

GOSFORD CITY COUNCIL

**WOY WOY, UMINA,
ETTALONG PENINSULA
DRAINAGE STRATEGY STUDY**

JUNE, 1992



WEBB, McKEOWN & ASSOCIATES PTY. LTD.
CONSULTING ENGINEERS

GOSFORD CITY COUNCIL

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EXECUTIVE SUMMARY

Development of the Woy Woy, Umina, Ettalong Peninsula over the past 50 years has resulted in a stormwater drainage system which is inadequate by today's standards. It does not meet the expectations of the local community and it does not satisfy Council's current design criteria for stormwater drainage.

This study was commissioned by Council to assist it in planning possible future trunk drainage works and in the preparation of a Development Control Plan (DCP). As part of the study, proposed works were costed to enable Council to estimate Section 94 drainage contributions for areas zoned Residential 2(b) (medium density).

The study determined that the existing pipe drainage system had capacities ranging from zero (where no drainage lines were in place) to the 1 in 100 Annual Exceedance Probability (AEP) flood. The range reflects the fact that much of the existing system has probably been installed on a needs basis as development progressed. The increased development, including extension of existing dwellings and an increased housing density, would have contributed to increased flows and reduced the effectiveness of the existing system. Coupled with the increasing problems, community expectations in this regard are higher than they were 20 to 30 years ago when the area was first developed. Council's resources to keep up with these changes have been limited in real dollars over the same time period.

The study determined the catchments of the existing drainage system for existing levels of development, as well as future drainage catchments with drainage reserves and easements. The future conditions allowed for the increase in runoff and higher drainage standards applicable for redevelopment of large portions of the peninsular to medium density - residential zoning 2(b).

The pipes and culverts required to augment the existing drainage system to meet contemporary design standards were estimated for different scenarios and these were costed to provide an estimate for setting Section 94 contributions.

Costs were calculated for pipe and channel works, culverts and bridges, services relocation and minor drainage works. Excluded from the estimates were land acquisition costs for drainage reserves and compensation payments for easements.

The cost to upgrade all the current trunk drainage schemes for existing development conditions is \$14.2 million. The cost to upgrade and modify all existing schemes to suit proposed future drainage systems catering for ultimate future development is \$29.7 million.

The difference in cost of the two schemes (existing and future) was used in estimating Section 94 contributions for trunk drainage. The rate was calculated to be \$17 720/ha. It should be noted that this rate does not take into account costs for drainage reserves and compensation for easements which can only be estimated at the detailed design stage.

The cost to provide minor drainage works for Residential 2(a) is \$17 000/ha. The cost to provide minor drainage works for Residential 2(b) is \$30 000. The difference in the two zonings was used to estimate Section 94 contributions for minor drainage works. The rate is \$13 000/ha.

The estimated Section 94 drainage contribution for both trunk and minor drainage works is thus \$30 720/ha.

1. INTRODUCTION

In March, 1990 Gosford City Council commissioned Webb, McKeown & Associates to investigate trunk stormwater drainage options for the Woy, Umina, Ettalong Peninsula (Figure 1).

A number of floods in recent years have highlighted the problems of the existing drainage system. In addition current redevelopment of the Peninsula is increasing the housing density which is exacerbating the present drainage problems.

The objectives of the study were:

Data Collection

- collect survey data,
- review existing drainage studies.

Drainage Strategies and Design Criteria

- investigate the available secondary drainage strategies to reduce the load on the trunk drainage system,
- propose and justify a design standard.

Existing Drainage System

- define the boundaries of the existing catchment areas, each with its own outfall to Brisbane Water or Broken Bay,
- establish hydrologic and hydraulic models to analyse each existing stormwater drainage system under the existing (mainly low density) level of development,
- identify the sections of each drainage system that are under capacity,
- determine the augmentation works required to ensure each existing system can cater for:
 - a) the 1 in 10 AEP peak flow where secondary flow paths are available for larger floods or where nuisance flooding is minimal,

- b) the 1 in 100 AEP peak flows elsewhere,
- estimate the cost of these works.

Future Drainage System

- define efficient drainage flow paths for a future stormwater drainage system and adjust the existing catchment area boundaries accordingly,
- establish hydrologic and hydraulic models to analyse the drainage system in each of the major catchments with the peninsula re-developed for (medium density) ultimate future development,
- determine the works necessary to enable each system to cater for the 1 in 100 AEP peak flows, by modifying and amplifying the existing drainage systems,
- investigate the backwater influence of high downstream water levels on the flow capacity of selected drainage systems,
- specify necessary drainage easement and reserve widths,
- estimate the cost of the future drainage works.

Section 94 Contributions

- for areas zoned 2(b), estimate the Section 94 drainage contribution rate per unit area.

2. STUDY AREA

2.1 General

The study area (Figures 1 and 2) lies within the City of Gosford and is approximately 10km south of the Gosford CBD. It is bounded to the north and east by Brisbane Water, to the south by Broken Bay and to the west by Brisbane Water National Park.

2.2 Topography

The topography of the catchment consists mainly of a series of low lying sand dunes except for that portion backing onto the National Park and Blackwall Mountain. In the sand dunes ground levels are generally between 1m and 6m above Australian Height Datum (AHD - approximately mean sea level). The developed portion of the catchment backing onto the National Park rises to approximately 80m AHD. The catchments within the National Park rise to 176m AHD.

2.3 Geology

The ground generally above 6m AHD consists mainly of Hawkesbury Sandstone. The major portion of the study area, which lies between 1m AHD and 6m AHD, is located on sand dunes. The total depth of sand is unknown, but it exceeds 3m.

2.4 Brisbane Water Levels

Water levels in Brisbane Water were examined in some detail by the Public Works Department (PWD) (1988) which found that the main factors influencing the water level were tidal range, super-elevation, ocean wave setup, winds and barometric pressure. The estimated 1 in 100 annual exceedance probability (AEP) water level was estimated to be 2m AHD.

Gosford City Council in its Specification for Design of Stormwater Drainage Works (January 1990) recommends the following levels in Brisbane Water and Broken Bay for various recurrence interval storms.

TABLE 1
BRISBANE WATER LEVELS (m) AHD

Outlet Location	1,2,5,10	Design Average Recurrence Interval					
		20		50		100	
		All Discharges		Design Discharge (m ³ /s)			
		<3	>3	<3	>3	<3	>3
Brisbane Water and Tributaries (upstream of the Rip)	0.35	0.35	0.70	0.70	1.00	1.00	1.50
Brisbane Water (between Half Tide Rocks and The Rip)	0.45	0.45	0.80	0.80	1.20	1.20	1.60
Pacific Ocean/Broken Bay	0.50	0.50	0.80	0.80	1.20	1.20	1.60

2.5 Development Status

The current development (land usage) of the study area consists mainly of single, free standing residential dwellings. Most of the older dwellings are built with either fibro or weatherboard cladding and have elevated floors set on brick piers. Newer residences are typically brick veneer with slab on ground construction.

In recent times redevelopment has taken place by changing lots with single older dwellings into lots with villas and townhouses. The rate of redevelopment has increased with the change in dual occupancy rules introduced by the State Government. The impact on the stormwater drainage regime has been to increase surface runoff. In physical terms, redevelopment has meant extra paving and roofing which has decreased the area which allows direct infiltration into the sand beds and thereby increase the amount flowing overland as surface runoff.

3. EXISTING DRAINAGE PROBLEMS AND FUTURE STRATEGIES

3.1 Existing Drainage Problems

The subdivision of the Peninsula, which took place mainly in the early 1900's, was based on a rectangular road grid pattern. The pattern paid little or no regard to topographic contours and secondary drainage flow paths. This did not cause obvious problems at the time as most of the roof water was collected in water tanks for domestic use. Runoff from road and yard areas drained to low points in the dune system from where it either gradually seeped away into the groundwater or evaporated. Some of these low points were located in backyards, however the nuisance flooding this caused was not considered a serious problem as houses tended to be elevated above the ponding level.

The volume of ponding at low points has increased with:

- the advent of a piped water supply which has removed the need for collecting roof water,
- more intensive development,
- paving of roads, driveways and portions of backyards,
- increased dwelling size reflecting improved living standards,
- filling in of natural depressions,
- decrease in soil infiltration rate with the creation and importation of silty topsoil material.

A large proportion of the existing drainage network was designed, built, and where necessary extended, on a needs basis. Designs of the original outfalls were probably based on "rule of thumb" or empirical formulae. Even progression to more modern methods of design has not necessarily provided optimal solutions, as estimates of design rainfall intensities have tended to increase with time as more data are collected.

3.2 Possible Drainage Strategies

The existing drainage problems, the typical geology and topography of the area, and the future redevelopment potential of the Peninsula, increase the priority for planning of cost effective trunk drainage solutions.

The following options were considered in order to reduce the load on the trunk drainage system (the disadvantages are highlighted below each option):

- a) Piping of roof runoff direct to groundwater via sump (absorption) pits (see Figure 6 (Reference 1)).
 - requires large numbers, regular maintenance and strict control.
- b) Porous road pavements and parking areas (see Figure 7 (Ref. 2)).
 - reduced pavement strength, increased maintenance.
- c) Groundwater drainage systems (see Figure 8).
 - costly and requires work in private property.
- d) Surface collection facilities at local low points in yards and roads (see Figure 9).
 - costly and requires work in private property.

Consideration was given to using the deep underlying sand layer, which covers approximately 90 percent of the local catchment, to supplement the overland stormwater drainage paths. The highly pervious nature of the sand, and its significant storage capacity above the groundwater table, suggested that this could be utilised as a natural detention basin to mitigate peak discharges, and thus reduce the size and cost of surface drainage works.

However, observations during periods of continuous rainfall showed that the groundwater in low lying areas rose above ground level leading to ponding at low points for a number of days. Directing more runoff to the groundwater would exacerbate this problem.

4. DATA COLLECTION

A set of plans detailing the existing stormwater drainage layout was available from Council. Although not complete, it included over 95 percent of the existing pipes and open channels. Physical features, such as pipe diameter, were available for most of the trunk drainage part of the system. A survey of the remaining trunk drainage pipes and open channels was carried out as part of this study to complete the database.

A number of drainage studies have been carried out for limited sections of the catchment by Consultants and Gosford City Council. These studies were re-evaluated so as to take into account the present and future drainage needs of the whole Peninsula.

Ground level data were extracted from the sewer design plans prepared by the PWD in the early to mid 1980's. Levels were on average approximately 50 metres apart, and they were used in conjunction with general contours from 1:2000 scale orthophotomaps to locate drainage problem areas and estimate the general slope of the land. These maps were also used to estimate the existing, and likely future proportions of paved and unpaved area in each sub-catchment.

As part of this study Peter Bolan & Associates (Registered Surveyors), were commissioned to carry out a detailed survey of an open channel traversing Ryans Rd, Veron Rd, Dulkara Rd, Nambucca Drive to the Railway Line (see Appendix D). These survey data were used for hydraulic grade line calculations.

Areas known to be subject to flooding from local ponding of stormwater drainage were obtained from Council's records. The nature of the flooding or ponding problems at each location was clarified in discussion with Council Officers.

5. DESIGN CRITERIA

5.1 General

The drainage criteria set out in Council's Stormwater Drainage Works Specification were considered in selecting an appropriate recurrence interval, together with the recommendations of Australian Rainfall & Runoff (Reference 3). Council's criteria are set out in Table 3.

It was determined, in conjunction with Council's Officers, that the drainage criteria would need to be flexible to meet the expectations of the community and budget limitations of Council.

For areas where housing and population densities were expected to remain low, a limited amount of nuisance flooding should be acceptable, i.e., temporary storage of floodwaters in backyards, front yards and road areas.

For areas where housing densities were to be increased, nuisance flooding would affect more people and therefore would be less acceptable by the community. A higher trunk drainage standard was considered for such areas.

A trunk drainage design criteria of either 1 in 10 AEP or 1 in 100 AEP was adopted, depending on the actual situation in the field (i.e, level of development, local topography, drainage escape paths, etc.).

The large number of localised depressions meant that it was not practical to provide overland flow paths for large storm events. Retarding basins did not provide a practical alternative because of the lack of suitable land. Therefore, to keep flooding to an acceptable level, a set of design standards were proposed and these are listed in Section 5.2 below.

5.2 Proposed Design Standards

Existing Catchment Areas

- a) 1 in 10 AEP trunk drainage standard where either:
 - (i) housing and population densities are low,
 - (ii) secondary flow paths are available, or
 - (iii) nuisance flooding is low.
- b) 1 in 100 AEP trunk drainage standard where either:
 - (i) housing density is medium or higher, or
 - (ii) no secondary flow paths are available.
- c) All new development to have floor levels at least 500mm above road level. This will allow vertical realignment of local roads in the future, which would in turn progressively eliminate isolated low points. Where depressions are greater than 500mm in depth, then floor levels should be at least 300mm above the edge of the depression,
- d) Where (c) is not feasible, Council to acquire land for drainage reserves in order to create secondary flow paths.

Future Catchment Areas with Medium and High Density Development

- (a) 1 in 10 AEP trunk drainage standard where secondary flow paths are available.
- (b) 1 in 100 AEP trunk drainage standard where no secondary flow paths are available.
- c) All new development to have floor levels at least 500mm above road level. This will allow vertical realignment of local roads in the future, which would in turn progressively eliminate isolated low points. Where depressions are greater than 500mm in depth, then floor levels should be at least 300mm above the edge of the depression.
- d) Where (c) is not feasible, Council to acquire land for drainage reserves in order to create secondary flow paths.

5.3 Proposed Design Philosophy for Future Works

- pipe drainage to be used only when velocities would be greater than 0.6m/s for self cleansing purposes,
- open drains to be used where velocities are lower as they normally would have lower hydraulic losses and are easier to maintain. Option of:
 - i) grass lined channels to increase infiltration and for aesthetic purposes, or
 - ii) concrete lined channels to reduce land take and reduce maintenance requirements,
- drainage lines to be as short and direct as possible to Brisbane Water/Broken Bay,
- existing and/or future culverts and bridges to be sized to provide a flow area of at least the same as the upstream pipe or channel.

TABLE 3
GOSFORD COUNCIL CRITERIA FOR SELECTING RECURRENCE
INTERVALS

Drainage Situation	Design Flood Average Recurrence Interval
1 Residential streets with overflow or bypass along the street.	10 years
2 Residential streets at low points with overflow along public reserves and pathways.	10 years
3 Residential streets at low points with drainage lines traversing building allotments or other locations where surface flow may cause property damage.	20 years
4 Major system traversing developed areas (residential, commercial or industrial).	50 years
(Major systems are defined as those having catchment areas in excess of 15ha or having 1 in 50 AEP runoff in excess of 3m ³ /sec.	
5 Industrial and neighbourhood business areas.	20 years
6 Larger business areas.	50 years
7 In any specific case where damage to property or unusual inconvenience is likely to result from surcharging of the drainage system, the City Engineer may require a longer average recurrence interval to be used than those given above.	

6. ILSAX MODEL

6.1 Choice of Model

To properly simulate the rainfall-runoff process on an urban catchment, an urban drainage model must be capable of developing runoff hydrographs at each entry point to a pipe or open channel system, and then route and combine the hydrographs through the drainage network.

The computer based model ILSAX meets these requirements. It contains a comprehensive hydrologic model for calculating flows, and an approximate procedure for pipe sizing. It can be used to analyse an existing drainage system, upgrade an existing system, or design and cost a new system.

6.2 Hydraulic Calculations

Hydraulic calculations in ILSAX are simplified. The model assumes that the pipes flow full but not under pressure. It therefore cannot do hydraulic grade line calculations, and can only estimate the correct pipe size to match the peak calculated flow at pipe capacity. Pipe capacity is calculated in the model using the Colebrook-White friction relationship assuming that the energy line slope is the same as the pipe slope. Upstream or downstream controls cannot be simulated.

In this study ILSAX was used in design mode, in which it estimates the peak flow and required pipe size for each pipe reach. These data were used to determine which sections of existing pipe or open channel were under capacity, and what augmentation works were required.

The following upgrade options were considered in analysing both the existing and future systems:

- (i) Additional Pipes - calculated by determining the pipe diameter required to produce a combined flow area at least the same as the pipe size recommended by ILSAX.
- (ii) New Pipes - as recommended by ILSAX.
- (iii) New Open Channel - calculated by using Manning's equation to determine the open channel size required to take the flow rate calculated in ILSAX.

ILSAX results were checked for three selected catchments using the Hydraulic Grade Line method (see Section 9.4).

6.3 Pipe System Definition

The ILSAX pipe system is made up of:

- overland flow entry points representing pits, called *nodes*,
- pipe lengths between nodes, called *reaches*.

A number of consecutive reaches is called a branch. A pipe system is made up of one or more pipes, and is defined by nominating the nodes where two or more branches join to form one branch.

6.4 Catchment Definition

Each sub-catchment boundary has to be defined so that the contributing catchment area can be measured. In ILSAX, each pipe reach has a sub-catchment which contributes surface water inflow to the upstream node on the reach. The amount of flow from each sub-catchment is influenced by area, extent of development, soil type, and overland flow travel time.

6.5 Rainfall Data

ILSAX uses standard design rainfall intensities and temporal patterns. These are taken from Australian Rainfall and Runoff (AR&R) (Reference 3).

6.6 Rainfall Losses

Losses need to be subtracted from rainfall to determine surface runoff.

In ILSAX, losses from paved areas consist of an initial loss only. Ongoing losses are considered to be zero. Losses from grassed areas employ an infiltration equation (Horton's equation), where the infiltration capacity of the soil decreases with time. The form of the equation and the values of the parameters adopted at Woy Woy are given in Table 2.

TABLE 2
Soil Infiltration Parameters

$$\text{Horton's Equation: } f = f_c + (f_o - f_c) \cdot e^{-kt}$$

Initial Rate	f_o	=	250 mm/h
Final Rate	f_c	=	25 mm/h
Shape Factor	k	=	2

7. UNIT RATES

7.1 Inclusions and Exclusions

The costings include all pipe and channel works required as well as culvert and bridge works and services relocation. Excluded items include land acquisition for drainage reserves and compensation payments for easements.

7.2 Pipework Unit Rates

The unit rates used for pipework were obtained from Council and reviewed against "Contractor Rates" for recent contracts. The Council rates were found to be consistent and realistic and were therefore adopted. The unit rates included all components of material, labour and plant required to install the pipes and complete the drainage system.

Table 4 lists the unit rates adopted for pipework as used in ILSAX.

