	0.64 (1% PoE)
10%	120	0.64 (1% PoE)
20%	15, 30, 60, 90, 120, 180	0.64 (1% PoE)
50%	120	0.64 (1% PoE)
100%	120	0.64 (1% PoE)

## 6.3 Results

Model results of the flood extent, peak depth, peak level, and peak flow speed due to local catchment runoff are shown in the figures as included in Volume 2 of this Study Report. Each of the figures is presented on two sheets – Sheet A focussing on the Empire Bay residential area and Sheet B focussing on the Empire Bay bushland residential and Bensville areas. As the rainfall-on-grid modelling methodology in SOBEK models rainfall on every cell within the extent, the results figures are filtered and show flood parameters at locations where the depth is greater than or equal to 0.10m. Model results from the foreshore within the estuary are not shown for clarity of presentation. This filtering process improves interpretation of results for evaluating areas with significant runoff. The extent figures presented therefore show locations where the flow depth is greater than or equal to 0.10m.

**Table 6.4 Model Results Figures**

<b>AEP</b>	<b>Flood Extent</b>	<b>Peak Water Levels</b>	<b>Peak Flood Depth</b>	<b>Peak Flow Speed</b>
PMF	Figure 6.5 A & B	Figure 6.14 A & B	Figure 6.23 A & B	Figure 6.32 A & B
0.5%	Figure 6.6 A & B	Figure 6.15 A & B	Figure 6.24 A & B	Figure 6.33 A & B
1%	Figure 6.7 A & B	Figure 6.16 A & B	Figure 6.25 A & B	Figure 6.34 A & B
2%	Figure 6.8 A & B	Figure 6.17 A & B	Figure 6.26 A & B	Figure 6.35 A & B
5%	Figure 6.9 A & B	Figure 6.18 A & B	Figure 6.27 A & B	Figure 6.36 A & B
10%	Figure 6.10 A & B	Figure 6.19 A & B	Figure 6.28 A & B	Figure 6.37 A & B
20%	Figure 6.11 A & B	Figure 6.20 A & B	Figure 6.29 A & B	Figure 6.38 A & B
50%	Figure 6.12 A & B	Figure 6.21 A & B	Figure 6.30 A & B	Figure 6.39 A & B
100%	Figure 6.13 A & B	Figure 6.22 A & B	Figure 6.31 A & B	Figure 6.40 A & B

## 7. SENSITIVITY ANALYSIS

The sensitivity of the model was tested to demonstrate the range of uncertainty in the model results for changes in key parameters. The following variables were tested for sensitivity:

- Catchment rainfall – increased and decreased by 20%
- Catchment roughness – increased and decreased by 20%
- Downstream boundary condition – increased and decreased by 20%
- Culvert and pipe blockage – for all systems and for particular systems
- Future conditions.

The impact of potential climate change scenarios, such as increased sea levels and increased rainfall intensity was also modelled as described in Section 11.

The sensitivity modelling was undertaken for the 2 hour duration event which is the critical duration for the Empire Bay catchment. The variables were assessed for the 1% AEP event except for the pipe blockage scenario which was modelled for the 20% AEP event. Results for the varied parameters for the selected reference locations are included in Appendix B. These reference locations are listed in Table 6.1 and included on Figure 6.1.

### 7.1 Catchment Rainfall

The average rainfall intensity for the 1% AEP 2 hour duration was increased by 20% and decreased by 20% for the sensitivity analysis. The resultant average intensities for the events were: 57 mm/h for the standard storm, 68.4 mm/h for the 20% increased scenario, and 45.6 mm/h for the 20% decreased scenario. Initial loss and continuing loss rates were applied to the resultant five minute timestep rainfall patterns.

The peak water levels shown in Table B.1 (in Appendix B) show that the 20% adjustment to the rainfall results in changes to the base case levels of several centimetres around the main residential areas at Bensville and Empire Bay. The largest fluctuations in water levels occur in the vicinity of Pomona Road and Palmers Lane where runoff from the high elevation areas is concentrated into drainage channels. The model shows consistent results as the reduced rainfall results in lower peak water levels and higher peak water levels for the increased rainfall scenario.

### 7.2 Catchment Roughness

Values of the hydraulic roughness parameter applied to the model, described in Section 5.4, were increased and decreased by 20% for the sensitivity analysis. Resultant peak water levels at the reference locations are listed in Table B.2 (Appendix B).

Water levels do not vary significantly in most reference locations as they are in locations where flow is not concentrated into a main flowpath conveying large flowrates. Locations 9, 10, and 12 show the largest variation for the roughness parameter as runoff from the upstream bushland areas are concentrated to these points. The model shows consistent results as the increased roughness results in an increase to the peak water level (noting that the difference at Location 8 is not significant at less than 0.01m different).

### 7.3 Downstream Boundary Condition

The downstream boundary condition applied to the model influences peak water levels in lower parts of the catchment where the estuary level controls peak water levels. Table B.3 (Appendix B) shows the variation in water levels for the 1% AEP 2h event with for three scenarios of water level in the Brisbane Waters estuary –

- Base scenario – 1% PoE level of 0.64m AHD,
- 20% decrease to base scenario at 0.51m AHD,
- 20% increase to base scenario at 0.77m AHD.

All of the reference locations are at elevations above the varied estuary level and thus peak water levels are unaffected. The change shown at Location 12 is not significant as it less than 0.01m.

### 7.4 Culvert and Pipe Blockage

Two scenarios for pipe blockage were analysed for the 20% AEP 2 hour storm event. The case of all pipes and culverts blocked was evaluated and a case for particular pipes and culverts blocked was also modelled. The drainage lines selected for the second case were based on determining flood behaviour that may result in a higher peak water level for certain areas. Specifically those selected were the downstream reaches of pipeline branches and pipelines located in properties that conveyed runoff from upstream roads or areas. Figure 7.1 shows the piped drainage infrastructure in the model, and the lines blocked for the second scenario.

Table B.4 in Appendix B shows that the peak water levels are generally increased by blockages to pipelines and culverts. Some of the reference locations are unaffected or increased only slightly (up to 0.01m) but other areas show a higher increase, such as Locations 1, 4, 7, 9, and 16.

Blockage of the pipelines results in a decrease in peak water levels (up to 0.09m) at intersection of Gordon Road and Boongala Avenue as the pipelines from Rickard Road do not convey runoff to Gordon Road. Instead, peak water levels increase around Rickard Road and flow overland towards the intersection of Sorrento Road and Boongala Avenue. Peak water levels increase along Rickard Road from about 0.05m at Myrtle Road to 0.16m near Kendall Road. The peak water level at the intersection of Sorrento Road and Boongala Avenue increases by about 0.10m for the pipe blockage scenario.

Peak water levels across Empire Bay Drive also increase at several locations where water is generally conveyed across the road through culverts. The increase due to pipe blockage is up to 0.11m near Awinya Close and rises by about 0.17m near Wards Hill Road.

### 7.5 Future Conditions

The Empire Bay catchment is effectively fully developed within Council's property zoning in the area. Land zoned for general residential development (zone 2(a)) is located within the north-western area of Empire Bay (north of Empire Bay Drive and West of Kendall Road), and in Bensville. Between these two areas, the zoning is generally 7(c2) for rural-residential properties or undeveloped areas (zoned 8 Nature Reserves or 6(a) Open Space Recreation).

Potential development identified by Council may comprise amending the zoning of rural-residential properties along Empire Bay Drive to low-density residential (2(a)). The SOBEK model was amended to represent these amended sites as low-density residential areas. Figure 7.2 shows the locations of the amended sites.

Figure 7.2 also shows the resultant difference in peak water levels for the increased development. Peak water levels are shown to increase at scattered locations within the re-developed areas by up to 0.11m.

## 8. PROVISIONAL FLOOD HAZARD

### 8.1 General

Flood hazard can be defined as the risk to life and limb and damage caused by a flood. The hazard caused by a flood varies both in time and place across the floodplain. The Floodplain Development Manual (NSW Government, 2005) describes various factors to be considered in determining the degree of hazard. These factors are:

- Size of the flood,
- Depth and velocity of floodwaters,
- Effective warning time,
- Flood awareness,
- Rate of rise of floodwaters,
- Duration of flooding,
- Evacuation problems,
- Access.

Hazard categorisation based on all the above factors is part of establishing a Floodplain Risk Management Plan. The scope of the present study calls for determination of provisional flood hazards only, which when considered in conjunction with the above listed factors provides comprehensive analysis of the flood hazard.