TABLE 4
Pipework Unit Rates (\$ Dec 1990)

Pipe Size (mm)	Excavation & Backfill (\$/m)	Supply RCP (\$/m)	Lay & Joint (\$/m)	Pits & Fittings (\$/m)	TOTAL COST (\$/m)
375	66	31	37	8.5	143
450	75	44	40	9.0	167
525	85	57	45	9.5	196
600	96	70	51	10.0	227
675	104	89	53	11.0	257
750	112	109	57	12.0	289
825	119	134	60	13.0	326
900	125	164	62	15.0	367
1050	144	213	68	17.5	443
1200	163	268	76	20.0	527
1350	179	327	84	22.0	612
1500	194	398	92	24.0	708
1650	215	471	97	26.0	809
1800	235	550	105	28.0	918
1950	255	645	114	31.0	1045
2100	274	748	126	34.0	1181

7.3 Open Channel Unit Rates

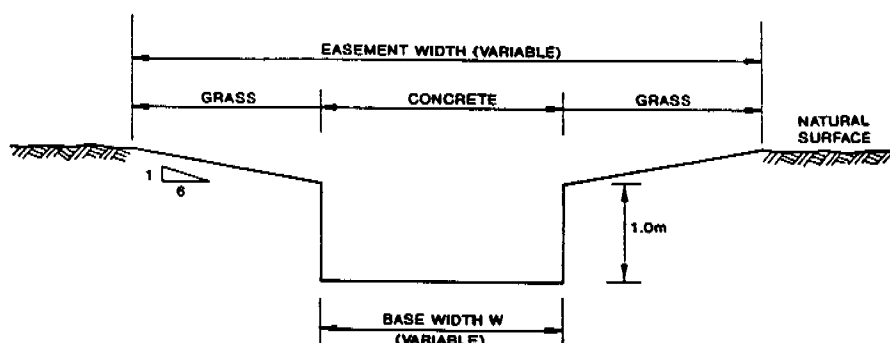
The unit rates for open channels depend on the type and size of channel. Council will need to use different types of open channels to suit different situations. In this study the channel was assumed to have a concrete base and concrete block sides, as shown in Figure 5. This configuration was adopted throughout for the purpose of estimating costs and easement widths. It should not necessarily be used for design.

Table 5 lists the unit rates adopted for the assumed open channel configuration of variable width (w). These rates were used in the ILSAX analysis.

TABLE 5
Open Channel Unit Rates, (\$ Dec 1990)

Item	Rate	Quantity/m	\$/m
mass concrete	\$15/m ²	w + 0.8	15w + 12.00
slab, with mesh	\$55/m ²	w + 0.4	55w + 22.00
reinforced blockwork	\$75/m ²	2.0	150.00
excavation & dewatering	\$26/m ³	2.2w + 1.2 ² x 6	57w + 225.00
trim embankment	\$1/m ²	1.2 x 6 x 2	14.40
supply & place topsoil	\$6.25/m ²	1.2 x 6 x 2	90.00
supply and place turfing	\$3.50/m ²	1.2 x 6 x 2	50.40
total			\$ (127w + 564)/m

- Notes i) Refer to Figure 5 for assumed channel configuration
 ii) Rates refer to a 1m length of channel, width = w metres.



7.4 Culvert Unit Rates

Rates for the estimation of culvert costs were based on manufacturer's cost for supply of pipes or box culverts and Council's rates for labour and plant. Road reinstatement costs were included in the estimated cost for each culvert. Culverts were assumed to be on average 18m long.

8. EXISTING DRAINAGE SYSTEM ANALYSIS

8.1 Definition of Existing Catchment Boundaries and Pipe Systems

The Peninsula has a number of stormwater outfalls into Woy Woy Channel, Brisbane Water and Broken Bay. Each outfall was labelled alphanumerically from A to AW (see Figures 3a & 3b). The extent of the catchment for each outfall was determined from the existing drainage layout and surface level data.

Pipe lengths were measured from Council's existing stormwater layout plan. Pipe slopes were not available, and had to be calculated assuming a reasonable pipe cover to the ground surface level, starting at the outlet and continuing up each of the branches.

These calculations and all ILSAX pipe and catchment data are tabulated in a spreadsheet (see Appendix A).

8.2 Augmentation Works for Sections which are Under Capacity

ILSAX was used to analyse each of the existing drainage systems relating to each of the catchment areas, A to AW, shown on Figures 3a & 3b. Details of each of the three augmentation options for each reach (refer to Section 6.2) are listed in a spreadsheet (see Appendix A).

8.3 Costings

The cost of the works for each of the three augmentation options was calculated using the unit rates. In most cases the more cost effective option was considered to be the preferred option. The costs of the preferred options were totalled for each catchment, and for the whole Peninsula. These results are listed in a spreadsheet (see Appendix A).

Costs were calculated for pipe and channel works to upgrade the system to the standards described in Section 5.2. Excluded items consisted of land acquisition for drainage reserves and compensation payments for easements.

9. FUTURE DRAINAGE SYSTEM ANALYSIS

9.1 Proposed Flow Paths and Catchment Boundaries

The lack of topographic relief over most of the Peninsula meant that the location of catchment boundaries under existing conditions tended to be somewhat arbitrary.

Future flow paths and catchment boundaries were determined taking the following factors into account:

- drainage lines should be as short and direct as possible,
- catchment areas should be of similar size to reduce the load on any one drainage system,
- existing drainage easements should be used if possible to reduce the need to resume land and create new easements,
- existing pipes and open channels that have adequate capacity (as calculated in the hydraulic analysis of the existing drainage) should be incorporated into the future drainage system wherever possible,
- potential increases in housing density should be allowed for,
- additional outfalls should be introduced where appropriate,
- existing open space at the rear of residential lots (laneways) should be utilised where possible,
- more open drains should be provided to improve hydraulic efficiency and to partially overcome the lack of grade.

Many of the small catchments that cannot properly be classified as forming part of the trunk drainage system, were not reviewed. The new catchments defined using the above factors, and their outfalls labelled A to P, are shown on Figures 4a and 4b.

9.2 Drainage Works Required

ILSAX was used to analyse the drainage systems for each of the future catchment areas A to P shown on Figures 4a & 4b. Details of the three upgrade options for each reach (refer to Section 6.2) are listed in a spreadsheet (see Appendix B).

9.3 Costings

The cost of the works for each of the three upgrade options was calculated using the unit rates. In most cases the more cost effective option was considered to be the preferred option. The costs of the preferred options were totalled for each catchment and for the whole Peninsula. These results are listed in a spreadsheet (see Appendix A). Costs were calculated for the trunk drainage works required to upgrade to the standards set in Section 5.2. As already noted, land acquisition for drainage reserves and compensation payments for easements were not included in the estimates.

The cost to upgrade all existing systems to cater for ultimate future development is \$30.8 million.

9.4 Hydraulic Grade Line Backwater Analyses

Design open channel widths for existing and future catchments (see Appendices A & B) were calculated using Manning's formula assuming normal depth along the channel. This assumption is only valid for long reaches where there are no upstream or downstream hydraulic controls. Three catchments were modelled using a full backwater analysis under future conditions in order to verify whether the channel widths calculated using the normal depth assumption were valid. The analyses are described in the following sub-sections.

9.4.1 Ryans Rd, Veron Rd, Dulkara Rd, Nambucca Drive to the Railway Line

A survey of the existing open channel was carried out in March 1991 (see Appendix D). Profiles were analysed with HEC-2 (Reference 8), a one-dimensional backwater model that uses the energy equation. 1 in 100 AEP flows calculated by ILSAX for the future development were used in the calculations. Profiles were calculated using:

- i) various Brisbane Water levels,
- ii) either the existing channel or the proposed future channel,
- iii) various combinations of existing culverts and upgraded culverts.

The results are given on Figure 10. They show that the existing culverts are the main contributor towards high flood levels. High Brisbane Water levels have only a small impact on flood levels, affecting the downstream reach.

If the culverts alone were upgraded to have at least the same flow area as the adjoining channel, then flood levels along some sections could be reduced by up to 0.8m. Upgrading the channel without upgrading the culverts would produce no significant reduction in flood levels. Upgrading the channel and all the culverts would reduce flood levels along some sections by up to 1.4m.

9.4.2 Ross St, Rowan Rd to Brisbane Water

The backwater analysis in this case was performed using a number of methods. Sections of open channel were modelled using HEC-2 (Reference 8). Pipe sections were modelled using hydraulic grade line calculations by hand (see AR&R Section 14.5.8). Culverts were modelled using Boyd's culvert program (Reference 9).

The existing channel, pipe and culvert details were taken from Centia's Report (Reference 5).

Flows calculated by ILSAX for the future development 1 in 100 AEP flood were used in the calculation of the surface water profiles. Various Brisbane Water levels were used. ILSAX demonstrated that the existing pipe system is inadequate because of the poor pipework geometry and this has therefore not been plotted. The profile calculated and shown on Figures 11a & 11b, demonstrates the performance of the channel and pipe system as calculated by the normal depth procedure. (Figure 11a considers existing flows and Figure 11b considers future flows.) The lower reaches of the system were sized for both existing catchment and future catchment areas (see Figure 3a - existing catchment Q, Figure 4a - future catchment I).

The results show that the system as proposed will function adequately with a reasonable freeboard. High Brisbane Water levels have a significant impact on levels in the lower catchment, but only a very small impact in the middle and upper catchment.

9.4.3 Bourke Road to Broken Bay

The backwater analysis in this case was performed using HEC-2 (Reference 8).

Flows calculated by ILSAX for the future development 1 in 100 AEP flood were used in the calculation of the Hydraulic Grade Line (HGL). The level in Broken Bay was taken to be 1.6m AHD. The profile calculated (see Figure 12) shows the performance of the channel widths as recommended by the normal depth procedure.

The results show that the system as designed will function adequately with reasonable freeboard. High Broken Bay water levels have no impact in the upper catchment.

9.5 Width of Drainage Reserves

The width of open channel drainage reserves required can be estimated from the open channel width calculated in the spreadsheets (Appendices A & B) by allowing for a suitable batter and an access way along one side.

In the spreadsheets, the batter width recommended on each side of the channel is the length required for a 1 in 6 batter slope to reach natural surface level from the level of the top of the channel. See Figure 5 for the assumed channel details.

9.6 Minimum Floor Levels

Minimum floor levels for all future development should be determined by providing a freeboard to allow for design uncertainties and larger floods (> 1 in 100 AEP).

In most cases an acceptable standard would be 0.5m above road level. However, some properties may be land-locked in local depressions. In such cases either the minimum floor level should be raised even further to provide adequate freeboard (at least 0.3m) above the escape level out of the depression, or the vertical road alignment should be altered to provide a flowpath for overflows.

In order to specify such levels over the whole area a detailed survey of road levels would be required. Alternatively, this matter should be considered at the time of determining new development applications.

9.7 Staging of the Works

9.7.1 General

In order to stage the works, appropriate priorities should be assigned to each problem area. It is recommended that the works providing maximum benefit be carried out first. This is sometimes determined by calculating and comparing benefit-cost ratios for each section of upgrade works. Other considerations may dictate that greater emphasis be given to social factors in ordering the priority of the works. It is also desirable as a general principle to carry out works from the downstream end up, thus ensuring no properties will be worse off in the time prior to completion of the whole system.

Within each existing catchment area there are often specific locations that act as hydraulic controls on flow capacity, typically at culverts. Solutions to these problems should be addressed first, because major improvements can often be achieved at relatively low cost.

Annual Council budget allowances have also been considered in determining works for the short to medium term upgrading of the existing system with no allowance for any increase in building density. These works are presented in Appendix C.

Council have indicated a particular interest in one of the catchments, and staging of the works for this area has been examined further in the following section. This also serves as an example of the application of the principles just discussed.

9.7.2 Ryans Rd, Veron Rd, Dulkara Rd, Nambucca Drive to the Railway Line

This catchment is referred to as catchment A. Hydraulic Grade Lines for this drainage system are discussed in Section 9.4.1 and plotted on Figure 10. The suggested priority order for the works in this catchment is set out below:

- i) if only the four upstream culverts are upgraded, then the flood level in the 1 in 100 AEP event will be reduced by up to 0.7m,
- ii) upgrading of these four culverts and the channel will produce a combined reduction in flood levels of up to 1.1m,
- iii) upgrading of all culverts, including the three downstream culverts at the railway line, and upgrading the channel, will produce a combined reduction in flood levels of up to 1.4m.

If the channel works or downstream culvert works are undertaken before the four upstream culverts are upgraded, then there will be no upstream benefit. Therefore, the four upstream culverts should be given the highest priority and undertaken first. Upgrading of the railway culvert produces a relatively small benefit and should be regarded as having low priority.

10. MINOR DRAINAGE WORKS

Minor drainage works are that part of the works necessary in each sub-catchment to collect overland runoff and transfer the flow to the trunk drains. Minor drainage works consist mainly of inlet pits and side lines.

To determine Section 94 drainage contributions for areas zoned future 2(b) a typical catchment within the study area was selected for further analyses.

Council had previously engaged a consultant to study the residential area adjacent to the Everglades Golf Course. The consultant determined the minor drainage works for all the sub-catchments within the Everglades Catchment for the existing level of development and to Council's current design standard.

A 3 hectare sub-catchment was selected for further analyses. Minor trunk drainage was determined for the catchment for residential 2(b) conditions.

The cost for existing level of development and residential 2(b) level of development are \$17 000 per hectare and \$30 000 per hectare respectively. The difference in the cost is \$13 000 per hectare and this is the estimated Section 94 contribution for minor drainage for residential 2(b) zoned land.

11. SECTION 94 DRAINAGE CONTRIBUTIONS

Section 94 drainage contributions have been estimated for areas zoned 2(b). These contributions have been calculated on the basis of the pipe and channel works, culvert and bridge works and services relocation costs. The costs of land acquisition for drainage reserves and compensation payments for easements have not been included.

The costs of drainage works within the 2(b) areas alone were calculated for each of the two schemes (existing and future). The difference in cost of the two schemes, proportioned over the total area zoned 2(b), was used in setting Section 94 contributions.

The rate was calculated to be \$17 720/ha for Trunk Drainage and \$13 000/ha for Minor Drainage. (A full set of calculations is given in Appendix D.) Total Section 94 contributions for drainage is thus \$30 720/ha excluding cost of land acquisition and easements.

12. CONCLUSIONS AND RECOMMENDATIONS

The purpose of this study was to develop a trunk drainage strategy for the whole of the Woy Woy, Umina, Ettalong Peninsula area. In developing the strategy, the works required to mitigate existing drainage problems and those required to allow for further development were considered separately, to enable calculation of Section 94 drainage contributions.

Costs were calculated for pipe and channel works including culverts and bridges, and services relocation, to upgrade to the proposed standards (refer to Section 5.2). The cost to upgrade all existing trunk systems to the proposed standard for existing development is \$14.2 million. The cost to upgrade to cater for ultimate future development is \$29.7 million.

The difference in cost was used in estimating appropriate Section 94 trunk drainage contributions. The rate was calculated to be \$17 720/ha, which includes only pipe and channel works, culverts and bridges, and services relocation. Land acquisition and compensation payments for easements have been excluded.

The estimated Section 94 minor drainage contribution is \$13 000/ha. Thus the Section 94 contribution for both trunk and minor drainage works is \$30 720/ha.

For the upgrading of the existing drainage system to cater for the existing level of development, funding of the works would be limited (refer to Gosford City Council's letter dated 20 December 1991) and a medium term timeframe is envisaged before all the works recommended in Appendix C could be implemented. The various sub-catchments should be ranked in order of priority to develop a program of works taking into account works which have the greatest hydraulic and social benefits. Implementation of minimum floor levels for new buildings or existing building extensions should be immediate.

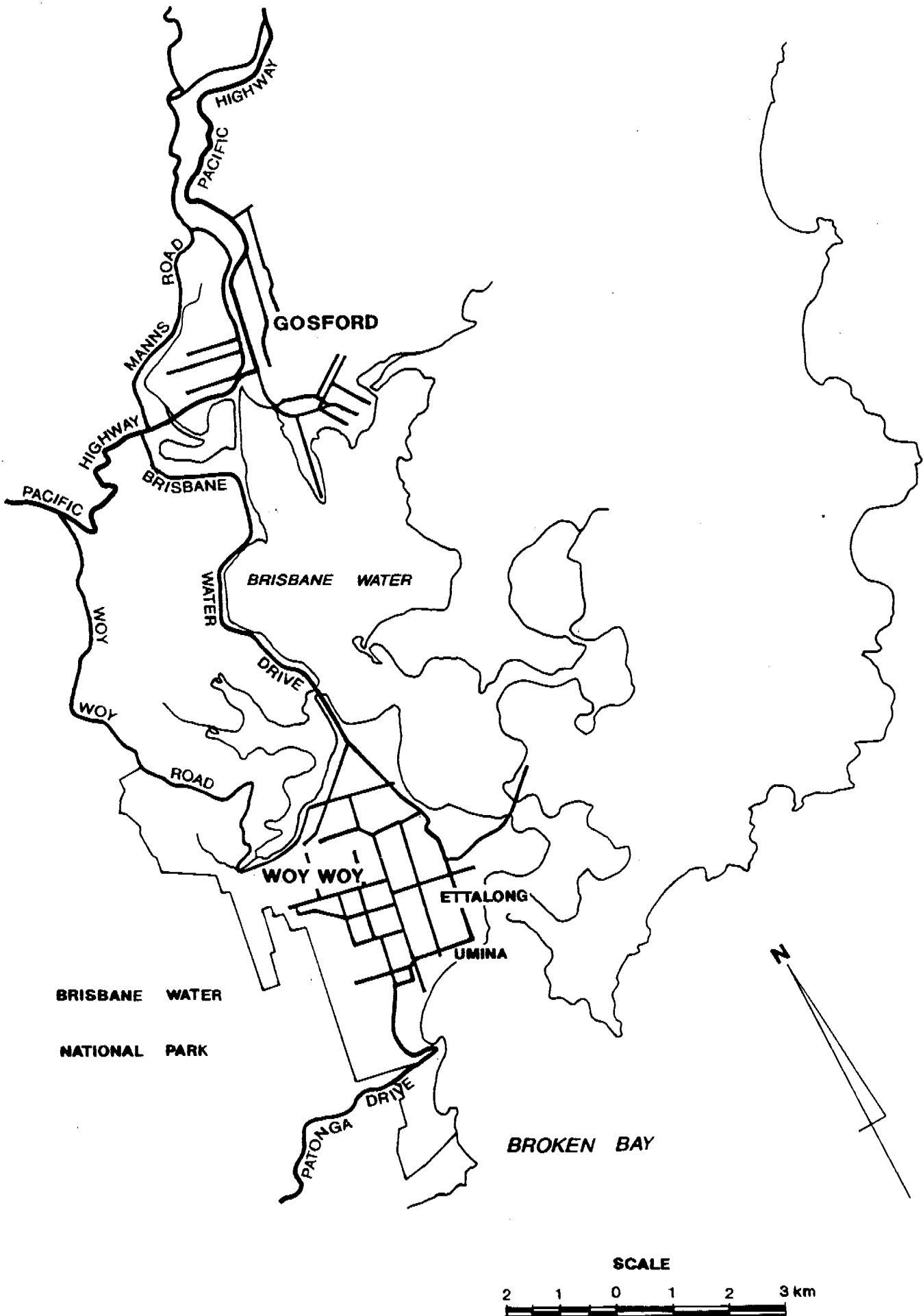
For areas to be zoned 2(b) - medium density development - the collection of Section 94 funds sufficient to construct the works would only be collected over a long period. Therefore, minimum floor levels should be incorporated in all new development to ensure flood freedom in the meantime.

13. REFERENCES

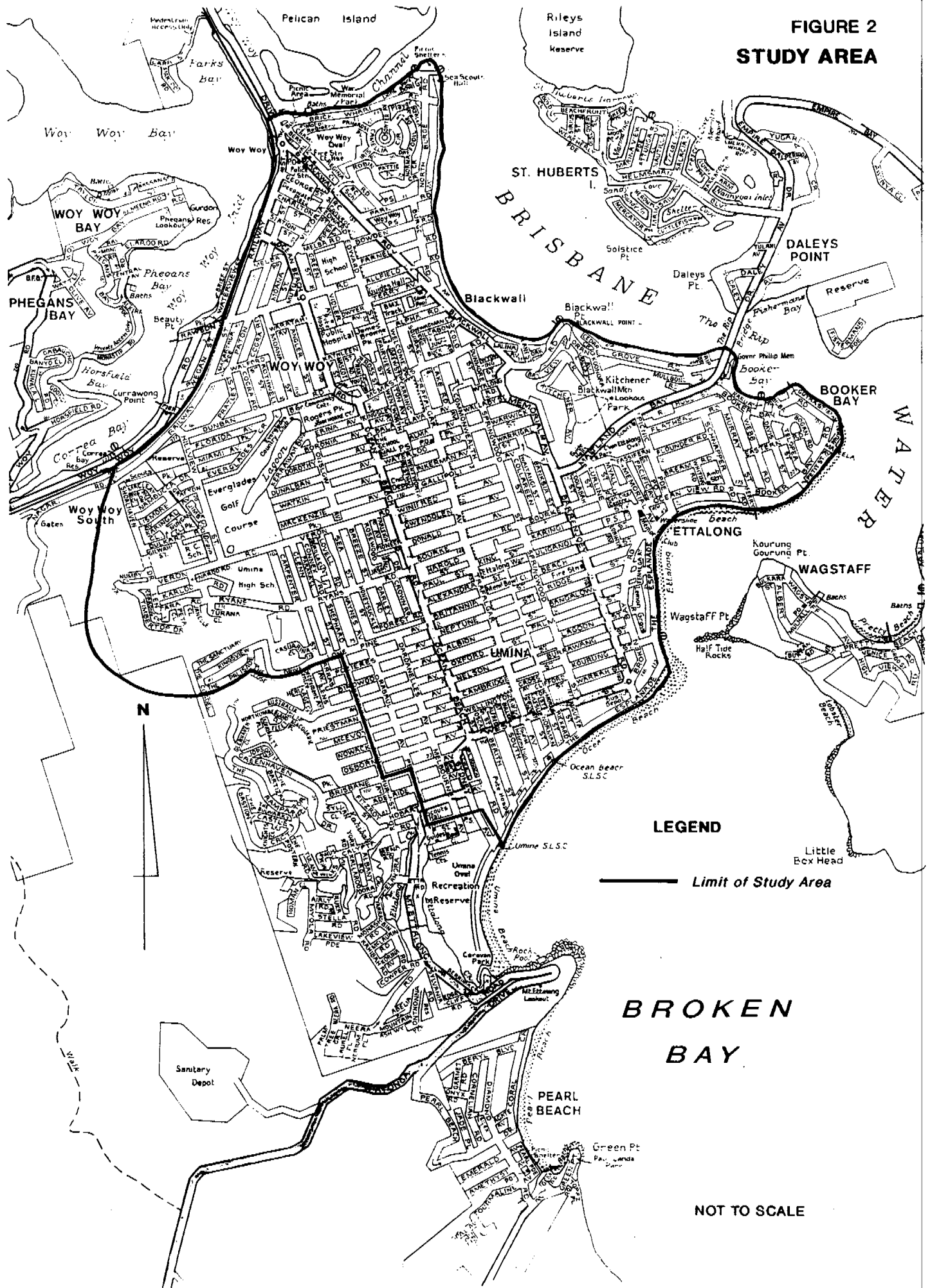
1. Somaratue, N M, et al
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University of Canterbury, Christchurch, 23-30 November, 1989.
2. **Pollution Control Manual for Urban Stormwater**
State Pollution Control Commission, 1989.
3. Australian Rainfall and Runoff
A Guide to Flood Estimation
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4. **Stormwater Investigation of Blackwall Mountain Catchment**
Giammarco Civil and Structural Engineering, April 1989.
5. **Stormwater Investigation of Ross-Rowan Catchment**
Centia
6. **Design of Everglades Lagoon Drainage System**
GHD, Newcastle
7. G O'Loughlin
The ILSAX Program for Urban Stormwater Drainage Design and Analysis
School of Civil Engineering
University of Technology, Sydney, August 1988.
8. U S Army Corps of Engineers
HEC-2 Water Surface Profiles, 1982.
9. Boyd M J
PC Programs for Culvert Hydraulics
University of Wollongong, 1988.

FIGURES

FIGURE 1
LOCALITY MAP



**FIGURE 2
STUDY AREA**



LEGEND

— Limit of Study Area

Little
Box Head

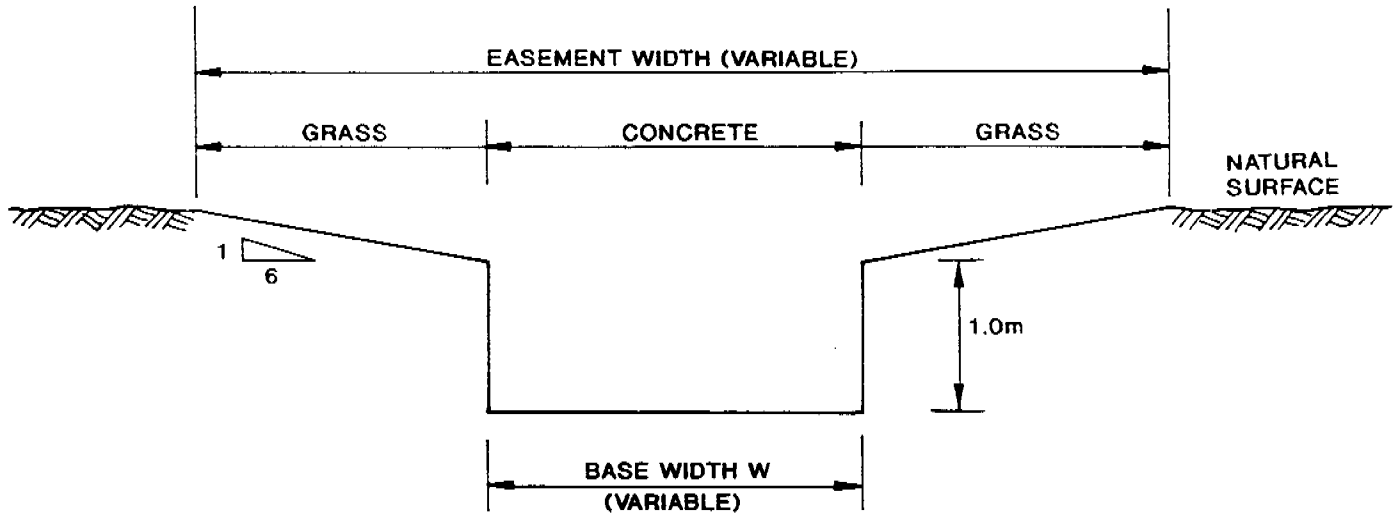
**BROKEN
BAY**

NOT TO SCALE

FIGURES 3a,3b & 4a,4b

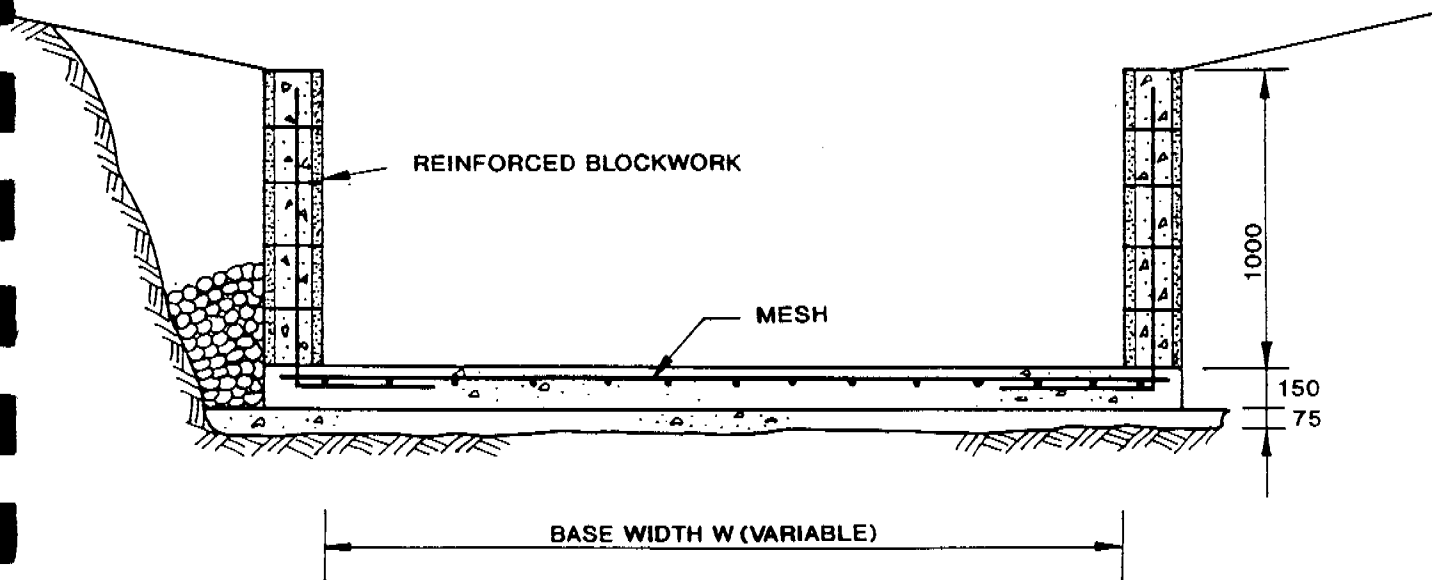
REFER ATTACHMENT A

DESIGN CHANNEL DIMENSIONS



DESIGN CHANNEL ADOPTED

1:50

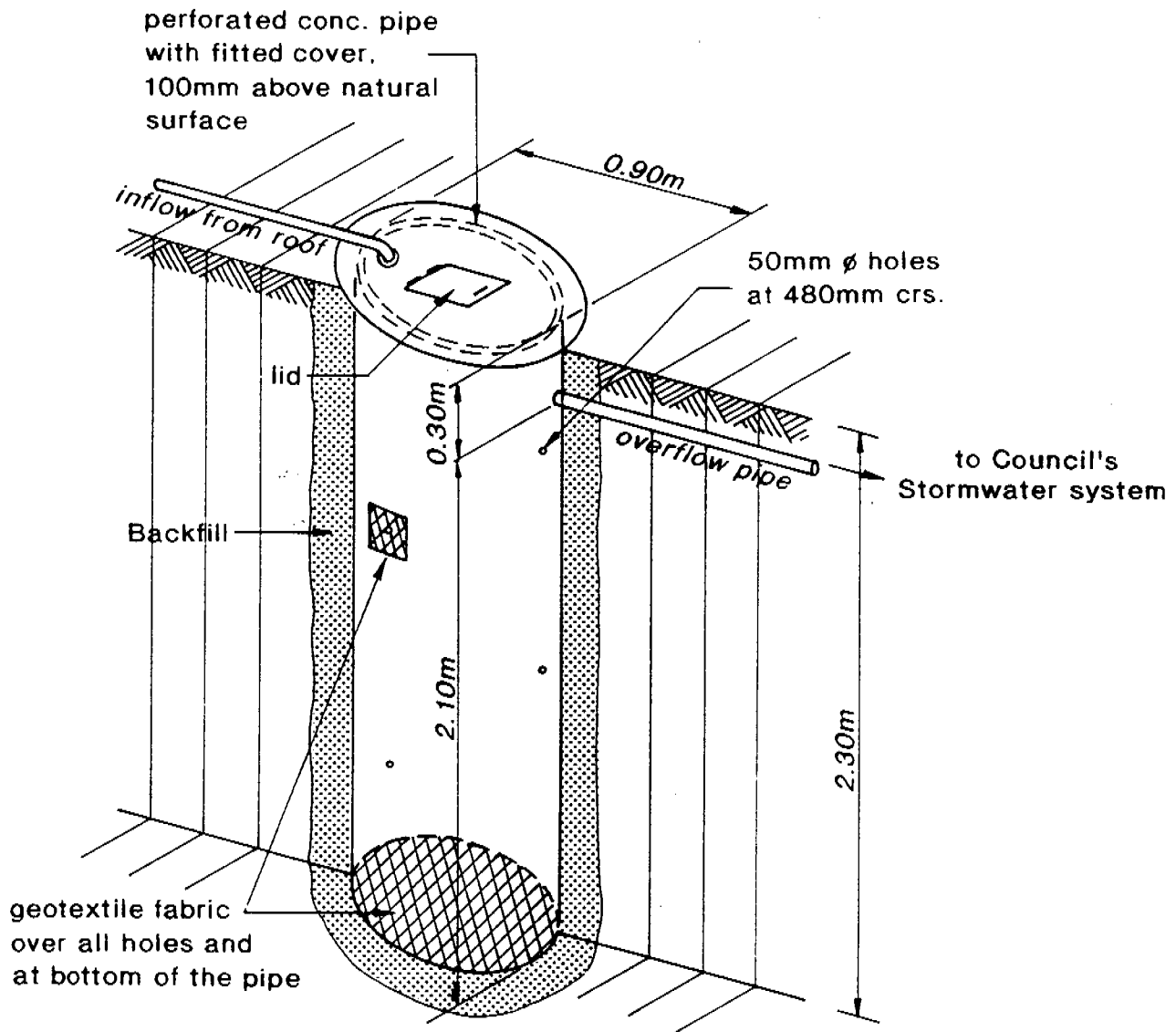


TYPICAL SECTION OF CONCRETE CHANNEL

NOTE: This section was adopted to estimate cost and easement width. Refer Section 7.3

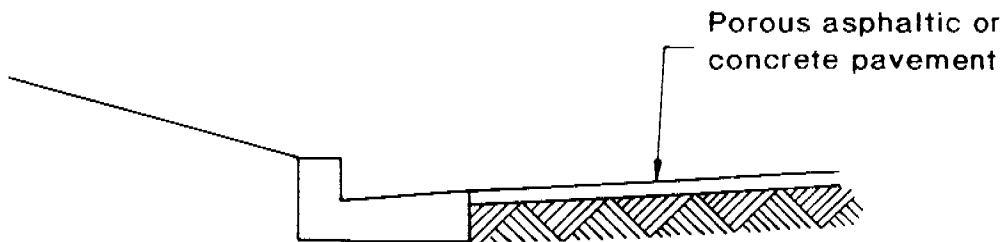
N.T.S.

ROOF RUNOFF TO GROUND-WATER VIA SUMP PITS



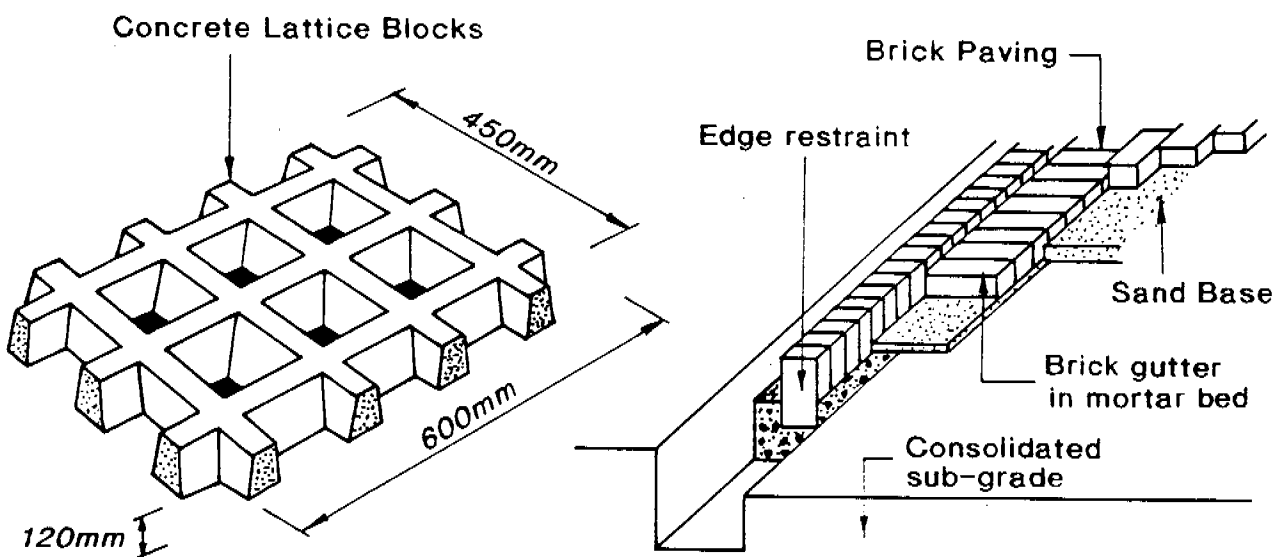
900mm diameter retention/overflow well located on private property