### 8.2 Provisional Flood Hazard

Provisional flood hazard is determined through a relationship developed between the depth and velocity of floodwaters as detailed in the Floodplain Development Manual (NSW Government, 2005). The provisional hazard is defined as either High or Low as shown in Figure 8.1. The transition zone between high and low is assumed as high hazard.

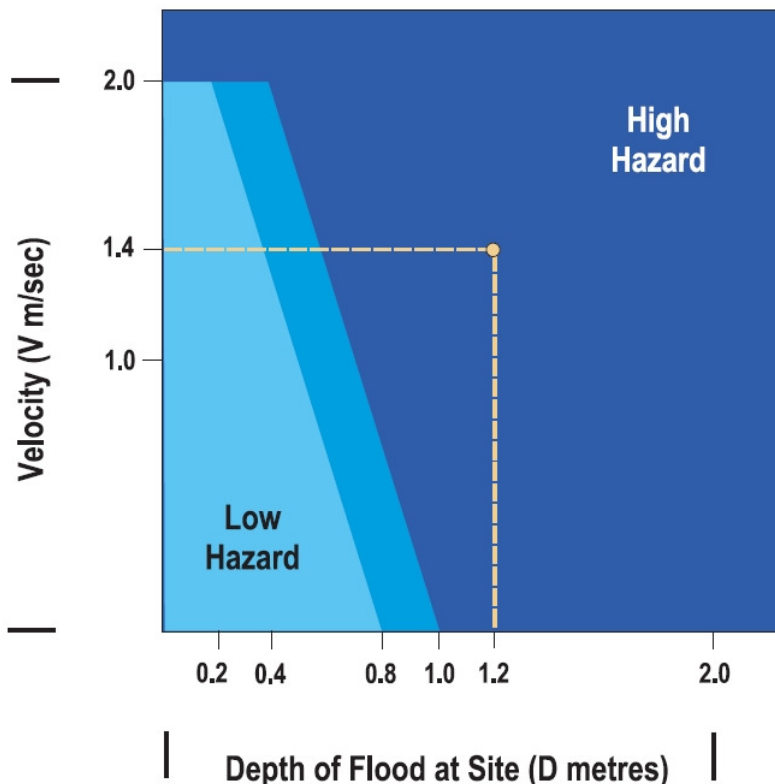


Figure 8.1 Provisional Hazard Classification (NSW Government)



The provisional flood hazard is determined using equations based on the graphs of Figure 8.1 relating the velocity and depth. Provisional hazard due to local catchment runoff determined in the flood model for the PMF, 1% AEP, 5% AEP, and 20% AEP events are shown in Figures 8.2 to 8.5 respectively.

In the 1% AEP event high hazard areas are shown in the channel near Myler Avenue and channels near Empire Bay Drive. Isolated areas adjacent to Empire Bay Drive are shown as high hazard locations. Some roadways are inundated in the 1% AEP event and are shown as low hazard conditions.

In the PMF event some roads are identified as provisional high hazard in the main residential areas of Empire Bay and Bensville. A large area of provisional high hazard is shown in the area between Pomona Road and across Empire Bay Drive to Palmers Lane.

## 9. HYDRAULIC CATEGORISATION

### 9.1 General

Hydraulic categorisation of the floodplain is used in the development of the Floodplain Risk Management Plan. The Floodplain Development Manual (2005) defines flood prone land to fall into one of the following three hydraulic categories:

- **Floodway** - Areas that convey a significant portion of the flow. These are areas that, even if partially blocked, would cause a significant increase in flood levels or a significant redistribution of flood flows, which may adversely affect other areas.
- **Flood Storage** - Areas that are important in the temporary storage of the floodwater during the passage of the flood. If the area is substantially removed by levees or fill it will result in elevated water levels and/or elevated discharges. Flood Storage areas, if completely blocked would cause peak flood levels to increase by 0.1m and/or would cause the peak discharge to increase by more than 10%.
- **Flood Fringe** - Remaining area of flood prone land, after Floodway and Flood Storage areas have been defined. Blockage or filling of this area will not have any significant affect on the flood pattern or flood levels.

### 9.2 Hydraulic Category Identification

Floodways were determined for the 1% AEP, 5% AEP, 20% AEP and PMF by considering those model branches that conveyed a significant portion of the total flow. These branches, if blocked or removed, would cause a significant redistribution of the flow. The criteria used to define the floodways are described below.

As a minimum, the floodway was assumed to follow the creekline from bank to bank. In addition, the following depth and velocity criteria were used to define a floodway:

- Velocity \* Depth product must be greater than 0.25 m<sup>2</sup>/s **and** velocity must be greater than 0.25 m/s; **OR**
- Velocity is greater than 1 m/s.

Flood storage was defined as those areas outside the floodway, which if completely filled would cause peak flood levels to increase by 0.1 m and/or would cause peak discharge anywhere to increase by more than 10%. The criteria were applied to the model results as described below.

Previous analysis of flood storage in 1D cross sections assumed that if the cross-sectional area is reduced such that 10% of the conveyance is lost, the criteria for flood storage would be satisfied. To determine the limits of 10% conveyance in a cross-section, the depth was determined at which 10% of the flow was conveyed. This depth, averaged over several cross-sections, was found to be 0.2 m (Howells et al, 2003). Thus the criteria used to determine the flood storage is:

- Depth greater than 0.2m
- Not classified as floodway.

All areas that were not categorised as Flood Way or Flood Storage, but still fell within the flood extent, where the depth is greater than 0.05 m, are represented as Flood Fringe.

The hydraulic categories for the PMF, 1% AEP, 5% AEP, and 20% AEP based on the peak depth and velocity from local catchment runoff determined in the flood model, are shown in Figures 9.1 to 9.4 respectively.

Floodways are shown in the 1% AEP event along part of Greenfield Rd, Murrong Road, Empire Bay Drive and on parts of Empire Bay Drive. Sections of private property and the drainage channels around Pomona Road are also categorised as floodway. Flood storage areas occur across the catchment and concentrated in parts of Gordon Road, Boongala Avenue, Rickard Road and Palmers Lane.

In the PMF event the main residential area of Empire Bay is predominantly categorised as flood storage and several roads are classified as floodway. Large areas of floodway are shown in the rural residential area of Empire Bay, particularly along the main drainage channels around Pomona Road to Palmers Lane. Empire Bay Drive and several streets in Bensville are categorised as floodways.

## 10. ANNUAL AVERAGE DAMAGE

### 10.1 Background

The economic impact of flooding can be defined by what is commonly referred to as 'flood damages'. Table 10.1 lists classifications of various types of flood damages incurred in a catchment. Direct damage costs are just one component of the entire cost of a flood event. There are also indirect costs. Both direct and indirect costs are referred to as 'tangible' costs. In addition to this there are also 'intangible' costs. The values discussed in this report are the 'total' damages and include an assumed intangible cost of 25% of the tangible cost.

**Table 10.1 Types of Flood Damages**

Type	Description
Direct	Building contents (internal) Structural (building repair and clean) External items (vehicles, contents of sheds etc)
Indirect	Clean-up (immediate removal of debris) Financial (loss of revenue, extra expenditure) Opportunity (non-provision of public services)
Intangible	Social – increased levels of insecurity, depression, stress General inconvenience in post-flood stage

Flood damages can be assessed by a number of means including the use of programs such as FLDAMAGE or ANUFLOOD or via more generic methods using spreadsheets. For the purposes of this project, generic spreadsheets have been developed based on damage curves adapted from the Department of Environment and Climate Change (DECC) [formerly Department of Infrastructure Planning and Natural Resources (DIPNR)].

### 10.2 Stage – Damage Curves

The Stage-Damage curves are based on the category of property identified within the floodplain, being:

- Residential
- Commercial
- Industrial

The Empire Bay catchment consists of predominantly residential dwellings, with some on large rural-residential allotments. Several commercial properties are located at the intersection of Kendall Road and Sorrento Road, Empire Bay and at Kallaroo Avenue, Bensville. In the study area there are 1131 residential properties and eight commercial properties. No properties within the study area are categorised as industrial.

#### 10.2.1 Residential

The draft DECC (DIPNR) Floodplain Management Guideline No.4 *Residential Flood Damage Calculation* (2004) was used for this study. This guideline includes a template spreadsheet program that determines damage curves for three types of residential buildings:

- Single Storey, slab on ground
- Two Storey, slab on ground

- Single Storey, 'high-set' eg piered structures (floor level assumed to be 1.5m above the ground).

All buildings were assumed to be single storey slab on ground with floor levels 0.30m above a ground level obtained by ALS at the dwelling.