POROUS ROAD PAVEMENTS & POROUS PARKING AREAS



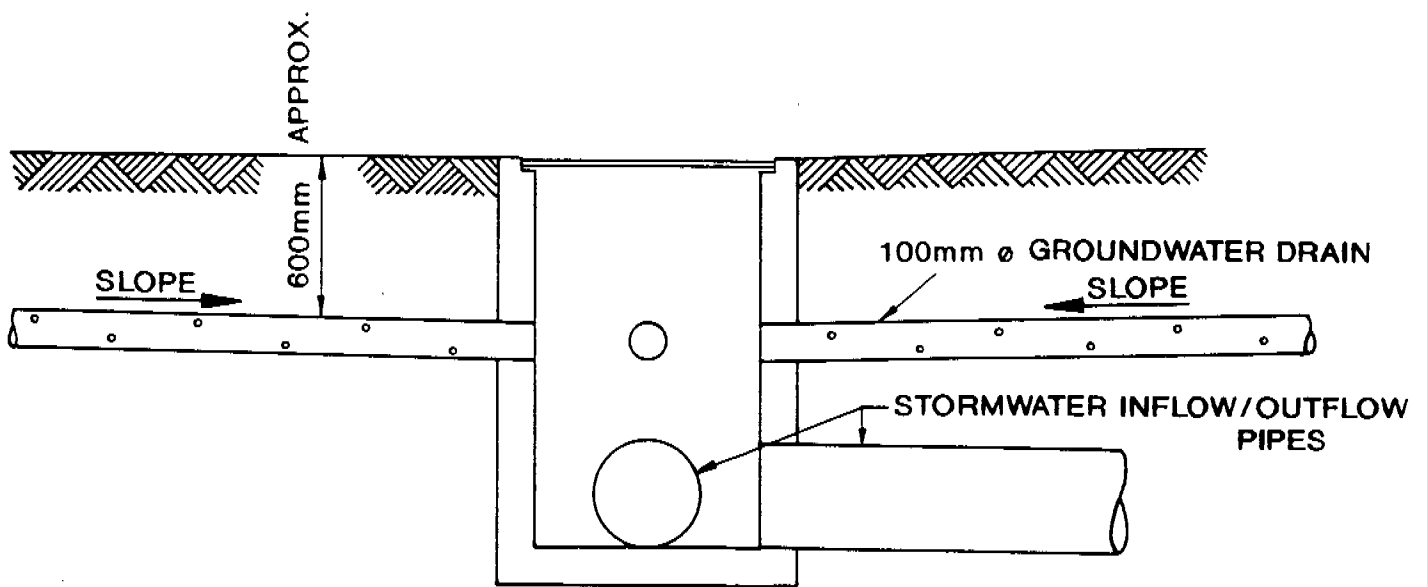
Typical road cross-section

POROUS ROAD PAVEMENT



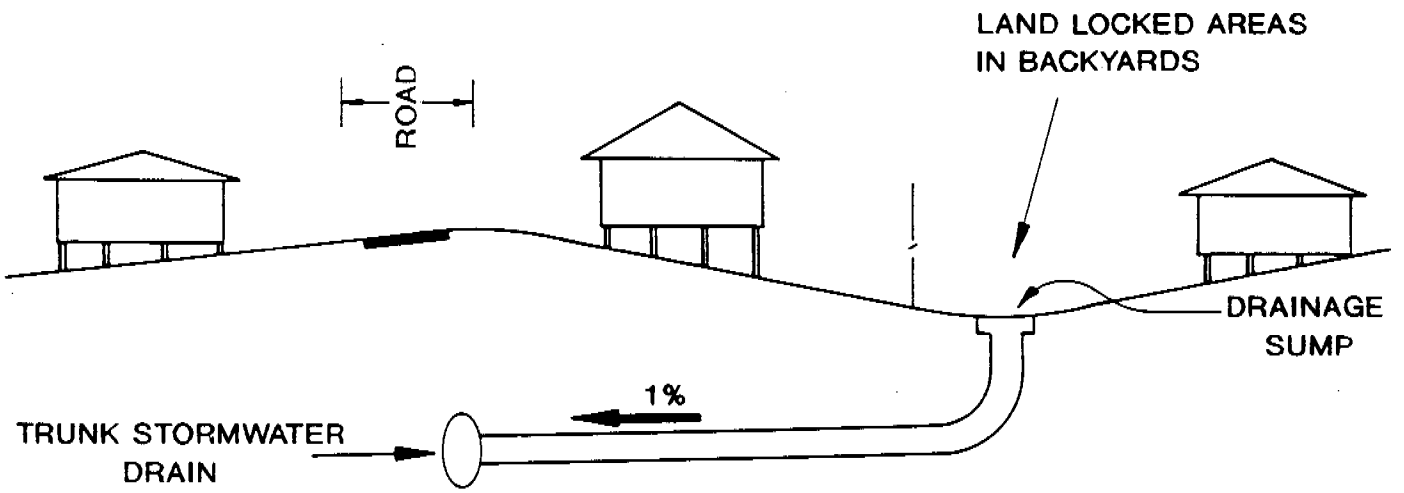
POROUS PARKING AREAS

FIGURE 8
GROUNDWATER DRAINAGE
SYSTEM



STORMWATER GULLY PIT OR JUNCTION MH.

**SURFACE RUNOFF
COLLECTION FACILITY
AT LOW POINTS**



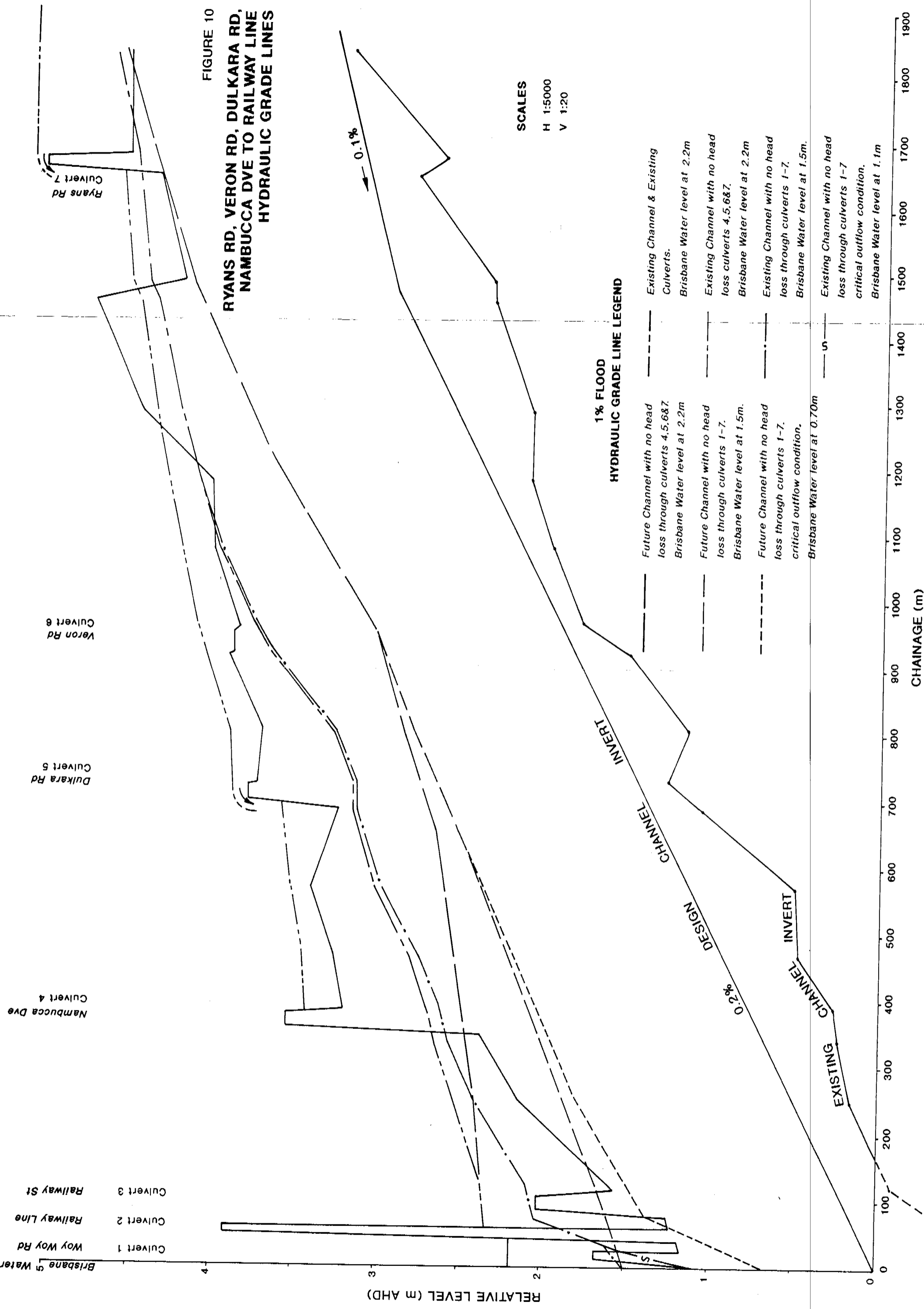
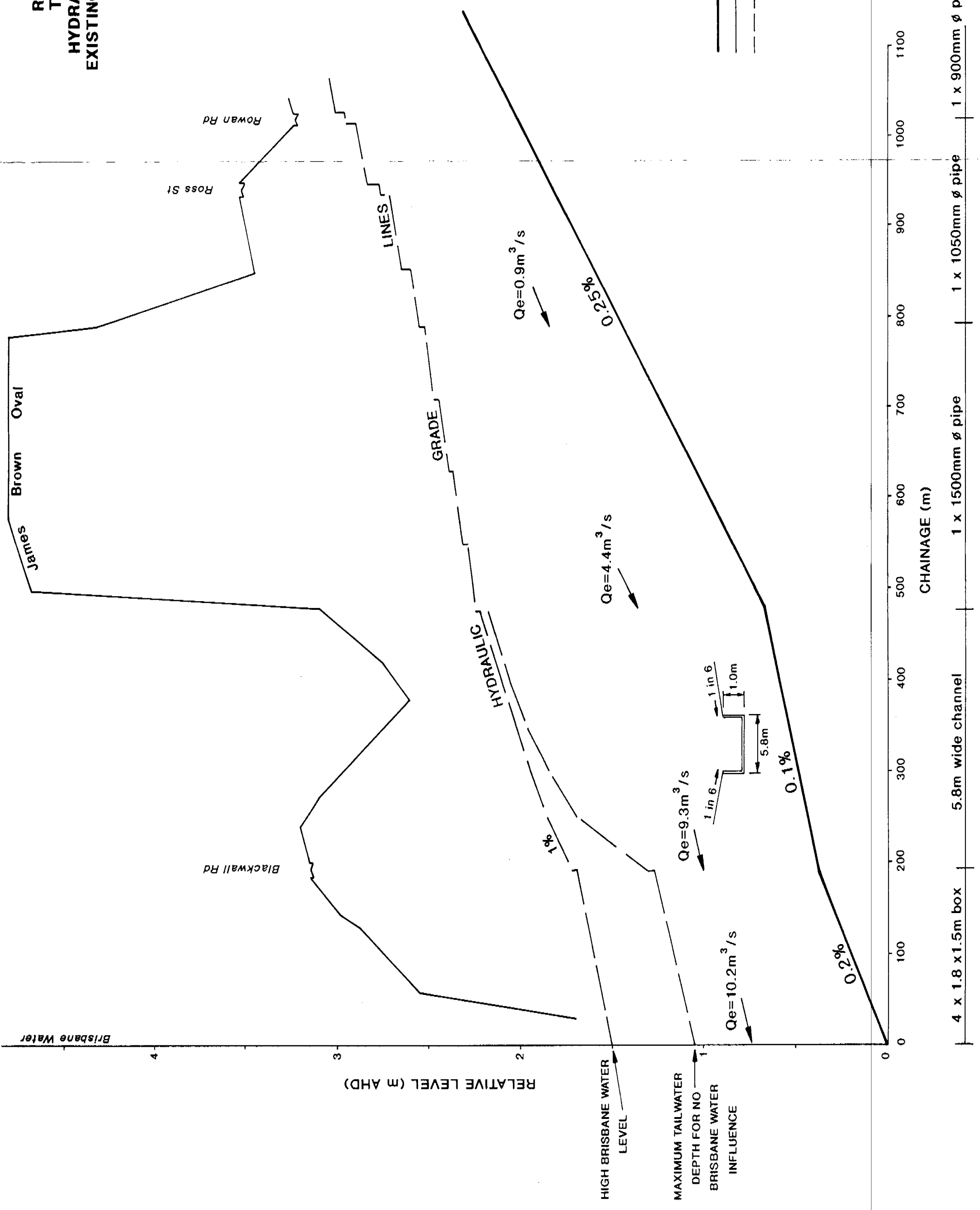


FIGURE 11(a)
ROSS ST., ROWAN RD
TO BRISBANE WATER
HYDRAULIC GRADE LINES
EXISTING CATCHMENT AREA



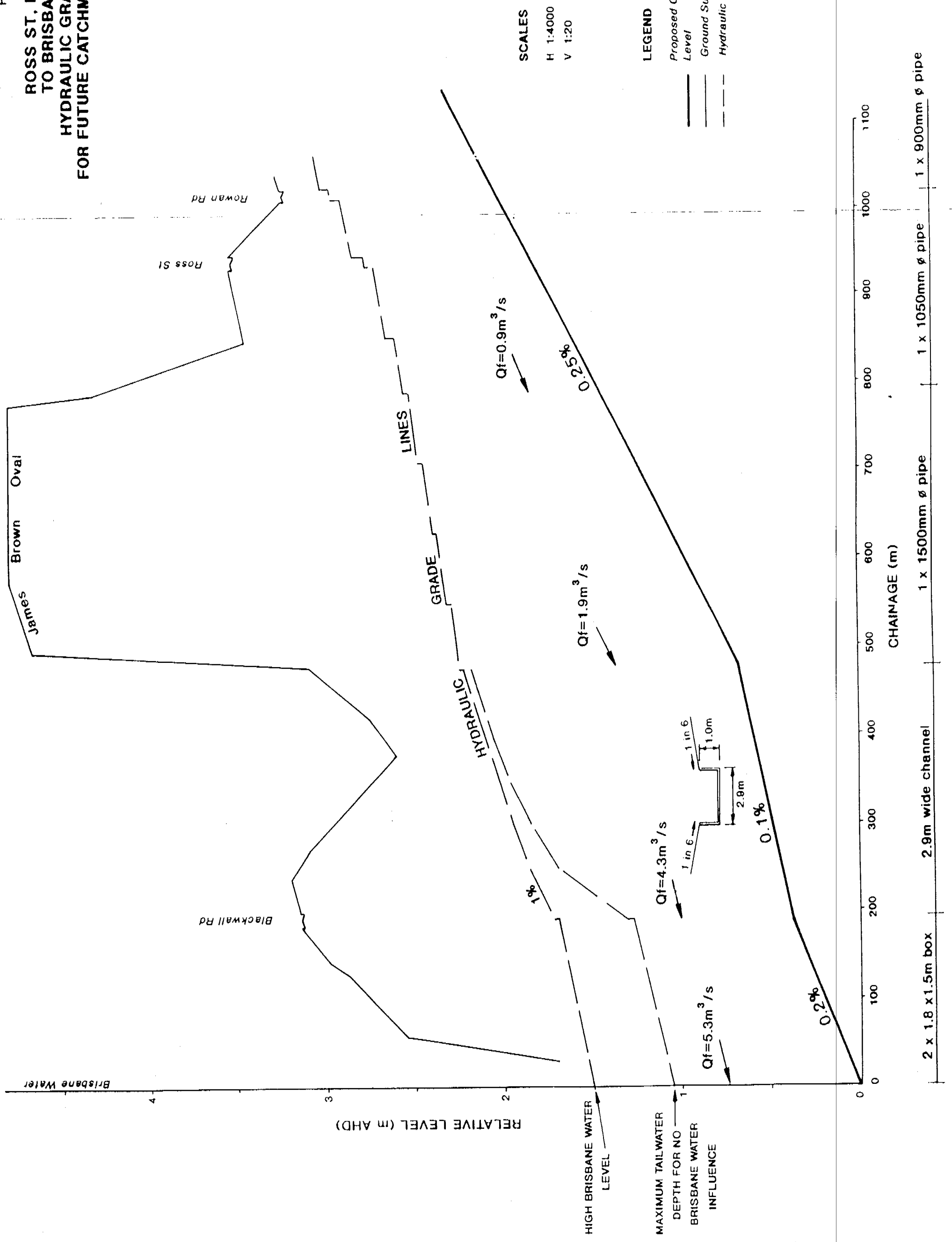
SCALES
 H 1:4000
 V 1:20

LEGEND
 Proposed Conduit Invert Level
 Ground Surface Level
 Hydraulic Grade Lines

CHAINAGE (m)
 0 100 200 300 400 500 600 700 800 900 1000 1100
 4 x 1.8 x 1.5m box
 5.8m wide channel
 1 x 1500mm ø pipe
 1 x 1050mm ø pipe
 1 x 900mm ø pipe

FIGURE 11(b)

ROSS ST., ROWAN RD
TO BRISBANE WATER
HYDRAULIC GRADE LINES
FOR FUTURE CATCHMENT AREA



SCALES
H 1:4000
V 1:20

LEGEND
Proposed Conduit Invert Level
Ground Surface Level
Hydraulic Grade Lines

CHAINAGE (m)

0 100 200 300 400 500 600 700 800 900 1000 1100

2 x 1.8 x 1.5m box

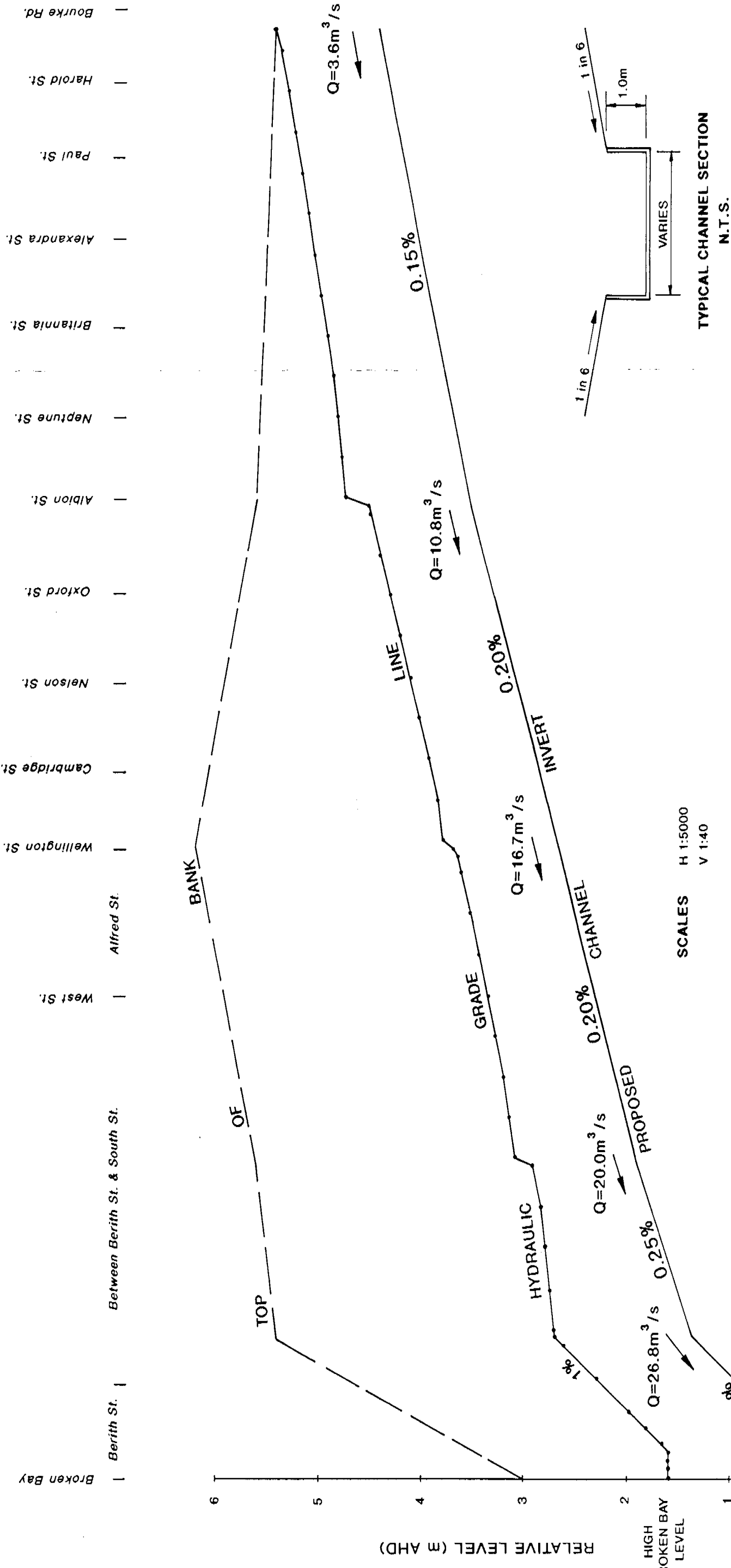
2.9m wide channel

1 x 1500mm ø pipe

1 x 1050mm ø pipe

1 x 900mm ø pipe

FIGURE 12
 BOURKE ROAD
 TO BROKEN BAY
 HYDRAULIC GRADE LINE



Broken Bay | Berith St. | Between Berith St. & South St. | West St. | Alfred St. | Wellington St. | Cambridge St. | Nelson St. | Oxford St. | Albion St. | Neptune St. | Britannia St. | Alexandra St. | Paul St. | Harold St. | Bourke Rd.

DATUM 0.0	FLOOD LEVEL	GROUND LEVEL	INVERT LEVEL	CHAINAGE (m)
1.50	2.70	3.0	0.00	0
2.70	2.70	5.5	1.88	170
2.93	2.93	4.8	1.89	380
3.64	3.64	6.2	2.67	770
4.50	4.50	5.6	3.51	1190
5.40	5.40	5.4	4.39	1780
				500
				1000
				1500

SCALES H 1:5000
 V 1:40

TYPICAL CHANNEL SECTION
 N.T.S.

APPENDIX A

APPENDIX A

ILSAX OUTPUT COLUMN DESCRIPTION

INPUT DATA

Network

1. Branch name. Each pipe branch (or arm) in each drainage system is given a alphabetic code,
2. Reach name. Each pipe reach, within a pipe branch, is given a numeric code. A pipe reach is the length of pipe downstream of a pit until the next pit,
3. At the end of a pipe branch the continuing branch name is specified so that ILSAX knows to combine the hydrographs at that location,

Conduit Details

4. Reach length (m),
5. Slope (%), assumed from existing ground levels,
6. Existing conduit size,

Catchment Details

7. Sub-catchment area (ha),
8. Directly connected paved area, as a %,
9. Paved area entry time (minutes),
10. Supplementary paved area, as a %,
11. Grassed area, as a %,
12. Grassed area travel time across the grassed surface (minutes),
13. Lag time for grassed area flow to begin to contribute to flow at the pit (minutes),

Levels

14. Invert level at the pit (mAHD), based on 5 above,
15. Ground level at the pit (mAHD),

OUTPUT DATA

Hydrology for chosen AEP

16. Critical storm duration for pipe reach (minutes),
17. Peak flowrate (m³/s),

Upgrade Option 1 - Supplement Existing with Extra Pipes

18. Number of pipes,
19. Diameter of pipes (mm),

Upgrade Option 2 - New Pipes

20. Number of pipes,
21. Diameter of pipes (mm),

Upgrade Option 3 - New Open Channels

22. Channel width (m) (see Fig.5 for channel dimensions),
23. Calculated normal depth in channel (m),

Proposed Option

24. Recommended upgrade option description,
25. Proposed easement width for open channels (see Fig.5).

Cost Breakdown

26. Cost estimate for trunk drainage works (\$K),
27. Estimate of extra cost for culverts under roads (\$K),
28. Cost estimate for services relocation (say 5%),
29. Total Cost (26 + 27 + 28) (\$K).

APPENDIX A1

**ILSAX RESULTS
1 IN 10 AEP FLOOD
EXISTING CATCHMENTS**

Network		Reach Details				Catchment Area Parameters				Levels			Hydrology 1 in 10 AEP		Proposed Upgrade Options			Proposed Upgrade Works			Cost Breakdown			Total Cost			
branch & reach	add to	L (m)	S (%)	existing conduit size (mm)	area (ha)	proved direct (%)	proved time (min)	proved exp. (%)	graze time (min)	graze lag (min)	graze level (mAFD)	invert level (mAFD)	ground level (mAFD)	critical duration (min)	flowrate (m ³ /s)	Extra Pipes no.	New Pipes dia. (mm)	Open Channel w (m)	Open Channel d (m)	Description	Easement Widths (m)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)	28	29	
AA 1	AA	335	0.20	675	13.13	30	14	10	60	55	7	3.8	5.2	20	1.4	1	825	1	1050	1.2	0.80	1 x 825mm dia pipe	109	9	6	124	
AA 2	AA	230	0.50	600	3.41	30	8	10	60	33	4	4.3	5.3	10	0.4	1	600	1	600	1.2	0.23						
AA 3	AA	185	0.20	2 x 900	10.99	30	13	10	60	52	6	3.1	5.1	25	2.9	1	1200	1	1500	1.7	1.00	1 x 1200mm dia pipe	153	15	8	176	
AA 4	AA	190	0.50	750	4.61	30	9	10	60	43	5	2.6	3.1	25	3.5	1	825	1	1500	1.9	1.00	1 x 825mm dia pipe	60	9	3	73	
AA 5	AA	130	0.20	2 x 900	5.58	30	10	10	60	40	5	2.2	4.6	25	4.4	1	1200	1	1650	2.3	1.00	2.3m wide channel	19	110	15	6	132
AA 6	AA	220	0.20	2 x 900	3.32	30	8	10	60	33	4	1.9	4.4	25	4.7	1	1350	1	1800	2.4	1.00	2.4m wide channel	20	190	18	10	219
AD 1	AD	585	0.20	600	14.58	30	14	10	60	58	7	4.3	5.5	20	1.5	1	1050	1	1200	1.2	0.84	1 x 1050mm dia pipe	259	13	14	266	
AD 2	AD	130	0.20	600	0.82	30	12	10	60	48	6	3.4	4.9	20	1.0	1	900	1	1050	1.2	0.62	1 x 900mm dia pipe	48	10	3	61	
AD 3	AD	255	0.20	1050	4.63	30	9	10	60	37	5	3.1	5.2	25	2.8	1	1200	1	1500	1.6	1.00	1 x 1200mm dia pipe	134	15	7	157	
AD 4	AD	90	1.00	1050	4.11	30	9	10	60	34	4	3.5	4.8	15	0.5	1	450	1	525	1.2	0.21						
AG 1	AG	40	0.30	750	2.97	30	8	10	60	32	4	2.8	4.2	28	0.2	1	600	1	600	1.2	0.28						
AG 2	AG	190	0.20	open channel	23.85	20	17	5	75	70	9	2.3	4.3	90	2.7	1	900	1	1500	1.7	1.00	1 x 900mm dia pipe	130	10	7	147	
AG 3	AG	205	0.20	open channel	1.99	30	7	10	60	27	3	1.9	5.0	90	5.4	1	1800	2.6	1.00	1.6m	1.00	1.6m wide channel	16	145	7	152	
AA 6	AA	490	0.18	open channel	6.58	30	11	10	60	43	5	1.5	4.3	25	10.5	3	1650	6.0	1.00	2.6m	1.00	2.6m wide channel	28	184	9	109	
AH 1	AA	310	3.80	900	7.88	30	11	10	60	46	6	12.8	14.1	15	1.1	1	600	1	600	1.2	0.23						
AI 1	AI	60	0.20	750	8.10	5	12	95	46	6	2.0	5.0	25	0.2	0.2	1	450	1	450	1.2	0.28						
AJ 1	AI	50	0.40	825	4.86	30	10	10	60	38	5	2.1	4.2	28	0.6	1	675	1.2	0.33								
AL 1	AL	180	0.15	975	5.33	30	10	10	60	39	5	3.2	4.7	25	0.7	1	900	1.2	0.53								
AL 2	AL	120	0.15	1275	4.26	30	9	10	60	36	5	2.9	4.7	20	1.1	1	1050	1.2	0.74								
AL 3	AL	110	0.15	1500	5.63	30	10	10	60	40	5	2.7	4.6	20	1.8	1	1350	1.3	1.00								
AL 4	AL	100	0.15	1950	4.06	30	9	10	60	34	4	2.5	5.3	20	2.2	1	1350	1.5	1.00								
AL 5	AL	290	0.15	1800	3.27	30	8	10	60	33	4	2.4	4.9	20	2.5	1	1500	1.6	1.00								
AI 2	AI	50	0.15	1800	3.29	30	8	10	60	33	4	2.0	4.2	25	2.8	1	1500	1.6	1.00								
AI 3	AI	40	0.50	2x1350	7.23	30	11	100	44	6	1.9	3.9	60	0.4	0.4	1	1500	1.2	0.23								
AM 1	AI	60	0.50	600	2.94	30	8	10	60	31	4	2.0	4.0	20	0.4	1	525	1.2	0.23								
AN 1	AI	130	0.27	825	5.28	30	10	10	60	39	5	2.0	4.9	15	0.6	1	750	1.2	0.38								
AI 4	AI	20	0.50	3x1350	2.31	30	7	100	28	4	1.7	3.8	90	0.9	0.9	1	750	1.2	0.38								
AI 5	AI	20	0.50	3x1350	3.22	30	8	100	33	4	1.5	3.6	60	0.5	0.5	1	750	1.2	0.38								
AI 6	AI	10	0.50	3x1350	3.14	30	8	100	32	4	1.4	3.5	90	0.5	0.5	1	750	1.2	0.27								
AI 7	AI	10	0.50	3x1350	0.70	30	6	100	18	3	1.4	3.4	90	0.5	0.5	1	750	1.2	0.27								
AI 8	AI	30	0.30	2x1050 + 825	1.42	30	6	100	24	3	1.3	3.3	3.3	90	0.5	1	1050	1.2	0.32								
AO 1	AI	30	1.09	900	3.91	15	9	85	35	4	1.6	4.3	10	0.2	0.2	1	375	1.2	0.12								
AI 9	AA	80	0.30	2x1050 + 825	1.50	30	6	10	60	24	3	1.2	4.3	90	0.5	1	1500	1.2	0.32								
AA 7	AA	250	0.10	open channel	20.94	5	17	95	66	8	1.0	4.5	25	11.1	3	1650	6.3	1.00									
AP 1	AP	120	5.00	900	20.16	30	16	10	60	65	8	11.9	23.1	15	0.8	1	525	1.2	0.17								
AO 1	AP	200	5.00	750	10.12	30	13	10	60	50	6	15.9	32.5	20	0.5	1	375	1.2	0.13								
AP 2	AP	290	1.00	1200	5.43	30	10	10	60	40	5	5.9	8.5	25	2.1	1	900	1.2	0.59								
AR 1	AP	95	5.00	1200	5.46	30	10	10	60	40	5	11.7	17.0	25	0.5	1	375	1.2	0.01								
AR 2	AP	215	1.80	1200	5.53	30	10	10	60	40	5	6.9	8.6	15	0.3	1	375	1.2	0.13								
AP 3	AA	185	1.00	open channel	10.37	30	13	10	60	51	6	3.0	4.9	25	4.3	1	1200	1.2	1.00								
AP 4	AA	215	0.20	1800	2.92	8	5	95	31	4	1.2	3.1	25	4.3	1	1650	2.2	1.00									
AA 8	AA	250	0.10	open channel	6.33	10	11	5	85	42	5	0.8	3.8	90	13.5	3	1800	7.5	1.00								

ILSAX RESULTS
1 IN 10 AEP FLOOD
EXISTING CATCHMENTS

Network	Reach Details			Catchment Area Parameters				Levels			Hydrology I in 10 AEP				Proposed Upgrade Options				Proposed Upgrade Works			Cost Breakdown				Total Cost (\$1000)																					
	branch	add hyd to	L S	existing conduit size (mm)	area (ha)	direct paved (%)	sup. paved (%)	grass time (min)	grass lag (min)	invert level mAHFD	ground level mAHFD	critical duration (min)	flowrate (m ³ /s)	Extra Pipes no.	New Pipes no.	Open Channel dia (mm)	Description	Essement Width (m)	Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)	Total (\$1000)																									
AS 1	AA	470	1.70	600	5.23	30	10	10	60	39	5	8.5	10.0		1	375	1.2	0.01																													
AA 9		290	0.10	open channel	17.45	15	5	80	62	8	0.5	3.4	14.2		3	1800	7.8	1.00	7.8m wide channel	30	452		23	475																							
AT 1		335	0.50	open channel	4.74	20	9	75	38	5	3.1	4.4	15		1	600	1.2	0.23	1.2m wide channel	4	240		12	252																							
AT 2	AA	245	0.50	open channel	10.82	25	13	10	65	52	6	1.5	3.0	20	1	900	1.2	0.33	1.2m wide channel	8	176		9	184																							
AA 10		175	0.10	open channel	22.38	20	17	5	75	68	8	0.2	3.0	90	4	1650	8.9	1.00	8.9m wide channel	30	297		15	312																							
AU 1		710	0.40	750	3.91	25	9	10	65	35	4	3.8	4.5	15	1	600	1.2	0.25																													
AU 2	AA	220	0.40	1500	8.32	25	12	10	65	47	6	0.9	2.5	20	1	825	1.2	0.32																													
AV 1	AA	235	1.00	1.50	30	6	10	60	24	3	2.4	4.0	10	1	375	1.2	0.12	1 x 375mm dia pipe	15	104	4	5	114																								
AA 11		60	0.10	4 x 1500	6.18	10	100	42	5	0.1	1.5	90	17.3	1	1500	4	1650	9.3	1.00	9.3m wide channel	15	165	21	6	132																						
																						\$4,415	\$152	\$228	\$4,795																						
BA 1		100	0.40		5.36	30	10	10	60	39	5	3.4	4.2	10	0.6	1	675	1.2	0.30	1 x 675mm dia pipe	26	7	2	34																							
BB 1	BA	90	0.40		5.91	30	10	60	41	5	3.3	4.9	20	0.7	1	675	1.2	0.34	1 x 675mm dia pipe	23	7	2	32																								
BA 2		125	0.40		2.64	30	8	10	60	36	4	3.0	4.6	25	1.7	1	1050	1.2	0.65	1 x 1050mm dia pipe	55	13	3	72																							
BA 3		65	0.40		3.49	30	8	10	60	34	4	2.5	4.5	25	2.1	1	1050	1.2	0.77	1 x 1050mm dia pipe	29	13	2	44																							
BA 4		60	0.40		2.39	30	7	10	60	29	4	2.2	4.6	20	2.3	1	1050	1.2	0.82	1 x 1050mm dia pipe	27	13	2	42																							
BA 5		60	0.40		3.14	30	8	10	60	32	4	2.8	4.6	25	2.7	1	1200	1.2	0.93	1 x 1200mm dia pipe	32	15	2	49																							
BA 6		70	0.40		3.51	30	8	10	60	34	4	1.7	4.1	25	3.1	1	1200	1.2	1.00	1 x 1200mm dia pipe	37	15	3	54																							
BA 7		140	0.40		2.97	30	8	10	60	32	4	1.5	4.3	20	3.4	1	1350	1.3	1.00	1 x 1350mm dia pipe	86	18	5	109																							
BA 8		240	0.10		2.17	30	7	10	60	28	3	0.9	4.6	25	3.7	1	1800	2.6	1.00	2.6m wide channel	35	214	26	12	252																						
BA 9		165	0.40		5.03	30	10	10	60	39	5	0.7	4.6	20	4.1	1	1800	1.5	1.00	1.5m wide channel	37	125	26	8	159																						
																						\$653	\$153	\$40	\$846																						
CA 1		165	0.50		750	3.05	30	8	10	60	32	4	2.3	4.7	15	0.4		1	525	1.2	0.23																										
CA 2		225	0.50		900	3.40	30	8	10	60	33	4	1.5	4.4	20	0.8		1	750	1.2	0.38																										
CA 3		75	0.50		1050	6.30	30	10	10	60	42	5	0.4	5.0	20	1.5		1	1050	1.2	0.59																										
																						12.8	ha																								
DA 1		70	0.20		675	4.36	30	9	10	60	36	5	0.3	1.2	10	0.3		1	750	1.2	0.38	1 x 375mm dia pipe	10	4	1	15																					
DA 2		65	0.20		675	1.12	30	6	10	60	22	3	0.1	1.5	25	0.7		1	825	1.2	0.48	1 x 525mm dia pipe	13	6	1	20																					
																						5.5	ha																								
EA 1		60	0.50		750	2.12	30	7	10	60	28	3	0.3	1.2	15	0.3		1	525	1.2	0.20																										
																						2.1	ha																								
FA 1		70	0.10		1050	1.94	60	7	20	20	27	3	0.2	1.2	10	0.5		1	825	1.2	0.48																										
FB 1	FA	80	0.1		0.75	60	6	20	20	19	3	0.2	1.2	10	0.2		1	600	1.2	0.25	1 x 600mm dia pipe	18	6	1	25																						
FA 2		145	0.1		1050	0.50	60	6	20	20	16	3	0.2	1.1	20	0.9		1	1050	1.2	0.74																										
FA 3		10	0.10		1050	1.22	60	6	20	20	22	3	0.0	1.1	25	1.2		1	1200	1.2	0.93	1 x 600mm dia pipe	2	6	0	9																					
																						4.4	ha																								
GA 1		50	0.50		525	1.05	30	6	10	60	21	3	0.4	1.2	10	0.1		1	375	1.2	0.09																										
GA 2		20	0.50		525	0.55	30	6	10	60	17	3	0.1	1.0	10	0.2		1	450	1.2	0.15																										
																						1.6	ha																								