The DECC (DIPNR) curves are derived for late 2001 (base curves). It is recommended to adjust values in the base residential damage curves by Average Weekly Earnings (AWE), rather than by the inflation rate as measured by the Consumer Price Index (CPI). While not specified, we have assumed that the base curves were derived in November 2001, which allows the use of November 2001 AWE statistics (issued quarterly). November 2001 AWE is shown in Table 10.2. The most recent data for AWE from the Australian Bureau of Statistics at the time of assessment was for August 2008. AWE values were sourced from the Australian Bureau of Statistics (ABS, 2008).

**Table 10.2 AWE Statistics from 2001 and 2008**

Month	Year	AWE
November	2001	\$676.40
August	2008	\$897.90
Change		32.7%

All ordinates in the base residential flood damage curves were therefore converted into August 2008 dollars. The residential damage curve is shown in Figure 10.1.

Damages are generally incurred on a property prior to any over floor flooding. The curves allow for a damage of \$8,891 (August 2008 dollars) to be incurred when the water level reaches the base of the house (determined as 0.1m below the floor level). Damage was assumed to occur for depths of water over the ground of 0.2m or more (that is, 0.1m below the floor level). Multi-unit properties, such as villas and townhouses, are calculated as a single dwelling.

## 10.2.2 Commercial

Commercial damage curves were determined based on those included in the FLDamage Manual (Water Studies Pty Ltd, 1992). FLDamage allows for three types of commercial properties:

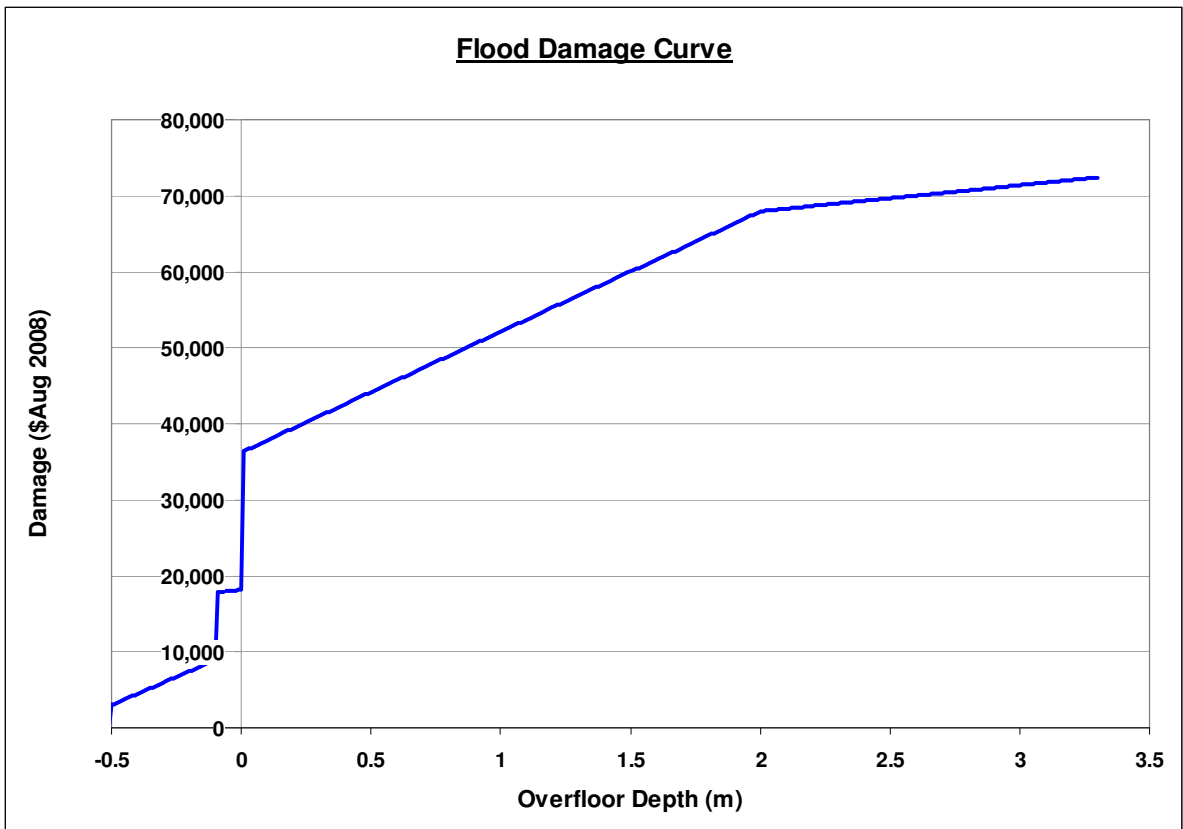
- Low Value Commercial
- Medium Value Commercial
- High Value Commercial.

The FLDamage curves have a base date of 1990. The Consumer Price Index (CPI) was used to adjust the 1990 data to December 2008 dollars (this data was obtained from the Australian Bureau of Statistics website (ABS, 2009). It was assumed that the FLDamage data was in June 1990 dollars. The CPI data is shown in Table 10.3.

**Table 10.3 CPI Statistics from 1990 and 2008**

Month	Year	CPI
June	1990	102.50
December	2008	166.00
Change		61.95%

Consequently, ordinates on the 1990 damage curves have been increased by 61.95% and GST has been included.



**Figure 10.1 Residential Flood Damage Curve**

In determining the ordinates on the damage curves, it has been assumed that the effective warning time is approximately zero, and the loss of trading days as a result of the flooding has been taken as 10.

The commercial properties in the study area are assumed as low-value commercial with a floor area of 100 m<sup>2</sup>.

### 10.3 Results

Table 10.4 outlines the flood impacts due to local catchment runoff. Based on the analysis, the average annual damage for the catchment as modelled is estimated to be \$548,125. A total of about 1140 dwellings are included in the assessment.

**Table 10.4 Flood Damage Summary**

Event/Property Type	Number of Properties with overfloor flooding	Maximum Overfloor Flooding Depth (m)	Number of Properties with overground flooding (within 0.10m of floor level)	Total Damage (\$Dec 2008)
<b>PMF</b>				
Residential	142	0.76	252	\$7,385,609
Commercial	1	0.03	2	\$4,094
PMF Total	143		254	\$7,389,703
<b>0.5% AEP</b>				
Residential	41	0.26	61	\$1,655,750
Commercial	0	N/A	0	\$0
0.5% AEP Total	41		61	\$1,655,750
<b>1% AEP</b>				
Residential	22	0.24	51	\$1,336,782
Commercial	0	N/A	0	\$0
1% AEP Total	22		51	\$1,336,782
<b>2% AEP</b>				
Residential	20	0.22	45	\$1,165,192
Commercial	0	N/A	0	\$0
2% AEP Total	20		45	\$1,165,192
<b>5% AEP</b>				
Residential	14	0.19	40	\$965,120
Commercial	0	N/A	0	\$0
5% AEP Total	14		40	\$965,120
<b>10% AEP</b>				
Residential	11	0.14	30	\$705,320
Commercial	0	N/A	0	\$0
10% AEP Total	11		30	\$705,320
<b>20% AEP</b>				
Residential	8	0.12	25	\$576,385
Commercial	0	N/A	0	\$0
20% AEP Total	8		25	\$576,385
<b>50% AEP</b>				
Residential	5	0.09	16	\$363,591
Commercial	0	N/A	0	\$0
50% AEP Total	5		16	\$363,591
<b>100% AEP</b>				
Residential	1	0.05	9	\$181,102
Commercial	0	N/A	0	\$0
100% AEP Total	1		9	\$181,102

## 11. CLIMATE CHANGE

Increased sea levels and increased rainfall intensities are expected to result from climate change effects. Potential impacts to flood behaviour in the Empire Bay catchment due to climate change have been analysed.

The Department of Environment and Climate Change in the guideline 'Practical Consideration of Climate Change' (2007) recommended that climate change assessments review three scenarios of increases to rainfall intensities: 10%, 20%, and 30%. A sea-level rise of up to 0.91m was identified as potentially occurring by the year 2100 due to climate change impacts. Council also nominated 0.2m sea level rise for assessment.

The 'Brisbane Water Foreshore Flood Study' (Cardno Lawson Treloar, 2009) assessed impacts to the Brisbane Water area resulting from sea level rise. Modelling showed that a rise in the mean sea level will result in an equivalent rise of the design levels within the estuary.

A combination of scenarios was modelled for the critical storm event of 1% AEP 2 hour duration with a base estuary level of the 1% probability of exceedance (PoE) level (0.64m AHD):

1. 10% increase to rainfall intensities,
2. 20% increase to rainfall intensities,
3. 30% increase to rainfall intensities,
4. 0.2m rise in estuary level,
5. 0.2m rise in estuary level and 30% increase to rainfall intensities,
6. 0.91m rise in estuary level,
7. 0.91m rise in estuary level and 30% increase to rainfall intensities.

Table C.1 (in Appendix C) lists results for the increased peak water levels at the reference locations of Figure 6.1 resulting from Scenarios 1, 2, and 3. The increased rainfall intensities show that peak water levels are generally unaffected or increased by up to 0.05m in some locations. However, in channels where runoff is concentrated and flows to the estuary, such as near Pomona Road and Empire Bay Drive, the peak water levels are increased significantly.

Table C.2 lists the peak water level results for Scenarios 4 and 5 with a 0.2m rise in estuary level and Table C.3 lists peak water levels for Scenarios 6 and 7. Figures 11.1 and 11.2 show the peak depths (>0.10m) for Scenario 5 and Scenario 7 respectively.

A 0.2m rise in estuary level above the 1% PoE level (namely 0.84m AHD) is below the general elevation of roads and properties in Empire Bay and Bensville, thus most areas are unaffected. The 30% increase to rainfall intensities results in similar peak water levels than those shown in Table C.1 without the raised estuary level showing that the dominant flooding in this scenario is the catchment runoff.

A 0.91m rise in estuary level above the 1% PoE level (namely 1.55m AHD) is higher than a large proportion of the properties in the main residential area of Empire Bay. Thus these properties are inundated by water flooding from the estuary rather than specifically from catchment runoff.



## 12. REPORT QUALIFICATIONS

This report has been prepared for Gosford City Council to define the nature and extent of flooding for the study area of the Empire Bay catchment. Hydrologic and hydraulic modelling was completed to assess flood behaviour within the catchment. Flood modelling is based on local catchment flooding only, the impact of flood levels from the Brisbane Water estuary has not been accounted for in the modelling. Estuary flooding is described in the Brisbane Water Foreshore Flood Study (2009). Flow characteristics including depth, velocity and provisional hazard were evaluated based on the computer modelling.

The investigation and modelling procedures adopted for this study follow current best practice and considerable care has been applied to the preparation of the results. However, model set-up and calibration depends on the quality of data available and there will always be some uncertainties. The flow regime and the flow control structures are very complicated and can only be represented by schematised model layouts.

Hence there will be an unknown level of uncertainty in the results and this should be borne in mind in their application.

Study results should not be used for purposes other than those for which they were prepared.

### 13. CONCLUSION

Flood modelling of local catchment runoff was completed the Empire Bay catchment for a range of annual exceedance probabilities of storms from 100% AEP to 0.5% AEP and up to a PMF event.

In a 1% AEP event, the modelling shows that some properties and roads may be inundated up to 0.5m. This occurs notably around Gordon Road, Boongala Avenue, Rickard Road, and Greenfield Road in the main residential area of Empire Bay and around the main drainage channels at Pomona Road and Empire Bay Drive to Palmers Lane in the rural-residential area. In the 1% AEP event, the modelling showed that 22 houses are flooded above the floor level when the storm runoff is combined with a 1% probability of exceedance level in the estuary.

Mapping of high provisional hazard in the catchment for the 1% AEP event shows that it is limited to the channel behind Myler Avenue and the channel north of Pomona Road which conveys runoff to Allawa Close. Scattered occurrences of high hazard also feature adjacent to parts of Empire Bay Drive. Hydraulic categorisation mapping for the 1% AEP shows floodway areas along the main watercourses from Pomona Road across Empire Bay Drive, and also some road near the main residential areas of Empire Bay and Bensville. Flood storage areas are identified in some properties, roads, and the open space / vegetated areas within the catchment. Increases to sea levels due to climate change have the potential to significantly affect flood impacts, particularly in the low elevation areas of the Empire Bay catchment.

The Floodplain Management Authority's Prioritisation Ranking table for the Empire Bay catchment is included as Appendix D.

## 14. REFERENCES

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Howells, L., McLuckie, D., Collings, G., Lawson, N. (2003). Defining the Floodway – Can One Size Fit All?. Floodplain Management Authorities of NSW 43rd Annual Conference, Forbes, February 2003.

Murphy, C. (1993). Soil Landscapes of the Gosford – Lake Macquarie 1:100,000 Sheet, Department of Conservation and Land Management (NSW).

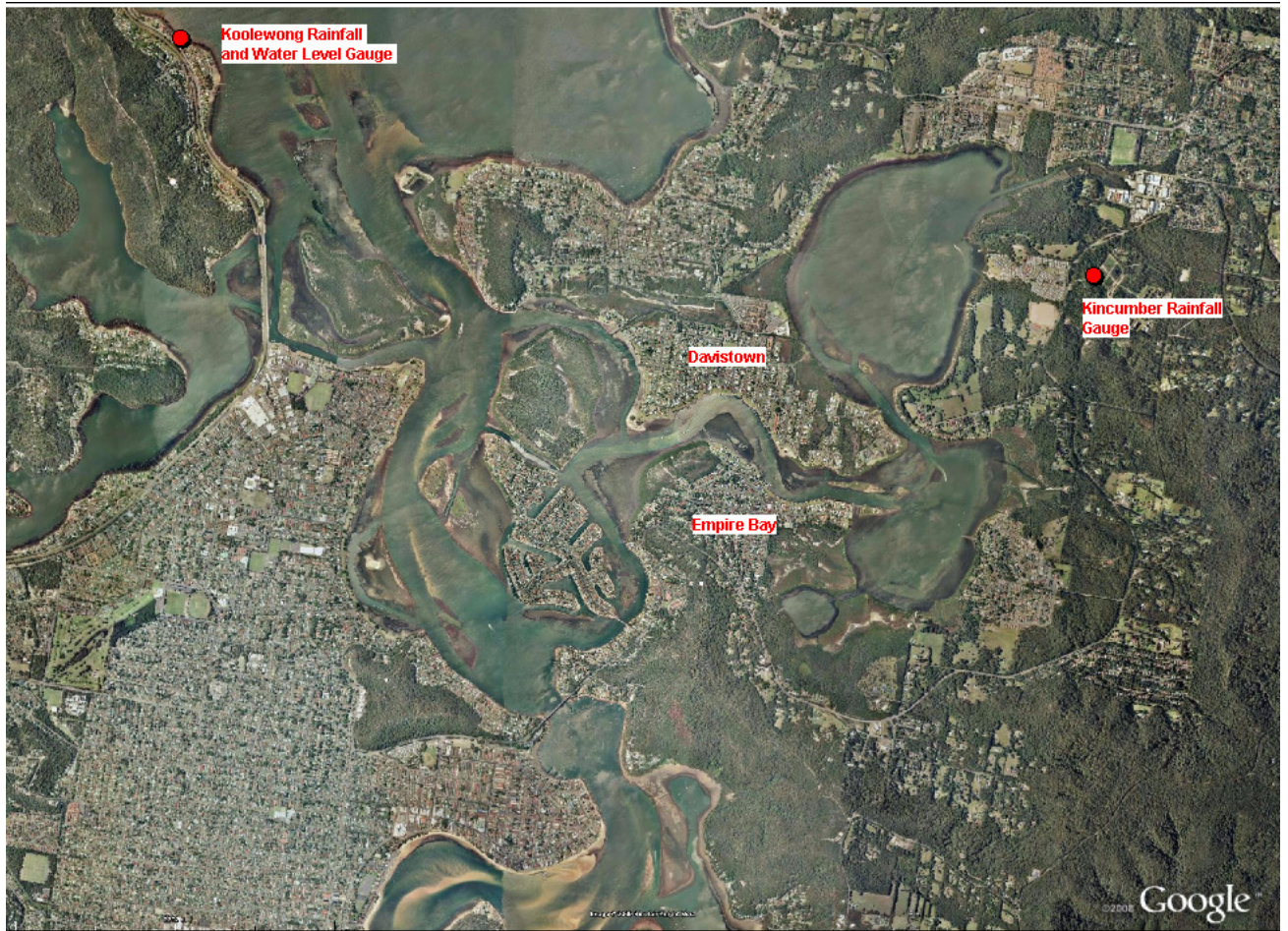
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NSW Government (2005). Floodplain Development Manual.