ILSAX RESULTS
1 IN 10 AEP FLOOD
EXISTING CATCHMENTS

Network	Reach Details			Catchment Area Parameters						Levels			Hydrology			Proposed Upgrade Options			Proposed Upgrade Works		Cost Breakdown			Total Cost (\$1000)						
	branch & reach	add hyd to	L (m)	S (%)	emerging conduit size (mm)	area (ha)	paved direct (%)	paved time (min)	paved sup. (%)	grass time (min)	grass lag (min)	invert level mAHID	ground level mAHID	critical dimension (m)	flowrate (m³/s)	Extra Pipes no. dia. (mm)	New Pipes no. dia. (mm)	Open Channel w d (m)	Description	Easement Width (m)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)							
																									total area	total area	total area	total area	total area	total area
FA 1	40	1.00				1.50	30	6	10	60	24	3	0.4	1.2	10	0.2	1	375	1.2	1.2	1.012	1 x 375mm dia pipe	4	4	0	4		\$4	\$0	\$4
IA 1	90	0.10			600	5.73	30	10	60	40	5	1.2	2.5	20	0.7	1	900	1.050	1.2	0.61	1 x 900mm dia pipe	33	10	2	45					
IB 1	155	1.00			450	5.65	30	10	60	40	5	2.7	4.0	20	0.7	1	450	1.050	1.2	0.27	1 x 450mm dia pipe	26	5	2	32					
IA 2	285	0.10			900	4.25	30	9	60	36	5	1.1	2.2	25	1.9	1	1050	1.6	1.00	1.00	1 x 1050mm dia pipe	126	13	7	146					
IA 3	185	0.10			830	3.30	30	12	60	47	6	0.8	1.8	25	2.7	1	1650	2.0	1.00	1.00	1 x 1650mm dia pipe	150	23	9	181					
IA 4	90	0.10			426	3.10	30	9	60	36	5	0.7	1.4	20	3.0	1	1650	2.2	1.00	1.00	1 x 1650mm dia pipe	73	23	5	101					
IA 5	170	0.80			450	3.14	30	8	60	32	4	1.9	3.1	15	0.4	1	375	1.2	1.2	0.20	1 x 375mm dia pipe	24	4	1	30					
IA 6	160	0.10			349	3.49	30	8	60	34	4	0.6	1.3	25	3.6	1	1800	2.5	1.00	1.00	2.5m wide channel	6	7	8	161					
IA 7	100	0.10			710	2.5	11	10	65	44	5	0.4	1.3	25	4.1	2	1500	2.8	1.00	1.00	2.0m wide channel	6	147	7	154					
IA 8	115	0.75			548	0.65	30	6	10	60	18	3	0.8	1.7	10	0.1	1	375	1.2	1.2	0.12	1 x 375mm dia pipe	67	4	4	74				
IA 9	20	0.10			480	5	9	5	90	38	5	0.1	1.0	25	4.3	2	1500	2.9	1.00	1.00	2.9m wide channel	6	107	5	112					
					56.1 ha																	\$1017	\$93	\$56	\$1166					
JA 1	140	0.30			1.94	30	7	10	60	27	3	0.5	1.5	10	0.2	1	525	1.2	1.2	0.17	1 x 525mm dia pipe	27	6	2	35					
JA 2	20	0.30			1.72	30	6	10	60	26	3	0.1	1.2	25	0.5	1	675	1.2	1.2	0.32	1 x 675mm dia pipe	5	7	1	13					
					3.7 ha																	\$33	\$13	\$2	\$48					
KA 1	80	0.15			0.90	15	6	85	20	3	0.1	0.8	10	0.1	1	375	1.2	1.2	0.14	1 x 375mm dia pipe	4	4	0	4						
					0.9 ha																	\$4	\$0	\$4						
LA 1	70	0.20			2.24	15	7	85	28	4	0.1	0.8	10	0.1	1	450	1.2	1.2	0.13	1 x 450mm dia pipe	12	5	1	18						
					2.2 ha																	\$12	\$5	\$1	\$18					
MA 1	70	0.20			2.07	30	7	10	60	27	3	0.5	1.4	15	0.3	1	600	1.2	1.2	0.27	1 x 600mm dia pipe	16	6	1	23					
MA 2	120	0.20			0.90	30	6	10	60	20	3	0.3	1.3	15	0.4	1	675	1.2	1.2	0.32	1 x 675mm dia pipe	31	7	2	40					
MA 3	115	0.10			375	0.50	30	6	10	60	16	3	0.2	0.8	10	0.1	1	375	1.2	1.2	0.16	1 x 375mm dia pipe	4	4	0	4				
MA 3	80	0.10			450	0.90	30	6	10	60	20	3	0.1	0.8	15	0.5	1	750	1.2	1.2	0.48	1 x 750mm dia pipe	23	9	2	34				
					4.4 ha																	\$70	\$26	\$5	\$101					
NA 1	110	0.30			1.46	30	6	10	60	24	3	1.0	2.0	10	0.1	1	375	1.2	1.2	0.11	1 x 375mm dia pipe	4	4	0	4					
NA 2	88	0.30			0.48	30	6	10	60	16	3	0.7	2.0	10	0.2	1	450	1.2	1.2	0.17	1 x 450mm dia pipe	15	5	1	21					
NA 3	58	0.30			0.39	30	6	10	60	16	3	0.5	1.7	10	0.2	1	525	1.2	1.2	0.17	1 x 525mm dia pipe	11	6	1	18					
NB 1	57	0.15			2.55	10	7	5	85	30	4	0.5	1.5	10	0.1	1	450	1.2	1.2	0.14	1 x 450mm dia pipe	10	5	1	15					
NB 2	68	0.15			0.87	30	6	10	60	20	3	0.5	1.6	10	0.2	1	525	1.2	1.2	0.22	1 x 525mm dia pipe	13	6	1	20					
NC 1	36	0.15			0.48	30	6	10	60	16	3	0.3	1.6	15	0.2	1	525	1.2	1.2	0.22	1 x 525mm dia pipe	7	6	1	14					
NB 3	53	0.15			0.69	30	6	10	60	18	3	0.4	1.5	15	0.5	1	750	1.2	1.2	0.41	1 x 750mm dia pipe	15	8	1	24					
NA 4	46	0.10			0.48	30	6	10	60	16	3	0.3	1.4	15	0.8	1	1050	1.2	1.2	0.67	1 x 1050mm dia pipe	20	13	2	35					
ND 1	77	0.80			0.90	30	6	10	60	20	3	1.2	1.9	10	0.1	1	375	1.2	1.2	-0.06	1 x 375mm dia pipe	34	4	2	40					

ILSAX RESULTS 1 IN 10 AEP FLOOD EXISTING CATCHMENTS

Network		Reach Details		Catchment Area Parameters				Levels			Hydrology 1 in 10 AEP			Proposed Upgrade Options			Proposed Upgrade Works			Cost Breakdown				Total Cost																											
branch reach	add. hyd. reach	L (m)	S (%)	existing conduit size (mm)	area (ha)	paved direct (%)	paved time (min)	grass time (%)	grass time (min)	invert level (m)	ground level (m)	critical duration (min)	flowrate (m ³ /s)	Extra Pipes no.	Extra Pipes dia. (mm)	New Pipes no.	New Pipes dia. (mm)	Open Channel w (m)	Open Channel d (m)	Description	Basement Width (m)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)	Total (\$1000)																										
ANA 1																										430	0.50	375	2.98	30	8	10	60	32	4	4.6	5.7	15	0.4	1	375	1	525	1.2	0.23	1 x 375mm dia pipe		61	4	3	69
ANA 2																										185	0.50	900	5.95	30	10	10	60	41	5	2.5	4.3	20	1.1	1	825	1.2	0.47								
ANB 1																										ANA	270	0.50	525	13.60	30	14	10	60	56	7	2.9	5.0	15	0.6	1	450	1.2	0.39	1 x 450mm dia pipe		45	5	3	53	
ANA 3																										70	1.00	1050	13.33	30	14	10	60	56	7	1.6	3.7	20	2.3	1	1050	1.2	0.63								
ANC 1																										ANA	225	0.50	750	13.33	30	14	10	60	56	7	2.0	3.7	20	0.8	1	750	1.2	0.38							
ANA 4																										85	1.00	1350	13.33	30	14	10	60	56	7	0.9	3.7	25	3.6	1	1200	1.2	0.89								
total area																										62.5	ha																								
AOA 1																										80	0.20	5.71	30	10	10	60	40	5	0.2	1.5	20	0.7	1	825	1.2	0.48	1 x 825mm dia pipe		26	9	2	37			
total area																										5.7	ha																								
APA 1																										215	0.35	600	4.80	30	9	10	60	38	5	3.9	5.2	15	0.6	1	375	1	675	1.2	0.34	1 x 375mm dia pipe		31	4	2	36
APA 2																										225	0.35	750	5.03	30	10	10	60	39	5	3.1	5.3	20	1.2	1	525	1	900	1.2	0.57	1 x 525mm dia pipe		44	6	3	53
APB 1																										APA	270	0.20	1050	9.40	30	12	10	60	49	6	2.9	4.6	20	1.0	1	1050	1.2	0.62							
APA 3																										340	0.35	1350	9.57	30	12	10	60	49	6	2.4	5.5	20	3.2	1	1350	1.4	1.00								
APC 1																										APA	210	1.00	750	5.56	30	10	10	60	40	5	3.2	4.8	20	0.7	1	600	1.2	0.27							
APA 4																										265	0.35	1350	12.86	30	14	10	60	55	7	1.1	3.5	25	5.0	1	1050	1	1650	2.0	1.00	1 x 1050mm dia pipe		117	13	7	137
APA 5																										85	0.20	2 x 1350	7.88	25	11	5	70	46	6	0.2	1.8	25	5.6	1	1800	2.7	1.00								
total area																										55.1	ha																								
AOA 1																										110	0.50	3.24	30	8	10	60	33	4	1.9	3.2	10	0.4	1	525	1	525	1.2	0.23	1 x 525mm dia pipe		22	6	1	29	
AOA 2																										125	0.50	4.49	30	9	10	60	37	5	1.3	3.2	25	1.0	1	825	1	825	1.2	0.44	1 x 825mm dia pipe		41	9	2	52	
AOB 1																										AOA	305	0.59	525	2.97	30	8	10	60	32	4	2.2	3.7	15	0.4	1	525	1.2	0.23							
AOA 3																										110	0.50	375	4.36	30	9	10	60	36	5	0.7	3.0	20	1.8	1	1050	1	1050	1.2	0.68	1 x 1050mm dia pipe		49	13	3	65
AOA 4																										30	0.50	2.04	30	7	10	60	27	3	0.2	2.0	25	2.1	1	1050	1	1050	1.2	0.77	1 x 1050mm dia pipe		13	13	1	28	
total area																										17.1	ha																								
ARA 1																										170	1.50	375	1.90	30	6	10	60	23	3	3.8	5.2	15	0.2	1	375	1.2	0.10								
ARA 2																										60	2.00	450	1.62	30	6	10	60	25	3	1.2	5.2	15	0.4	1	450	1.2	0.15								
total area																										2.9	ha																								
ASA 1																										30	5.00	0.62	30	6	10	60	17	3	1.5	4.8	10	0.1	1	375	1	375	1.2	0.05	1 x 375mm dia pipe		4	4	0	4	
total area																										0.6	ha																								
ATA 1																										30	5.00	1.72	30	6	10	60	26	3	1.5	4.6	10	0.2	1	375	1	375	1.2	0.07	1 x 375mm dia pipe		4	4	0	4	
total area																										1.7	ha																								
AUA 1																										590	0.30	6.93	30	11	10	60	44	5	3.9	5.3	20	0.8	1	825	1	825	1.2	0.45	1 x 825mm dia pipe		192	9	10	211	
AUA 2																										200	0.30	600	13.73	30	14	10	60	56	7	2.2	5.8	20	2.1	1	1050	1	1200	1.2	0.94	1 x 1050mm dia pipe		89	13	5	107
AUB 1																										225	0.90	5.81	30	10	10	60	41	5	4.6	5.8	20	0.7	1	600	1	600	1.2	0.28	1 x 600mm dia pipe		51	6	3	60	
AUB 2																										AUA	110	0.90	7.68	30	11	10	60	45	6	2.6	6.0	25	1.6	1	825	1	825	1.2	0.50	1 x 825mm dia pipe		36	9	2	47
AUA 3																										175	0.30	3.99	30	9	10	60	35	4	1.6	4.2	25	4.0	1	1500	1.8	1.00	1 x 1500mm dia pipe		124	21	7	152			
AUC 1																										AUA	260	1.50	525	2.45	30	7	10	60	29	4	5.0	6.0	10	0.3	1	450	1.2	0.13							
AU 1																										AUA	125	2.50	525	3.49	30	8	10	60	34	4	4.2	5.1	10	0.4	1	450	1.2	0.14							
AUA 4																										290	0.30	1050	7.93	30	11	10	60	46	6	1.1	4.9	25	5.5	1	1350	1	1650	2.3	1.00	1 x 1350mm dia pipe		177	18	10	205
AUE 1																										150	1.20	1050	2.74	30	8	10	60	31	4	3.7	4.6	10	0.3	1	450	1.2	0.14								

ILSAX RESULTS
1 IN 10 AEP FLOOD
EXISTING CATCHMENTS

Network	Reach Details			Catchment Area Parameters						Levels			Hydrology I in 10 AEP			Proposed Upgrade Options				Proposed Upgrade Works			Cost Breakdown			Total Cost (\$1000)
	branch & reach	L (ft)	S (%)	existing conduit size (mm)	area (ha)	paved direct (%)	paved time (min)	paved map. (%)	grass time (min)	grass lag (min)	invert level mAHD	ground level mAHD	critical duration (min)	flowrate (m ³ /s)	Extra Pipes no.	New Pipes no.	Open Channel w (m)	d (m)	Description	Basement Width (m)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)			
AUE 2	145	1.20		1200	2.62	30	8	10	60	36	4	1.9	4.8	20	0.7	1	600	1.2	0.25	1 x 1200mm dia pipe		32	15	2	40	
AUA 5	60	0.30		1350	8.23	30	12	10	60	46	6	0.2	5.5	25	6.8	1	1200	2.7	1.00		\$701	\$91	\$40	\$831		
				total area	65.6	ha																				
AVA 1	130	0.50		3.65	30	9	10	60	34	4	2.3	5.9	25	0.5	1	600	1.2	0.27	1 x 600mm dia pipe		30	6	2	37		
AVB 1	AVA	100	0.50	1.00	30	6	10	60	21	3	2.1	6.0	10	0.1	1	375	1.2	0.09	1 x 375mm dia pipe		14	4	1	19		
AVA 2	215	0.50		1.48	30	6	10	60	24	3	1.6	6.0	25	0.8	1	750	1.2	0.38	1 x 750mm dia pipe		62	8	4	74		
AVC 1	AVA	130	0.50	3.35	30	8	10	60	33	4	1.2	4.6	10	0.4	1	600	1.2	0.23	1 x 600mm dia pipe		30	6	2	37		
AVA 3	110	0.50		2.83	30	8	10	60	31	4	0.6	5.0	20	1.5	1	900	1.2	0.59	1 x 900mm dia pipe		40	10	3	53		
				total area	12.3	ha															\$176	\$34	\$10	\$220		
AW 1	250	0.50		450	1.93	30	7	10	60	27	3	4.2	6.5	25	0.3	1	375	1.2	0.20	1 x 375mm dia pipe		36	4	2	42	
AW 2	230	0.50		600	3.85	30	9	10	60	35	4	3.0	5.5	15	0.7	1	375	1.2	0.34	1 x 375mm dia pipe		33	4	2	39	
AW 1	AW	160	0.50	750	0.80	30	6	10	60	19	3	2.6	5.0	10	0.1	1	375	1.2	0.09							
AW 3	140	0.50		750	6.30	20	10	5	75	42	5	1.8	5.0	20	1.3	1	525	1.2	0.53	1 x 525mm dia pipe		27	6	2	35	
AW 4	225	0.50		900	4.08	20	9	5	75	36	4	1.1	6.0	20	1.6	1	600	1.2	0.63	1 x 600mm dia pipe		51	6	3	60	
				total area	17.0	ha															\$147	\$20	\$8	\$176		
				total area	113.63	ha															\$11,251	\$1,310	\$628	\$13,189		