## FIGURES



Figure 1.1 Site Locality



**Figure 5.5 Rainfall Stations Locations**

## APPENDIX A

### Resident Questionnaire

Our Ref W4715  
Contact Andrew Reid  
10 October 2007



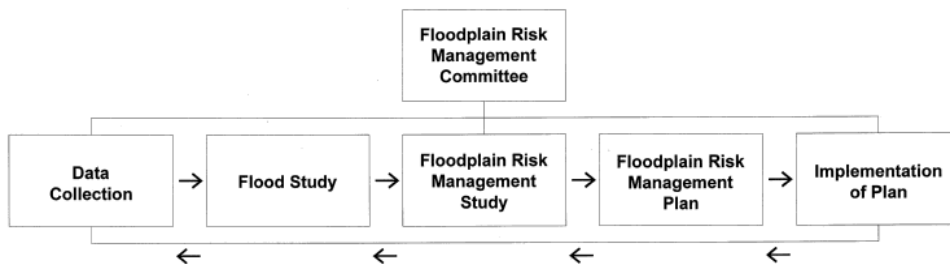
**To The Resident**

Dear Sir/Madam,

**EMPIRE BAY & BENSVILLE SOUTH CATCHMENT FLOOD STUDY**

Cardno Lawson Treloar have been commissioned by Gosford City Council to undertake a Flood Study for the Empire Bay and Bensville South catchment. A catchment layout and the study area are shown on the attached figure.

This Flood Study will form part of the overall Flood Plain Risk Management process (Figure 1) for the catchment, and can be used to optimise development potential, and to obtain social and economic benefits from the reduction in flood damages.



**Figure 1: The Floodplain Risk Management Process**

The Flood Study comprises a comprehensive technical investigation of flood behaviour in the catchment. The study defines the nature and extent of the flood risk by providing information on the level and velocity of floodwaters and on the distribution of flood flows at various locations in the floodplain.

The Flood Study provides a technical basis from which the Floodplain Risk Management Study (FRMS) and Floodplain Risk Management Plan (FRMP) are developed. They are usually completed in one project and would be completed immediately after the completion of the Flood Study, subject to grant funding.

The FRMS identifies, assesses and compares various risk management options and considers opportunities for environmental enhancement as part of floodplain management measures.

The FRMP provides input into the strategic and statutory planning roles of council. It also documents the adopted management strategy formally approved by Council after assessment of submissions following public exhibition.

The final stage of the process is the implementation of the Plan (which would need to compete for funding from various government sources where works are an option).

**Cardno Lawson Treloar Pty Ltd**  
ABN 55 001 882 873

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2072 Australia  
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Gold Coast  
Gosford  
Baulkham Hills  
Wollongong  
Busselton

Papua New Guinea  
Indonesia  
Vietnam  
China  
Kenya  
United Arab Emirates  
United Kingdom  
United States





Community involvement is important at all stages of the Floodplain Management Process. Resident's local knowledge of the catchment and personal experiences of flooding provide an invaluable source of data to define the nature and extent of flooding at the Flood Study stage of the process. In this regard, Council seeks your assistance in undertaking this Flood Study.

You can participate in the study process through your local community associations who can represent your views at Council's Floodplain Risk Management Committee (FRMC) meetings. The FRMC is responsible for overseeing the study and to ensure it follows the Floodplain Risk Management Process. Your community representatives on the FRMC are:

Ms. Shirley Crocker (President) & Mr. Maurie Pacey  
Empire Bay & District Progress Association  
31 Rickard Road  
Empire Bay NSW 2257  
Phone (02) 4369 2034

Enclosed please find a questionnaire, which focuses on whether your property or any nearby property has been flooded in the past. This questionnaire is similar to one completed by some residents in August 2006. However, this questionnaire covers the entire Empire Bay and Bensville South Catchment and allows for responses based on the June 2007 storm event to be incorporated into the Flood Study assessment process.

Please take the time to read the questions and answer them as best as you can. Any information you provide may prove vital to the success and accuracy of the study results.

Would you **please return** the questionnaire in the enclosed reply paid envelope **within three weeks** of receipt of this letter.

Please contact Andrew Reid from Cardno Lawson Treloar or Jim Gowing from Gosford City Council if you want to discuss or clarify items regarding the catchment study.

- Andrew Reid
  - Cardno Lawson Treloar
  - Telephone: 02 9499 3000
  - Facsimile: 02 9499 3033
  - Email: [andrew.reid@cardno.com.au](mailto:andrew.reid@cardno.com.au)
  
- Jim Gowing
  - Gosford City Council
  - Telephone: 02 4235 8818
  - Facsimile: 02 4323 2528
  - Email: [jim.gowing@gosford.nsw.gov.au](mailto:jim.gowing@gosford.nsw.gov.au)

Yours faithfully



*Andrew Reid*  
*Project Engineer*  
for **Cardno Lawson Treloar**

Encl. Empire Bay and Bensville South Questionnaire

# EMPIRE BAY & BENSVILLE SOUTH CATCHMENT FLOOD ASSESSMENT STUDY

## QUESTIONNAIRE

---

Please answer the following questions as best as you can. When you have finished answering the questions, please return these pages in the enclosed "reply paid" envelope.

If you have any queries, please contact:

---

**Andrew Reid** – CARDNO LAWSON TRELOAR

Ph: 02 9499 3000

Fax: 02 9499 3033

andrew.reid@cardno.com.au

**Jim Gowing** – GOSFORD CITY COUNCIL

Ph: 02 4325 8818

Fax: 02 4323 2528

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### Question 1

Could you please provide us with the following details? We may need to contact you to check some of the information with you.

(The information will remain completely CONFIDENTIAL)

Name: \_\_\_\_\_

Day time phone Number: \_\_\_\_\_

Email Address: \_\_\_\_\_

Address: \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_

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### Question 2

How long have you lived in this locality?

\_\_\_\_\_ Years \_\_\_\_\_ Months

Have you previously lived at another address within the catchment (shown on the attached map)?

Details: \_\_\_\_\_

\_\_\_\_\_

\_\_\_\_\_



### Question 3

Are you aware of flooding in the catchment?

*Please Tick One:*

Aware \_\_\_\_\_

Some knowledge \_\_\_\_\_

Not Aware \_\_\_\_\_

---

### Question 4

Have you ever been inconvenienced, or has your property been flooded because of uncontrolled floodwater in this locality?

(Your property may have been flooded inside the house or in your backyard, or you might have been stopped from getting to work)

*Please Tick:*

INSIDE HOUSE FLOODED - YES \_\_\_\_\_ NO \_\_\_\_\_

PROPERTY/YARD FLOODED - YES \_\_\_\_\_ NO \_\_\_\_\_

INCONVENIENCED - YES \_\_\_\_\_ NO \_\_\_\_\_

---

### Question 5

Can you remember when that was?

*Please Tick:*

YES \_\_\_\_\_

NO \_\_\_\_\_

If you answered YES, please give us as much detail as possible.  
To assist, flooding may have occurred on the following dates:

1. June 2007
2. January 1996
3. February 1992
4. February 1990
5. January 1989
6. April 1988
7. October 1985
8. November 1984
9. February 1981

- 10. January 1978
- 11. March 1977
- 12. May 1974
- 13. \_\_\_\_\_
- 14. \_\_\_\_\_

Details of flooding and when it occurred:

(How long after the rain started? How high was the water level? How long did it stay at this level? When did the water level reach its peak?)

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**Question 6**

If you have experienced flooding in the area, do you have any evidence of the extents of the floods (such as flood levels or depths at certain locations)?

*Please Tick:*

YES \_\_\_\_\_

NO \_\_\_\_\_

If you answered YES, please give as much detail as possible.

*You may have an old photograph, or may have taken a video. Some people remember marks on walls and posts, and this information could prove quite important. Alternatively, you may know someone who has lived in the locality for a long time who might have that type of information.*

**Details of information:**

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**Question 7**

If you answered Yes to Question 6, what type of property did you see flooded?

*You may tick more than one:*

- |             |       |               |       |
|-------------|-------|---------------|-------|
| RESIDENTIAL | _____ | COMMERCIAL    | _____ |
| PARKS       | _____ | ROADS & PATHS | _____ |
| OTHER       | _____ |               |       |

Please Specify: \_\_\_\_\_

Can you describe the area of the property that was flooded?

*You may tick more than one.*

- |                              |       |
|------------------------------|-------|
| BACKYARD                     | _____ |
| GARAGE                       | _____ |
| BUILDING (ABOVE FLOOR LEVEL) | _____ |
| BUILDING (BELOW FLOOR LEVEL) | _____ |
| FRONTYARD                    | _____ |
| OTHER                        | _____ |

Please Specify: \_\_\_\_\_

**Question 8**

Did you notice any bridges and/or culverts to be blocked during the event?

\_\_\_\_\_ YES                      \_\_\_\_\_ NO

If YES, please provide details (please mark the location on the map if possible), and how blocked would you say it was? (eg. 50% blocked, 80% blocked)

\_\_\_\_\_  
\_\_\_\_\_

If YES, what was causing the blockage? (eg. woody debris, shopping trolley, vehicle)

\_\_\_\_\_  
\_\_\_\_\_



**Question 9**

If possible, can you show the location of the flooding on the enclosed map?