SUMMARY OF 2(b) AREA 2(b) AREA = 413 ha

2(b) AREA COST = 413 / 1137 * 13,189 K = \$4,791 K

APPENDIX A2

**ILSAX RESULTS
1 IN 100 AEP FLOOD
EXISTING CATCHMENTS**

Network		Reach Details				Catchment Area Parameters						Levels			Hydrology 1 in 100 AEP		Proposed Upgrade Options				Proposed Upgrade Works		Cost Breakdown			Total Cost		
branch & reach	add byd to	L (m)	S (%)	existing conduit size (mm)	area (ha)	direct (%)	indirect (%)	preved time (min)	preved sup. (%)	grass (%)	grass time (min)	grass lag (min)	invert mAHHD	ground level mAHHD	critical duration (min)	flowrate (m ³ /s)	Extra dia. (mm)	Piped no. dia. (mm)	New Pipes no. dia. (mm)	Open Channel w (m)	d (m)	Description	Basement Width (m)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)	Total Cost (\$1000)	
AA 1		335	0.20	675	13.13	30	14	10	60	55	7	3.8	5.2	20	2.1	1	1200	1	1350	1.3	1.00	1 x 1200mm dia pipe		177	15	10	201	
AB 1	AA	230	0.50	600	3.41	30	6	10	60	33	4	4.3	5.3	20	0.7	1	375	1	675	1.2	0.34	1 x 375mm dia pipe		33	4	2	39	
AA 2		290	0.20	2 x 750	10.99	30	13	10	60	52	6	3.1	5.1	20	4.5	1	1350	1	1650	2.3	1.00	1 x 1350mm dia pipe		177	18	10	205	
AA 3		185	0.20	2 x 900	6.85	30	11	10	60	43	5	2.6	5.1	25	5.7	1	1350	1	1800	2.8	1.00	1 x 1350mm dia pipe		113	18	7	138	
AC 1	AA	190	0.50	750	4.61	30	9	10	60	37	5	3.1	4.4	20	0.9	1	1800	1	750	1.2	0.41							
AA 4		130	0.20	2 x 900	5.58	30	10	10	60	40	5	2.2	4.6	25	7.3	1	1800	2	1500	3.3	1.00	3.3m wide channel	20	129	50	9	187	
AA 5		220	0.20	2 x 900	3.32	30	8	10	60	33	4	1.9	4.4	25	7.7	1	1800	2	1500	3.5	1.00	3.5m wide channel	21	222	55	14	290	
AD 1		585	0.20	600	14.58	30	14	10	60	58	7	4.3	5.5	20	2.3	1	1350	1	1350	1.4	1.00	1 x 1350mm dia pipe		358	18	19	595	
AE 1	AD	130	0.20	600	8.82	30	12	10	60	48	6	3.4	4.9	20	1.5	1	1050	1	1200	1.2	0.84	1 x 1050mm dia pipe		58	13	4	74	
AD 2		255	0.20	1050	4.63	30	9	10	60	37	5	3.1	5.2	25	4.5	1	1350	1	1650	2.3	1.00	1 x 1350mm dia pipe		156	18	9	183	
AF 1	AD	90	1.00	1050	4.11	30	9	10	60	36	4	3.5	4.8	20	0.8	1	1800	1	675	1.2	0.36							
AD 3		355	0.20	1200	1.42	30	6	10	60	24	3	2.6	5.8	25	5.2	1	1350	1	1800	2.6	1.00	1 x 1350mm dia pipe		217	18	12	247	
AG 1	AD	190	0.20	open channel	21.85	30	17	10	60	8	8	2.3	4.5	90	8.7	2	1800	3.9	1.00	3.9m wide channel		18	200	23	11	234		
AD 4	AA	205	0.20	open channel	1.99	30	7	10	60	27	3	1.9	5.8	90	13.0	3	1650	5.4	1.00	5.4m wide channel		31	256	55	16	306		
AA 6		490	0.10	open channel	6.58	30	11	10	60	43	5	1.5	4.3	90	21.0	4	1800	11.1	1.00	11.1m wide channel		33	968	124	55	1147		
AH 1	AA	310	3.80	900	7.88	30	5	10	60	5	5	12.8	14.1	90	2.9	1	825	1.2	0.46									
AJ 1		60	0.20	750	8.10	5	12	10	95	44	6	2.0	5.0	90	0.5	1	750	1.2	0.38									
AJ 1	AI	40	0.30	750	2.97	30	8	10	60	32	4	2.0	4.2	20	0.6	1	750	1.2	0.37									
AK 1	AI	50	0.40	825	4.86	30	10	10	60	38	5	2.1	4.2	20	0.9	1	825	1.2	0.45									
AL 1	AI	180	0.15	975	5.33	30	10	10	60	39	5	3.2	4.7	20	1.0	1	450	1	1050	1.2	0.69	1 x 450mm dia pipe		30	5	2	37	
AL 2		120	0.15	1275	4.26	30	9	10	60	36	5	2.9	4.7	20	1.8	1	450	1	1350	1.3	1.00	1 x 450mm dia pipe		20	5	1	26	
AL 3		110	0.15	1500	5.63	30	10	10	60	40	5	2.7	4.6	20	2.8	1	1500	1.8	1.00									
AL 4		100	0.15	1950	4.06	30	9	10	60	36	4	2.5	5.3	20	3.5	1	1650	2.1	1.00									
AL 5		290	0.15	1800	3.27	30	8	10	60	33	4	2.4	4.9	20	4.1	1	1800	2.4	1.00									
AL 6	AI	50	0.15	1800	3.29	30	8	10	60	33	4	2.0	4.2	25	4.6	1	1800	2.6	1.00									
AI 2		40	0.50	lagoon link	7.23																							
AM 1	AI	60	0.50	600	2.94	30	8	10	60	31	4	2.0	4.0	20	0.6	1	375	1	675	1.2	0.53	1 x 375mm dia pipe		9	4	1	13	
AN 1	AI	130	0.27	825	5.28	30	10	10	60	39	5	2.0	4.9	20	1.0	1	375	1	900	1.2	0.55	1 x 375mm dia pipe		19	4	1	24	
AI 3		10	0.50	lagoon link	2.19																							
AI 4		20	0.50	lagoon link	2.31																							
AI 5		20	0.50	lagoon link	3.22																							
AI 6		10	0.50	lagoon link	3.14																							
AI 7		10	0.50	lagoon link	0.70																							
AI 8		30	0.30	2 x 825	1.42																							
AO 1	AI	30	1.09	900	3.91	15	9	10	85	35	4	1.6	4.3	90	0.5	1	525	1.2	0.21									
AI 9	AA	80	0.30	2 x 825	1.50	30	6	10	60	24	3	1.2	4.3	90	1.5	1	1500	1	1800	1.2	0.72	1 x 1500mm dia pipe		57	142	3	59	
AA 7		250	0.10	open channel	20.94	5	15	10	95	15	8	1.8	4.5	90	26.5	5	1800	13.8	1.00	13.8m wide channel		44	578	142	36	756		
AP 1		120	5.00	900	20.16	10	8	10	90	8	5	11.9	23.1	90	5.1	1	600	1	1050	1.2	0.63	1 x 600mm dia pipe		27	1	29	41	
AO 1	AP	200	5.00	750	10.12	10	5	10	90	5	5	15.9	32.5	90	3.6	1	900	1.2	0.48	1 x 900mm dia pipe		30	30	2	41			
AP 2		290	1.00	1200	5.43	30	4	10	70	4	5	5.9	8.5	90	10.5	1	1350	1	1800	2.4	1.00	1 x 1350mm dia pipe		177	6	9	193	
AR 1		95	5.00	5.46																								
AR 2	AP	215	1.80	1200	5.53	10	4	10	90	4	5	6.9	8.6	90	4.3	1	1200	1.2	0.81									
AP 3		185	1.00	open channel	10.37	40	6	10	60	10	5	3.0	4.9	90	16.4	2	1500	3.4	1.00	3.4m wide channel		14	183	31	11	225		
AP 4	AA	215	0.20	1800	2.92	10	3	10	90	5	5	1.2	3.1	90	16.9	2	1800	6.7	1.00	6.7m wide channel		46	395	23	21	439		
AA 8		230	0.10	open channel	6.33	10	11	10	85	42	5	0.8	3.8	90	43.2	7	1800	21.7	1.00	21.7m wide channel		46	765	194	48	1007		

ILSAX RESULTS
1 IN 100 AEP FLOOD
EXISTING CATCHMENTS

Network	Reach Details				Catchment Area Parameters						Levels		Hydrology 1 in 100 AEP		Proposed Upgrade Options				Proposed Upgrade Works		Cost Breakdown			Total Cost (\$1000)		
	branch & reach	L (m)	S (%)	add hyd to	existing conduit size (mm)	area (ha)	paved direct (%)	paved time (min)	grass exp. (%)	grass time (min)	grass (mm)	invert level mAHd	ground level mAHd	critical duration (min)	flowrate (m ³ /s)	Extra Pipes no. dia. (mm)	New Pipes no. dia. (mm)	Open Channel w (m)	d (m)	Description	Easement Width (m)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)		Services Relocation (\$1000)	
AS 1	AA	470	1.70	600	5.23	4	100	4	5	8.5	10.0	90	2.1	1	675	1	900	1.2	0.48	1 x 675mm dia pipe		121	4	6	131	
AA 9		290	0.10	open channel	17.45	15	5	80	25	10	0.5	3.4	46.0	4	1800	4	1800	23.1	1.00	23.1m wide channel	46	1013	207	61	1281	
AT 1	AA	335	0.50	open channel	4.74	20	9	5	38	5	3.1	4.4	20	0.6	675	1	675	1.2	0.30	1.2m wide channel	4	240	17	13	270	
AT 2	AA	245	0.50	open channel	10.82	25	13	10	65	52	6	1.5	3.0	2.2	1050	1	1050	1.2	0.80	1.2m wide channel	8	176	17	10	202	
AA 10		175	0.10	open channel	23.38	20	15	5	75	25	10	0.2	3.0	90	5	1800	5	1800	25.5	1.00	25.5m wide channel	47	665	237	45	947
AU 1	AA	710	0.40	750	3.91	25	9	10	65	35	4	3.8	4.5	20	0.7	1750	1	1750	1.2	0.37						
AU 2	AA	220	0.40	1500	8.32	25	12	10	65	47	6	0.9	2.5	20	1.8	1850	1	1850	1.2	0.74						
AV 1	AA	235	1.00	1500	1.50	30	4	10	60	5	3	2.4	4.0	25	0.6	1	600	1	600	1.2	0.24	1 x 600mm dia pipe	53	6	3	62
AA 11		60	0.10	4 x 1500	6.18	10	100	42	5	0.1	1.5	90	52.4	3	1850	5	1800	26.1	1.00	26.1m wide channel	31	233	263	25	521	
				total area	365.2 ha																	\$7,890	\$1,600	\$475	\$9,965	
BA 1	BA	100	0.40	5.36	30	10	10	60	39	5	3.4	4.2	20	1.0	825	1	825	1.2	0.44	1 x 825mm dia pipe	33	9	2	44		
BB 1	BA	90	0.40	5.91	30	10	10	60	41	5	3.3	4.9	20	1.1	825	1	825	1.2	0.47	1 x 825mm dia pipe	29	9	2	40		
BA 2		125	0.40	2.64	30	8	10	60	30	4	3.0	4.6	20	2.7	1200	1	1200	1.2	0.93	1 x 1200mm dia pipe	66	15	4	85		
BA 3		65	0.40	3.49	30	8	10	60	34	4	2.5	4.5	20	3.3	1350	1	1350	1.3	1.00	1 x 1350mm dia pipe	40	18	3	61		
BA 4		60	0.40	2.39	30	7	10	60	29	4	2.2	4.6	20	3.7	1350	1	1350	1.4	1.00	1 x 1350mm dia pipe	37	18	3	57		
BA 5		60	0.40	3.14	30	8	10	60	32	4	2.0	4.6	20	4.3	1350	1	1350	1.6	1.00	1 x 1350mm dia pipe	37	18	3	57		
BA 6		70	0.40	3.51	30	8	10	60	34	4	1.7	4.1	25	5.0	1500	1	1500	1.8	1.00	1 x 1500mm dia pipe	50	21	4	74		
BA 7		140	0.40	2.97	30	8	10	60	32	4	1.5	4.3	20	5.5	1500	1	1500	1.9	1.00	1 x 1500mm dia pipe	99	21	6	126		
BA 8		240	0.10	2.17	30	7	10	60	28	3	0.9	4.6	25	6.0	2	1500	2	1500	3.8	1.00	3.8m wide channel	36	250	57	323	
BA 9		165	0.40	5.83	30	10	10	60	39	5	0.7	4.6	25	6.8	2	1500	2	1500	2.2	1.00	2.2m wide channel	37	139	31	179	
				total area	36.6 ha																	\$780	\$217	\$50	\$1,047	
CA 1		165	0.50	750	3.05	30	8	10	60	32	4	2.3	4.7	15	0.6	1	675	1	675	1.2	0.30					
CA 2		225	0.50	900	3.40	30	8	10	60	33	4	1.5	4.4	20	1.3	1	900	1	900	1.2	0.53					
CA 3		75	0.50	1050	6.30	30	10	10	60	42	5	0.4	5.0	20	2.5	1	600	1	1200	1.2	0.88	1 x 600mm dia pipe	17	6	1	24
				total area	12.8 ha																	\$17	\$6	\$1	\$24	
DA 1		70	0.20	675	4.36	30	9	10	60	36	5	0.3	1.2	20	0.8	1	600	1	900	1.2	0.52	1 x 600mm dia pipe	16	6	1	23
DA 2		65	0.20	675	1.12	30	6	10	60	22	3	0.1	1.5	20	1.1	1	825	1	1050	1.2	0.66	1 x 825mm dia pipe	21	9	2	32
				total area	5.5 ha																	\$37	\$15	\$3	\$55	
EA 1		60	0.50	750	2.12	30	7	10	60	28	3	0.3	1.2	25	0.5	1	600	1	600	1.2	0.27					
				total area	2.1 ha																					
FA 1		70	0.10	1050	1.94	60	7	20	20	27	3	0.2	1.2	15	0.8	1	1050	1	1050	1.2	0.63					
FB 1	FA	80	0.1	0.75	60	6	20	20	19	3	0.2	1.2	10	0.3	1	675	1	675	1.2	0.33	1 x 675mm dia pipe	21	7	1	29	
FA 2		145	0.1	1050	0.50	60	6	20	20	16	3	0.2	1.1	25	1.4	1	600	1	1200	1.3	1.00	1 x 600mm dia pipe	33	6	2	41
FA 3		10	0.10	1050	1.22	60	6	20	20	22	3	0.0	1.1	20	1.8	1	900	1	1350	1.5	1.00	1 x 900mm dia pipe	4	10	1	14
				total area	4.4 ha																	\$57	\$23	\$4	\$84	
GA 1		50	0.50	525	1.05	30	6	10	60	21	3	0.4	1.2	90	0.3	1	525	1	525	1.2	0.20					
GA 2		20	0.50	525	0.53	30	6	10	60	17	3	0.1	1.0	25	0.4	1	375	1	600	1.2	0.23	1 x 375mm dia pipe	3	4	0	7
				total area	1.6 ha																	\$3	\$4	\$0	\$7	

ILSAX RESULTS
1 IN 100 AEP FLOOD
EXISTING CATCHMENTS

Network	Reach Details				Catchment Area Parameters				Levels		Hydrology		Proposed Upgrade Options				Proposed Upgrade Works				Cost Breakdown				Total Cost (\$1000)			
	branch & reach to	L (m)	S (%)	existing conduit size (mm)	area (ha)	perched direct (%)	perched time (min)	perched sup. (%)	grass time (%)	grass time (min)	grass lag (min)	grass level (min)	invert level (min)	ground level (min)	critical duration (min)	flowrate (m ³ /s)	Extra no.	Pipes no.	New Pipes no.	Open Channel w/d (m)	Channel d (m)	Description	Basement Width (m)	Trunk Drainage (\$1000)		Proposed Culverts (\$1000)	Services Relocation (\$1000)	
H/A 1		40	1.00		1.50	30	6	10	60	24	3	0.4	1.2	90	0.4	1	525	1	525	1.2	0.19	1 x 525mm dia pipe	8	8	6	1	15	\$15
		total area 1.5 ha																										
JA 1	90	0.10		600	5.73	30	10	10	60	40	5	1.2	2.5	30	1.1	1	1050	1	1200	1.2	0.87	1 x 1050mm dia pipe	40	40	13	3	36	
JA 2	155	1.00		450	5.65	30	10	10	60	40	5	2.7	4.0	20	1.1	1	675	1	750	1.2	0.37	1 x 675mm dia pipe	40	40	7	2	49	
JA 3	285	0.10		900	4.25	30	9	10	60	36	5	1.1	2.2	20	2.9	1	1500	1	1650	2.1	1.00	1 x 1500mm dia pipe	202	202	41	11	234	
JA 4	185	0.10		630	8.30	30	12	10	60	47	6	0.8	1.8	20	4.3	2	1500	2	1500	2.9	1.00	2 x 1500mm dia pipe	262	262	42	15	319	
JA 5	90	0.10		426	3.14	30	8	10	60	32	4	1.9	3.1	15	0.6	1	450	1	600	1.2	0.26	1 x 450mm dia pipe	127	127	42	8	178	
JA 6	170	0.80		450	3.14	30	8	10	60	34	4	0.6	1.3	25	5.9	1	1350	2	1500	3.7	1.00	3.7m wide channel	7	171	57	11	239	
JA 7	160	0.10		open channel	7.10	25	11	10	65	44	5	0.4	1.3	25	6.9	1	1500	2	1800	4.2	1.00	4.2m wide channel	7	176	60	12	248	
JA 8	100	0.10		open channel	5.48	10	10	90	40	5	0.2	1.1	25	7.1	1	1500	2	1800	4.3	1.00	4.3m wide channel	7	111	61	9	181		
JA 9	260	0.25		0.65	30	6	10	60	18	3	0.8	1.7	25	0.2	1	450	1	450	1.2	0.19	1 x 450mm dia pipe	43	43	5	2	51		
JA 10	105	0.75		0.70	30	6	10	60	18	3	0.9	1.7	20	0.2	1	375	1	375	1.2	0.13	1 x 375mm dia pipe	15	15	4	1	20		
JA 11	115	0.10		open channel	4.80	5	9	5	90	38	5	0.1	1.0	25	7.6	1	1500	2	1800	4.6	1.00	4.6m wide channel	8	132	65	10	207	
JA 12	20	0.10		2x1000x1800	2.54	30	7	10	60	30	4	0.0	1.3	25	7.8	1	1200	2	1800	4.7	1.00	1 x 1200mm dia pipe	11	11	15	1	27	
		total area 56.1 ha																										
		\$1359 \$397 \$98 \$1844																										
JA 1	140	0.30		1.94	30	7	10	60	27	3	0.5	1.5	15	0.4	1	600	1	600	1.2	0.28	1 x 600mm dia pipe	32	32	6	2	40		
JA 2	20	0.30		1.72	30	6	10	60	26	3	0.1	1.2	20	0.6	1	825	1	825	1.2	0.45	1 x 825mm dia pipe	7	7	9	1	16		
		total area 3.7 ha																										
KA 1	80	0.15		0.90	15	6	85	20	3	0.1	0.8	10	0.8	10	0.1	1	375	1	375	1.2	0.14	1 x 375mm dia pipe	11	11	4	1	16	
		total area 0.9 ha																										
LA 1	70	0.20		2.24	15	7	85	28	4	0.1	0.8	90	0.3	1	600	1	600	1.2	0.27	1 x 600mm dia pipe	16	16	6	1	23			
		total area 2.2 ha																										
MA 1	70	0.20		2.07	30	7	10	60	27	3	0.5	1.4	25	0.5	1	750	1	750	1.2	0.38	1 x 750mm dia pipe	20	20	8	1	30		
MA 2	120	0.20		0.90	30	6	10	60	20	3	0.3	1.3	25	0.7	1	825	1	825	1.2	0.48	1 x 825mm dia pipe	39	39	9	2	51		
MA 3	115	0.10		375	0.50	30	6	10	60	16	3	0.2	0.8	10	0.1	1	375	1	450	1.2	0.16	1 x 375mm dia pipe	16	16	4	1	21	
MA 4	80	0.10		450	0.90	30	6	10	60	20	3	0.1	0.8	25	0.9	1	1050	1	1050	1.2	0.74	1 x 1050mm dia pipe	35	35	13	2	51	
		total area 4.4 ha																										
NA 1	110	0.30		1.46	30	6	10	60	24	3	1.0	2.0	10	0.3	1	525	1	525	1.2	0.23	1 x 525mm dia pipe	22	22	6	1	29		
NA 2	88	0.30		0.48	30	6	10	60	16	3	0.7	2.0	25	0.5	1	675	1	675	1.2	0.32	1 x 675mm dia pipe	23	23	7	1	31		
NA 3	58	0.30		0.39	30	6	10	60	16	3	0.5	1.7	90	0.6	1	750	1	750	1.2	0.37	1 x 750mm dia pipe	17	17	8	1	26		
NB 1	57	0.15		2.55	10	7	5	85	30	4	0.5	1.5	90	0.3	1	600	1	600	1.2	0.29	1 x 600mm dia pipe	13	13	6	1	20		
NB 2	68	0.15		0.87	30	6	10	60	20	3	0.5	1.6	90	0.1	1	750	1	750	1.2	0.41	1 x 750mm dia pipe	20	20	8	1	29		
NB 3	36	0.15		0.48	30	6	10	60	16	3	0.3	1.6	10	0.1	1	450	1	450	1.2	0.14	1 x 450mm dia pipe	6	6	5	1	12		
NB 4	53	0.15		0.69	30	6	10	60	18	3	0.4	1.5	25	0.7	1	900	1	900	1.2	0.53	1 x 900mm dia pipe	19	19	10	1	31		
NA 4	46	0.10		0.48	30	6	10	60	16	3	0.3	1.4	90	1.4	1	1200	1	1200	1.3	1.00	1 x 1200mm dia pipe	24	24	15	2	41		
ND 1	77	0.80		0.90	30	6	10	60	20	3	1.2	1.9	10	0.2	1	375	1	375	1.2	0.13	1 x 375mm dia pipe	41	41	4	2	47		