*Please Tick:*

YES \_\_\_\_\_

NO \_\_\_\_\_

*If you have indicated yes, please remember to enclose the map in the envelope, clearly marked.*

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**Question 10**

Is there anything else you can tell us about the flooding in this locality?

If so, please provide the information below.

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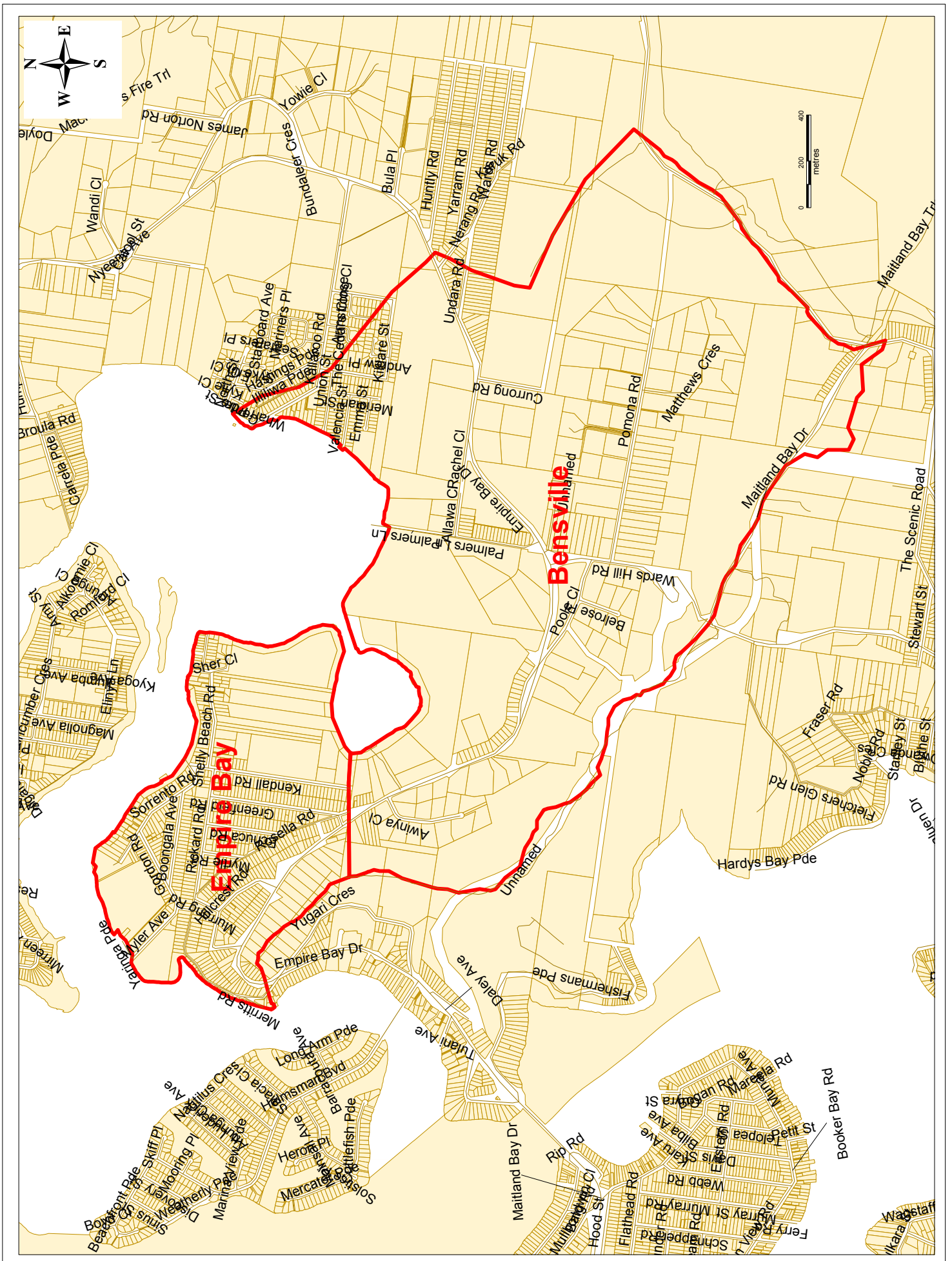
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**Thank you for providing the above information. Please remember to put it back in the reply paid envelope. A representative from Cardno Lawson Treloar may contact you in the near future to discuss your response.**



















**Photos forwarded by residents**



May 2001 – Bundaleer Cres, Bensville



December 2005 - Bundaleer Cres, Bensville



August 2006 – Rickard Road, Empire Bay



2006 – Vicinity of Wards Hill Road



2006 – Vicinity of Wards Hill Road

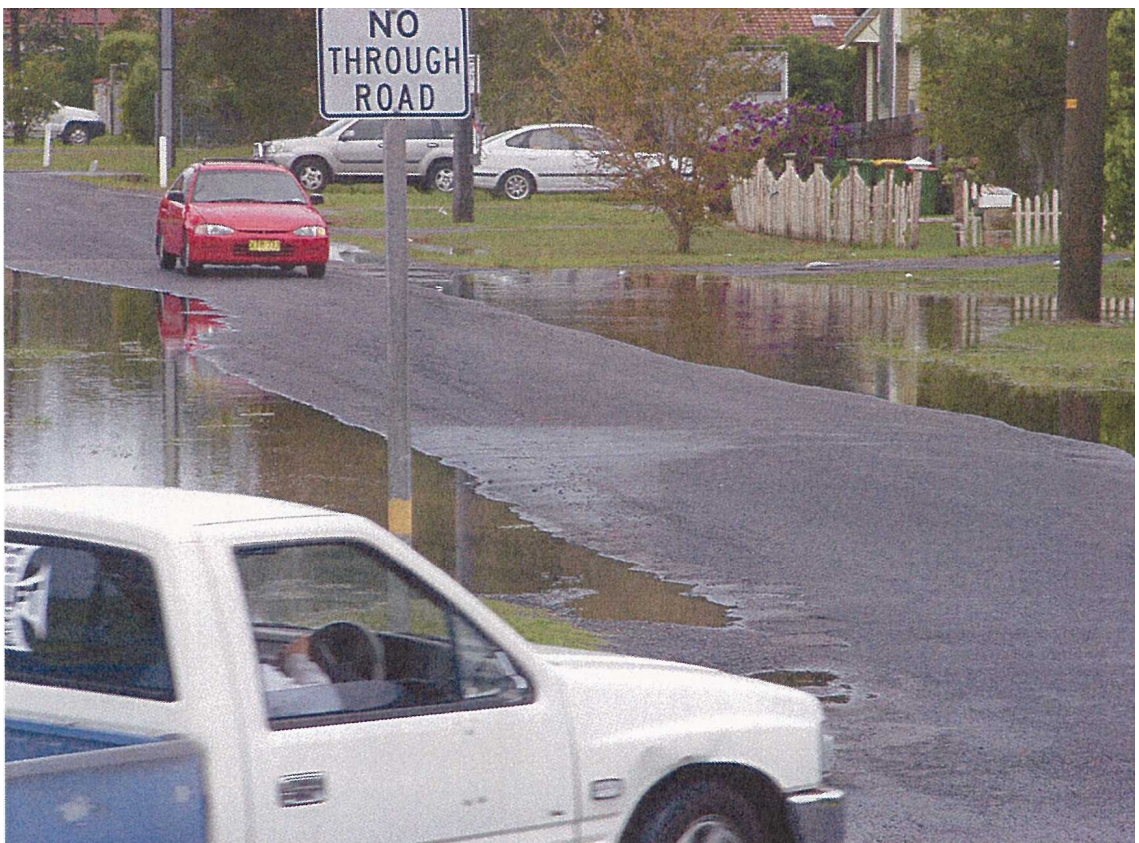


2006 – Vicinity of Wards Hill Road





2006 – Vicinity of Wards Hill Road



June 2007



June 2007



June 2007



June 2007 – Park at Sorrento Road



June 2007



07:00 June 9 2007 – at Sorrento Road



November 2007 - Boongala Road



Open Drain behind houses on Rickard Rd



Date and location not specified



Date and location not specified

## **APPENDIX B**

### **Sensitivity Analysis**



**Table B.1 Catchment Rainfall Sensitivity**

Point	Location	Base Case	20% Decrease		20% Increase	
		Peak Water Level (m AHD)	Peak Water Level (m AHD)	Difference to Base Case (m)	Peak Water Level (m AHD)	Difference to Base Case (m)
1	Intersection Gordon Rd & Boongala Ave	1.37	1.32	-0.05	1.41	0.04
2	Intersection Sorrento Rd & Gordon Rd	1.24	1.20	-0.04	1.28	0.03
3	Sorrento Rd near Kendall Rd	1.31	1.31	0.00	1.31	0.00
4	Intersection Greenfield Rd and Rickard R	1.47	1.42	-0.05	1.50	0.04
5	Intersection Shelly Beach Rd & Sher Cl	1.25	1.25	0.00	1.26	0.00
6	Greenfield Rd	1.79	1.76	-0.03	1.81	0.03
7	Empire Bay Dr (near Awinya Cl)	9.40	9.38	-0.02	9.42	0.02
8	Palmers Lane	6.69	6.62	-0.07	6.72	0.03
9	Empire Bay Dr	8.82	8.69	-0.14	8.87	0.05
10	Empire Bay Dr	9.04	9.01	-0.03	9.06	0.02
11	Wards Hill Rd	12.12	12.11	-0.01	12.13	0.01
12	21 Pomona Rd	16.93	16.72	-0.21	17.09	0.16
13	Pomona Rd	17.76	17.74	-0.02	17.77	0.02
14	Valencia St	1.30	1.29	-0.01	1.31	0.01
15	Emma St	1.80	1.79	-0.02	1.82	0.01
16	Kildare St	7.87	7.85	-0.02	7.90	0.03

\* Location of reference points shown on Figure 6.1.

**Table B.2 Catchment Roughness Sensitivity**

Point	Location	Base Case	20% Decrease		20% Increase	
		Peak Water Level (m AHD)	Peak Water Level (m AHD)	Difference to Base Case (m)	Peak Water Level (m AHD)	Difference to Base Case (m)
1	Intersection Gordon Rd & Boongala Ave	1.37	1.36	-0.01	1.38	0.01
2	Intersection Sorrento Rd & Gordon Rd	1.24	1.24	-0.01	1.25	0.00
3	Sorrento Rd near Kendall Rd	1.31	1.31	0.00	1.31	0.00
4	Intersection Greenfield Rd and Rickard R	1.47	1.46	0.00	1.47	0.00
5	Intersection Shelly Beach Rd & Sher Cl	1.25	1.25	0.00	1.25	0.00
6	Greenfield Rd	1.79	1.78	0.00	1.80	0.01
7	Empire Bay Dr (near Awinya Cl)	9.40	9.39	-0.01	9.40	0.00
8	Palmers Lane	6.69	6.64	-0.05	6.68	-0.01
9	Empire Bay Dr	8.82	8.74	-0.09	8.88	0.06
10	Empire Bay Dr	9.04	9.01	-0.03	9.06	0.02
11	Wards Hill Rd	12.12	12.12	0.00	12.13	0.01
12	21 Pomona Rd	16.93	16.85	-0.08	17.00	0.07
13	Pomona Rd	17.76	17.73	-0.02	17.77	0.01
14	Valencia St	1.30	1.30	-0.01	1.31	0.01
15	Emma St	1.80	1.79	-0.01	1.82	0.01
16	Kildare St	7.87	7.86	0.00	7.87	0.00

\* Location of reference points shown on Figure 6.1.

**Table B.3 Downstream Boundary Sensitivity**

Point	Location	Ground Elevation (m AHD)	Base Case	20% Decrease		20% Increase	
			Peak Water Level (m AHD)	Peak Water Level (m AHD)	Difference to Base Case (m)	Peak Water Level (m AHD)	Difference to Base Case (m)
1	Intersection Gordon Rd & Boongala Ave	1.17	1.37	1.37	0.00	1.37	0.00
2	Intersection Sorrento Rd & Gordon Rd	1.08	1.24	1.24	0.00	1.24	0.00
3	Sorrento Rd near Kendall Rd	1.30	1.31	1.31	0.00	1.31	0.00
4	Intersection Greenfield Rd and Rickard R	1.35	1.47	1.47	0.00	1.47	0.00
5	Intersection Shelly Beach Rd & Sher Cl	1.25	1.25	1.25	0.00	1.25	0.00
6	Greenfield Rd	1.62	1.79	1.79	0.00	1.79	0.00
7	Empire Bay Dr (near Awinya Cl)	9.30	9.40	9.40	0.00	9.40	0.00
8	Palmers Lane	6.23	6.69	6.69	0.00	6.69	0.00
9	Empire Bay Dr	7.60	8.82	8.82	0.00	8.82	0.00
10	Empire Bay Dr	8.85	9.04	9.04	0.00	9.04	0.00
11	Wards Hill Rd	12.05	12.12	12.12	0.00	12.12	0.00
12	21 Pomona Rd	15.33	16.93	16.92	-0.01	16.93	0.00
13	Pomona Rd	17.60	17.76	17.75	0.00	17.75	0.00
14	Valencia St	1.22	1.30	1.30	0.00	1.30	0.00
15	Emma St	1.75	1.80	1.80	0.00	1.81	0.00
16	Kildare St	7.73	7.87	7.87	0.00	7.87	0.00

\* Location of reference points shown on Figure 6.1.

**Table B.4 Pipe Blockage Sensitivity (20% AEP)**

Point	Location	Base Case	All Pipes Blocked		Selected Pipes Blocked	
		Peak Water Level (m AHD)	Peak Water Level (m AHD)	Difference to Base Case (m)	Peak Water Level (m AHD)	Difference to Base Case (m)
1	Intersection Gordon Rd & Boongala Ave	1.26	1.23	-0.04	1.25	-0.01
2	Intersection Sorrento Rd & Gordon Rd	1.16	1.15	-0.01	1.16	0.01
3	Sorrento Rd near Kendall Rd	1.31	1.31	0.00	1.31	0.00
4	Intersection Greenfield Rd and Rickard R	1.37	1.44	0.07	1.45	0.08
5	Intersection Shelly Beach Rd & Sher Cl	1.25	1.25	0.00	1.25	0.00
6	Greenfield Rd	1.73	1.74	0.01	1.73	0.01
7	Empire Bay Dr (near Awinya Cl)	9.35	9.43	0.08	9.41	0.06
8	Palmers Lane	6.58	6.58	0.00	6.58	0.00
9	Empire Bay Dr	8.52	8.66	0.15	8.66	0.15
10	Empire Bay Dr	8.96	8.96	0.00	8.96	0.00
11	Wards Hill Rd	12.10	12.10	0.01	12.10	0.00
12	21 Pomona Rd	16.43	16.42	-0.01	16.43	0.00
13	Pomona Rd	17.72	17.73	0.01	17.72	0.00
14	Valencia St	1.28	1.28	0.00	1.28	0.00
15	Emma St	1.77	1.78	0.00	1.77	0.00
16	Kildare St	7.82	7.92	0.10	7.92	0.10

\* Location of reference points shown on Figure 6.1.

## **APPENDIX C**

### **Climate Change**

**Table C.1 Climate Change Assessment – Increased Rainfall (1% AEP 2h)**

Point	Location	Base Case	10% Increased Rainfall		20% Increased Rainfall		30% Increased Rainfall	
		Peak Water Level (mAHD)	Peak Water Level (mAHD)	Diff. to Base Case (m)	Peak Water Level (mAHD)	Diff. to Base Case (m)	Peak Water Level (mAHD)	Diff. to Base Case (m)
1	Intersection Gordon Rd & Boongala Ave	1.37	1.39	0.02	1.41	0.04	1.43	0.05
2	Intersection Sorrento Rd & Gordon Rd	1.24	1.26	0.02	1.28	0.03	1.29	0.05
3	Sorrento Rd near Kendall Rd	1.31	1.31	0.00	1.31	0.00	1.31	0.00
4	Intersection Greenfield Rd and Rickard R	1.47	1.49	0.02	1.50	0.04	1.52	0.05
5	Intersection Shelly Beach Rd & Sher Cl	1.25	1.25	0.00	1.26	0.00	1.26	0.01
6	Greenfield Rd	1.79	1.80	0.01	1.81	0.03	1.82	0.04
7	Empire Bay Dr (near Awinya Cl)	9.40	9.41	0.01	9.42	0.02	9.42	0.02
8	Palmers Lane	6.69	6.70	0.02	6.72	0.03	6.73	0.04
9	Empire Bay Dr	8.82	8.85	0.03	8.87	0.05	8.97	0.15
10	Empire Bay Dr	9.04	9.05	0.01	9.06	0.02	9.08	0.03
11	Wards Hill Rd	12.12	12.13	0.01	12.13	0.01	12.14	0.02
12	21 Pomona Rd	16.93	17.00	0.07	17.09	0.16	17.22	0.30
13	Pomona Rd	17.76	17.77	0.01	17.77	0.02	17.78	0.03
14	Valencia St	1.30	1.31	0.01	1.31	0.01	1.32	0.02
15	Emma St	1.80	1.81	0.01	1.82	0.01	1.82	0.02
16	Kildare St	7.87	7.88	0.01	7.90	0.03	7.91	0.04

\* Location of reference points shown on Figure 6.1.

**Table C.2 Climate Change Assessment – 0.2m Raised Estuary Level**

Point	Location	1% AEP 2h (Base Case)	1% AEP 2h Storm and 0.2m Raised Estuary Level		1% AEP 2h Storm with additional 30% rainfall & 0.2m Raised Estuary Level	
		Peak Water Level (mAHD)	Peak Water Level (mAHD)	Difference to Base (m)	Peak Water Level (mAHD)	Difference to Base (m)
1	Intersection Gordon Rd & Boongala Ave	1.37	1.37	0.00	1.43	0.06
2	Intersection Sorrento Rd & Gordon Rd	1.24	1.25	0.00	1.29	0.05
3	Sorrento Rd near Kendall Rd	1.31	1.31	0.00	1.31	0.00
4	Intersection Greenfield Rd and Rickard R	1.47	1.47	0.00	1.52	0.05
5	Intersection Shelly Beach Rd & Sher Cl	1.25	1.25	0.00	1.26	0.01
6	Greenfield Rd	1.79	1.79	0.00	1.82	0.04
7	Empire Bay Dr (near Awinya Cl)	9.40	9.40	0.00	9.42	0.02
8	Palmers Lane	6.69	6.69	0.00	6.73	0.04
9	Empire Bay Dr	8.82	8.82	0.00	8.97	0.15
10	Empire Bay Dr	9.04	9.04	0.00	9.08	0.03
11	Wards Hill Rd	12.12	12.12	0.00	12.14	0.02
12	21 Pomona Rd	16.93	16.92	-0.01	17.22	0.29
13	Pomona Rd	17.76	17.75	0.00	17.78	0.03
14	Valencia St	1.30	1.30	0.00	1.32	0.02
15	Emma St	1.80	1.80	0.00	1.82	0.02
16	Kildare St	7.87	7.87	0.00	7.91	0.04

\* Location of reference points shown on Figure 6.1.

**Table C.3 Climate Change Assessment – 0.91m Raised Estuary Level**

Point	Location	1% AEP 2h (Base Case)	1% AEP 2h Storm and 0.91m Raised Estuary Level		1% AEP 2h Storm with additional 30% rainfall & 0.91m Raised Estuary Level	
		Peak Water Level (mAHD)	Peak Water Level (mAHD)	Difference to Base (m)	Peak Water Level (mAHD)	Difference to Base (m)
1	Intersection Gordon Rd & Boongala Ave	1.37	1.58	0.21	1.60	0.22
2	Intersection Sorrento Rd & Gordon Rd	1.24	1.56	0.31	1.56	0.32
3	Sorrento Rd near Kendall Rd	1.31	1.56	0.25	1.56	0.25
4	Intersection Greenfield Rd and Rickard R	1.47	1.59	0.12	1.61	0.14
5	Intersection Shelly Beach Rd & Sher Cl	1.25	1.55	0.30	1.55	0.30
6	Greenfield Rd	1.79	1.77	-0.01	1.81	0.03
7	Empire Bay Dr (near Awinya Cl)	9.40	9.39	-0.01	9.42	0.02
8	Palmers Lane	6.69	6.68	-0.01	6.73	0.04
9	Empire Bay Dr	8.82	8.80	-0.02	8.94	0.12
10	Empire Bay Dr	9.04	9.04	0.00	9.07	0.03
11	Wards Hill Rd	12.12	12.11	-0.01	12.14	0.01
12	21 Pomona Rd	16.93	16.87	-0.06	17.17	0.24
13	Pomona Rd	17.76	17.75	0.00	17.78	0.02
14	Valencia St	1.30	1.55	0.25	1.55	0.25
15	Emma St	1.80	1.80	0.00	1.82	0.02
16	Kildare St	7.87	7.86	-0.01	7.90	0.03

\* Location of reference points shown on Figure 6.1.



## APPENDIX D

### FMA Prioritisation Ranking

## Prioritisation of catchment areas regarding the undertaking of Flooding and Drainage Works Flood Mitigation Works

### Empire Bay Catchment

#### Categories

##### 1. Hazard level in area

Place tick in appropriate boxes  
(applies to worst recorded event)

##### 2. Social Impact

Place tick in appropriate boxes  
(Existing or anticipated problems)

##### 3. Scale of problem - No. of dwellings affected

Place tick in one box for highest no. dwellings affected  
by above floor flooding  
(applies to worst recorded event)

##### 4. Scale of problem - % of dwellings flooded

Place tick in one box for highest % dwellings  
affected by overfloor flooding.  
(applies to worst recorded event)

##### 5. Scale of Problem - Frequency of over floor flooding

Place tick in one box for highest no. incidences of  
over floor flooding  
(all recorded events)

##### 6. Evacuation

Placed tick in appropriate boxes  
(applies to worst recorded event)

##### 7. Damage

Place tick in appropriate boxes  
(applies to worst recorded event)

##### 8. Environmental Damage

Place tick in appropriate boxes  
(Existing or anticipated problems)

#### Attributes

a	Area is a high hazard floodway, (defined by the Floodplain Management Manual)	1	-
b	Little warning time, (less than 2 days)	1	1
c	Rapid water level rise, (more than 0.1m per hour)	1	1
d	Typical depth above floor greater than 0.1m	1	1
e	Typical depth above floor greater than 0.5m	1	-

a	Business area closes down affecting long term viability	1	-
b	Community infrastructure affected (Hospitals, Schools etc.)	1	-
c	Community isolated in major floods	1	1
d	Essential infrastructure at risk of failure (electricity, water, sewerage etc.)	1	-
e	Long term isolation / long duration flooding in the town area (greater than 1 day)	1	-

a	1 - 4 dwellings affected	1	-
b	5 - 9 dwellings affected	2	-
c	10 - 14 dwellings affected	3	-
d	15 - 19 dwellings affected	4	-
e	Greater than 20 dwellings affected	5	5

a	Reports/records show 9% or more dwellings affected	5	-
b	Reports/records show 6 - 9% or more dwellings affected	4	4
c	Reports/records show 4 - 6% or more dwellings affected	3	-
d	Reports/records show 2 - 4% or more dwellings affected	2	-
e	Reports/records show 0 - 2% or more dwellings affected	1	-

a	Reports/records show at least 1 incidence of flooding	1	-
b	Reports/records show at least 2 incidences of flooding	2	-
c	Reports/records show at least 3 incidences of flooding	3	-
d	Reports/records show at least 4 incidences of flooding	4	-
e	Reports/records show 5 or more incidences of flooding	5	5

a	Evacuation to centres outside the community required (adajcent suburbs etc.)	1	-
b	Urgent evacuation due to quickly rising water levels and associated danger to personal safety	1	1
c	Evacuation leaving no time for damage reduction	1	1
d	Evacuation from entire area required (localised group of suburbs)	1	-
e	External evacuation assistance required due to lack of overland evacuation route (SES etc.)	1	-

**DECC / DIPNR Sub-Total**

**20**

a	Structural damage mainly to houses (house undermining, extensive structural damage etc.)	1	-
b	Non-structural damage mainly to houses (hose out house, replace carpets etc.)	1	1
c	Flooding of yards, sheds, garages, pools, and or downstairs areas	1	1
d	Minor property damages	1	1

a	Potential damage to designated environmentally sensitive areas (SEPP Wetlands etc.)	1	-
b	Major erosion and siltation problems causing an increase in flood levels and /or loss of waterway area	1	1
c	Loss of Riparian Vegetation and Fauna Habitat associated with erosion of banks or bed of creek	1	-

<b>9. Maintenance Issues</b>
Place tick in appropriate boxes (Existing or anticipated problems)

<b>10. Development</b>
Place tick in appropriate boxes

a	Tendency to require regular maintenance (blocked pits/pipes, vegetation in open drains)	1
b	Old pipelines in area (Possibility of cracking, mis-alignment, or requiring replacement)	1

1
1

a	Detailed investigation complete	1
b	Design Complete	1
c	Approval Complete (owner, DLWC, Fisheries, DA, etc.)	1
d	Environmental assessment complete	1
e	Management plan complete	1
f	Community involvement in project	1

n/a
n/a
n/a
n/a
n/a
n/a

<b>GCC Sub-Total</b>		
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n/a
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**TOTAL**

n/a
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