ILSAX RESULTS
1 IN 100 AEP FLOOD
EXISTING CATCHMENTS

Network	Reach Details			Catchment Area Parameters						Levels			Hydrology 1 in 100 AEP			Proposed Upgrade Options			Proposed Upgrade Works			Cost Breakdown			Total Cost (\$1000)	
	branch & reach	add hyd to	L (m)	S (%)	existing conduit size (mm)	area (ha)	paved direct (%)	paved time (min)	grass sup. (%)	grass time (min)	grass lag (min)	invert level mAHID	ground level mAHID	critical duration (min)	flowrate (m ³ /s)	Extra Pipes no. dia. (mm)	New Pipes no. dia. (mm)	Open Channel w (m)	d (m)	Description	Easement Width (m)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)		
ND 2	NA	38	0.80		1.65	30	6	10	60	25	3	0.5	1.8	25	0.6	1	600	1.2	0.26	1 x 600mm dia pipe	9	9	6	1	15	
NA 5	NA	131	0.10		0.57	30	6	10	60	17	3	0.2	1.5	90	2.1	1500	1.500	1.7	1.00	1 x 1500mm dia pipe	93	21	6	6	119	
NE 1	NA	114	0.15	375	0.60	30	6	10	60	17	3	0.3	1.0	90	0.2	375	1.525	1.2	1.2	1 x 375mm dia pipe	16	4	4	1	21	
NA 6	NA	104	0.10		1.00	30	6	10	60	21	3	0.1	1.0	90	2.4	1500	1.500	1.9	1.00	1 x 1500mm dia pipe	74	21	5	5	99	
					total area																\$375	\$121	\$25		\$521	
OA 1	OA	120	0.80		750	259	30	7	20	50	4	2.4	3.5	25	0.6	1	600	1.2	0.26							
OA 2	OA	155	0.80		750	0.75	5	6	95	19	3	1.4	3.8	90	0.7	1	675	1.2	0.29							
OB 1	OA	150	0.10		375	0.87	30	6	10	60	20	3	0.3	1.0	10	0.2	1	525	1.2	0.25	1 x 375mm dia pipe	21	4	1	1	27
OA 3	OA	80	0.20		1.99	30	7	10	60	27	3	0.2	1.5	90	1.3	1050	1.050	1.2	0.76	1 x 1050mm dia pipe	35	13	2	2	51	
					total area																\$57	\$17	\$4		\$78	
PA 1	PA	100	0.50		13.05	30	14	10	60	55	7	0.5	4.5	20	2.1	1050	1.050	1.2	0.77	1 x 1050mm dia pipe	44	13	3	3	60	
					total area																\$44	\$13	\$3		\$60	
QA 1	QA	195	0.70		2.25	30	7	10	60	28	4	4.0	5.0	20	0.5	1	600	1.2	0.24	1 x 600mm dia pipe	44	6	3	3	53	
OB 1	QA	95	0.40		675	1.80	30	7	10	60	26	3	3.0	4.1	15	0.4	1	600	1.2	0.25						
QA 2	QA	130	0.40		675	2.37	10	7	90	29	4	2.6	4.5	25	1.1	1	525	1.2	0.52	1 x 525mm dia pipe	25	6	2	2	33	
QA 3	QA	175	0.40		750	0.77	20	12	5	75	48	6	2.1	3.6	20	2.1	1050	1.2	0.84	1 x 1050mm dia pipe	78	13	5	5	95	
QA 4	QA	190	0.40		900	4.69	10	9	20	70	38	5	1.4	3.2	90	2.7	1200	1.2	1.00	1 x 1200mm dia pipe	62	9	4	4	74	
QC 1	QA	135	0.35		750	5.55	30	10	10	60	40	5	1.8	3.6	20	1.1	525	1.2	0.54	1 x 525mm dia pipe	26	6	2	2	34	
QC 2	QA	195	0.35		750	2.99	30	8	10	60	32	4	1.3	3.3	20	1.6	750	1.2	0.71	1 x 750mm dia pipe	56	8	3	3	68	
QA 5	QA	115	0.10		2 x 900	3.34	5	8	5	90	33	4	0.6	2.5	90	4.4	1500	2.9	1.00	1 x 1500mm dia pipe	81	21	5	5	108	
OD 1	QA	185	0.50		900	4.78	25	9	10	65	38	5	3.6	4.8	20	0.8	1	750	1.2	0.38						
QD 2	QA	305	0.70		1050	2.17	25	7	10	65	28	3	2.7	5.2	20	1.1	825	1.2	0.42							
QE 1	QA	255	0.50		600	6.03	30	10	10	60	41	5	1.8	4.6	20	1.2	1	600	1.2	0.50	1 x 600mm dia pipe	58	6	3	67	
OF 1	QE	90	1.00		525	4.83	15	9	5	80	38	5	1.4	4.6	20	0.5	1	525	1.2	0.21						
QE 2	QA	310	0.50		9.48	25	12	5	70	49	6	2.1	4.3	20	3.0	1200	1.2	1.00	1 x 1200mm dia pipe	163	15	9	9	187		
QA 6	QA	405	0.10	open channel	11.20	10	13	5	85	52	7	0.5	3.0	90	9.3	3	1650	5.4	1.00	5.4m wide channel	23	508	78	29	615	
OA 7	QA	120	0.10	1050 + 1200	8.95	25	12	5	70	48	6	0.1	3.2	90	10.2	2	1800	5.9	1.00	2 x 1800mm dia pipe	220	52	14	286		
					total area																\$1323	\$220	\$77		\$1620	
RA 1	RA	135	0.30		750	6.85	30	11	10	60	43	5	0.8	2.2	20	1.3	1	750	1.2	0.65	1 x 750mm dia pipe	39	8	2	49	
RA 2	RA	80	0.50		900	1.85	30	7	10	60	26	3	0.4	3.5	20	1.6	1	600	1.2	0.63	1 x 600mm dia pipe	18	6	1	25	
					total area																\$37	\$14	\$4		\$75	
SA 1	SA	285	0.25		4.69	30	9	10	60	38	5	5.0	6.1	20	0.9	1	900	1.2	0.52	1 x 900mm dia pipe	105	10	6	6	120	
SA 2	SA	345	0.25		12.15	30	13	10	60	54	7	4.3	5.9	20	2.9	1350	1.5	1.00	1 x 1350mm dia pipe	211	18	11	241			
SA 3	SA	325	0.25		14.70	30	14	10	60	58	7	3.4	5.5	20	5.2	1650	2.3	1.00	1 x 1650mm dia pipe	263	23	14	300			
SA 4	SA	200	0.25		900	9.63	30	12	10	60	49	6	2.6	5.0	25	6.6	1800	2.8	1.00	1 x 1800mm dia pipe	184	26	10	220		
SB 1	SA	250	0.30		4.83	15	9	5	80	38	5	3.9	5.0	20	0.5	1	600	1.2	0.27	1 x 600mm dia pipe	57	6	3	66		
SB 2	SA	100	0.50		2.03	30	7	10	60	27	3	2.6	4.8	20	0.9	1	750	1.2	0.41	1 x 750mm dia pipe	29	8	2	39		
SA 5	SA	195	0.25		1200	4.83	30	9	10	60	38	5	2.1	5.1	25	8.0	1800	3.3	1.00	1 x 1800mm dia pipe	179	26	10	215		
SA 6	SA	160	0.25		1350	2.18	30	7	10	60	28	4	1.6	4.6	25	8.3	1650	2	1.00	1 x 1650mm dia pipe	129	23	8	160		
SC 1	SA	175	0.20		3.48	30	8	10	60	33	4	3.5	5.9	20	0.7	1	825	1.2	0.48	1 x 825mm dia pipe	57	9	3	69		

**ILSAX RESULTS
1 IN 100 AEP FLOOD
EXISTING CATCHMENTS**

Network	Reach Details				Catchment Area Parameters								Levels			Hydrology			Proposed Upgrade Options				Proposed Upgrade Works			Cost Breakdown				Total Cost (\$1000)
	branch & reach	add to	L (m)	S (%)	existing conduit size (mm)	area (ha)	paved direct (%)	paved time (min)	grass sup. (%)	grass time (min)	grass lag (min)	invert level mASLD	ground level mASLD	critical duration (min)	flowrate (m ³ /s)	Extra Pipes no. dia. (mm)	New Pipes no. dia. (mm)	Open Channel w (m)	d (m)	Description	Easement Width (m)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)	Total Cost (\$1000)					
SC 2		235	0.20		6.10	30	10	10	60	41	5	3.1	6.3	20	1.9	1	1200	1.2	1.00	1 x 1200mm dia pipe		124	15	7	146					
SC 3		240	0.20		8.58	30	12	10	60	47	6	2.7	5.7	20	3.3	1	1500	1.8	1.00	1 x 1500mm dia pipe		170	21	10	200					
SC 4		90	0.20		9.93	30	12	10	60	50	6	2.2	4.9	20	4.9	1	1800	2.4	1.00	1 x 1800mm dia pipe		83	26	5	114					
SC 5		290	0.20		6.83	30	11	10	60	43	5	2.0	4.9	20	6.0	2	1650	2.9	1.00	1 x 1650mm dia pipe		227	23	12	262					
SC 6		110	0.20		1350	9.45	30	12	10	60	49	6	1.5	4.3	20	7.4	1	1650	3.4	1.00	1 x 1650mm dia pipe		89	23	6	118				
SC 7	SA	135	0.20		1500	1.13	30	6	10	60	22	3	1.5	4.1	20	7.6	1	1500	3.5	1.00	1 x 1500mm dia pipe		96	21	6	122				
SA 7		85	0.25		1800	3.55	30	8	10	60	34	4	1.2	4.8	25	16.2	2	1500	5.9	1.00	2 x 1500mm dia pipe		120	42	8	170				
SA 8		130	0.25		1800	2.20	30	7	10	60	28	4	1.0	4.5	25	16.4	2	1500	6.0	1.00	2 x 1500mm dia pipe		184	42	11	237				
SD 1	SA	220	0.70		675	3.87	30	9	10	60	35	4	2.2	3.4	20	0.8	1	675	1.2	0.34										
SA 9		220	0.25		1800	3.87	30	9	10	60	35	4	0.7	3.2	25	17.6	2	1500	6.3	1.00	2 x 1500mm dia pipe		312	42	18	371				
SA 10		60	0.25		1800	3.95	30	9	10	60	35	4	0.2	3.3	25	18.1	2	1500	6.5	1.00	2 x 1500mm dia pipe		85	42	6	133				
					total area	118.1	ha															\$2,702	\$446	\$157	\$3,305					
TA 1		285	0.30		600	14.27	30	14	10	60	57	7	1.8	3.3	20	2.3	1	1050	1.2	1.00	1 x 1050mm dia pipe		126	13	7	146				
TA 2		170	0.30		600	5.50	30	10	10	60	40	5	1.0	3.9	20	3.2	1	1350	1.5	1.00	1 x 1350mm dia pipe		104	18	6	128				
TB 1	TA	50	2.00		600	2.39	30	7	10	60	29	4	1.5	2.7	20	0.5	1	450	1.2	0.17										
TA 3		120	0.30		593	30	10	10	60	41	5	0.5	2.5	25	4.7	1	1650	2.0	1.00	1 x 1650mm dia pipe		97	23	6	126					
TA 4		30	0.30		132	30	6	10	60	23	3	0.1	1.8	25	4.9	1	1650	2.1	1.00	1 x 1650mm dia pipe		24	23	2	50					
					total area	29.4	ha															\$352	\$77	\$21	\$450					
UA 1		65	0.50		450 + 600	3.61	30	8	10	60	34	4	0.3	1.5	20	0.7	1	750	1.2	0.34										
					total area	3.6	ha																							
VA 1		75	1.00		525	1.57	30	6	10	60	25	3	0.8	2.0	25	0.4	1	525	1.2	0.19										
					total area	1.6	ha																							
WA 1		25	5.00		375	0.72	30	6	10	60	18	3	1.3	12.0	15	0.2	1	375	1.2	0.07										
					total area	0.7	ha																							
XA 1		50	5.00		2 x 525	17.97	30	16	10	60	63	8	2.5	7.8	20	2.6	1	750	1.2	0.38										
					total area	18.0	ha																							
YA 1		70	5.00		651	30	11	10	60	42	5	3.5	7.0	20	1.2	1	600	1.2	0.22	1 x 600mm dia pipe		16	6	1	23					
					total area	6.5	ha															\$16	\$6	\$1	\$23					
AAA 1		200	0.25		525	1.63	30	6	10	60	25	3	4.5	5.9	25	0.4	1	450	1.2	0.30	1 x 450mm dia pipe		33	5	2	40				
AAA 2		280	0.25		675	5.65	30	10	10	60	40	5	4.0	5.7	20	1.4	1	825	1.2	0.73	1 x 825mm dia pipe		91	9	5	105				
AAA 3		110	0.25		1050	4.44	30	9	10	60	37	5	3.3	5.3	20	2.2	1	600	1.2	1.00	1 x 600mm dia pipe		25	6	2	33				
AAA 4		145	0.25		1200	2.84	30	8	10	60	31	4	3.1	5.0	20	2.7	1	750	1.4	1.00	1 x 750mm dia pipe		37	7	2	46				
AAA 5		205	0.25		1350	3.04	30	8	10	60	32	4	2.7	5.2	20	3.2	1	675	1.50	1.00	1 x 675mm dia pipe		53	7	3	63				
AAA 6		150	0.25		1350	9.07	5	12	20	75	48	6	2.2	5.0	90	3.9	1	675	1.50	1.00	1 x 675mm dia pipe		39	7	2	48				
AAA 7	AAA	190	1.00		600	2.45	15	7	5	80	29	4	3.7	4.7	20	0.3	1	450	1.2	0.15										
AAA 8		320	0.25		1350	5.05	5	10	20	75	39	5	1.8	4.0	90	4.7	1	1050	1.650	2.2	1.00	1 x 1050mm dia pipe		142	13	8	162			
AAA 9		280	0.25		1500	5.93	5	10	10	85	41	5	1.0	3.9	90	5.1	1	750	1.650	2.3	1.00	1 x 750mm dia pipe		81	8	4	93			

ILSAX RESULTS
1 IN 100 AEP FLOOD
EXISTING CATCHMENTS

Network	Reach Details		Catchment Area Parameters						Levels		Hydrology 1 in 100 AEP		Proposed Upgrade Options				Proposed Upgrade Works				Cost Breakdown				Total Cost (\$1000)						
	add hyd to	L S (%)	existing conduit size (mm)	area (ha)	perched direct (%)	perched time (min)	perched sup. (%)	grass time (%)	grass time (min)	grass lag (min)	invert level (m)	ground level (m)	critical duration (min)	flowrate (m ³ /h)	Extra Pipes no. dia. (mm)	New Pipes no. dia. (mm)	Open Channel w d (m)	Estement Width (m)	Description	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)									
AAA 9	50	0.25	1500	6.73	30	11	10	60	43	5	0.3	2.6	90	5.8	1	1050	1	1800	2.6	1.00	1 x 1050mm dia pipe	22	13	2	37						
AAAC 1	110	0.50	675	2.70	30	8	10	60	30	4	1.9	3.5	90	0.6	1	675	1	675	1.2	0.30											
AAAD 1 AAC	130	0.50	750	7.10	30	11	10	60	44	5	2.0	3.5	20	1.3	1	525	1	900	1.2	0.53	1 x 525mm dia pipe	25	6	2	33						
AAAC 2	140	0.50	900	2.02	30	7	10	60	27	3	1.4	3.3	20	2.2	1	1050	1	1050	1.2	0.80	1 x 600mm dia pipe	32	6	2	40						
AAAC 3 AAA	100	0.50	1050	2.78	30	8	10	60	31	4	0.7	2.1	25	2.8	1	600	1	1200	1.2	0.96	1 x 600mm dia pipe	23	6	1	30						
AAA 10	70	0.25	2x1200+1050	0.75	30	6	10	60	19	3	0.2	2.1	90	8.0	2	1500	3.3	1.00		\$603	\$93	\$35				\$731					
ABA 1	70	1.00	750	1.55	30	6	10	60	25	3	0.7	1.9	25	0.4	1	525	1.2	0.19													
				total area																											
ACA 1	100	0.50		4.54	30	9	10	60	37	5	0.5	1.8	20	0.9	1	750	1.2	0.41	1 x 750mm dia pipe	29	8	2	39								
				total area																	\$29	\$8	\$2				\$39				
ADA 1	70	0.50	525	1.72	30	6	10	60	26	3	0.4	1.4	20	0.4	1	525	1.2	0.23													
				total area																											
AEA 1	75	0.30		2.62	30	8	10	60	30	4	0.2	1.2	10	0.5	1	675	1.2	0.32	1 x 675mm dia pipe	19	7	1	28								
				total area																	\$19	\$7	\$1				\$28				
AFA 1	60	0.50		1.37	30	6	10	60	24	3	0.3	1.5	15	0.3	1	525	1.2	0.20	1 x 525mm dia pipe	12	6	1	19								
				total area																	\$12	\$6	\$1				\$19				
AGA 1	60	0.50		1.50	30	6	10	60	24	3	0.3	1.5	90	0.4	1	525	1.2	0.23	1 x 525mm dia pipe	12	6	1	19								
				total area																	\$12	\$6	\$1				\$19				
AHA 1	80	0.30		0.87	30	6	10	60	20	3	0.2	1.1	10	0.2	1	450	1.2	0.17	1 x 450mm dia pipe	13	5	1	19								
				total area																	\$13	\$5	\$1				\$19				
AJA 1	70	0.50		7.40	30	11	10	60	45	6	0.2	1.6	20	1.4	1	1050	1.2	0.68	1 x 1050mm dia pipe	31	13	2	46								
				total area																	\$31	\$13	\$2				\$46				
AJA 1	65	0.50		6.50	30	11	10	60	42	5	0.3	1.9	20	1.2	1	900	1.2	0.50	1 x 900mm dia pipe	24	10	2	36								
				total area																	\$24	\$10	\$2				\$36				
AKA 1	35	5.00		0.55	30	6	10	60	17	3	1.8	3.2	10	0.1	1	375	1.2	0.05	1 x 375mm dia pipe	5	4	0	9								
				total area																	\$5	\$4	\$0				\$9				
ALA 1	100	2.50		6.05	30	10	10	60	41	5	2.5	3.9	20	1.2	1	675	1.2	0.29	1 x 675mm dia pipe	26	7	2	34								
				total area																	\$26	\$7	\$2				\$34				
AM 1	90	3.00		5.03	30	10	10	60	39	5	2.7	3.9	20	1.0	1	600	1.2	0.23	1 x 600mm dia pipe	20	6	1	28								
				total area																	\$20	\$6	\$1				\$28				

ILSAX RESULTS
1 IN 100 AEP FLOOD
EXISTING CATCHMENTS

Network		Reach Details				Catchment Area Parameters						Levels			Hydrology 1 in 100 AEP			Proposed Upgrade Options				Proposed Upgrade Works			Cost Breakdown				Total Cost
branch & reach	add. hyd. to	L (m)	S (%)	existing conduit size (mm)	area (ha)	paved direct (%)	paved time (min)	paved imp. (%)	grass time (min)	grass lag (min)	grass maAHD (min)	invert level maAHD	ground level maAHD	critical duration (min)	flowrate (m ³ /s)	Extra Pipes no.	Pipes dia. (mm)	New Pipes no.	Pipes dia. (mm)	Open Channel W (m)	d (m)	Description	Easement Width (m)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)	Total Cost (\$1000)		
ANA 1	430	0.50	375	2.96	30	6	10	60	32	4	4.6	5.7	20	0.6	1	600	1	675	1.2	0.30	1 x 600mm dia pipe	96	96	6	5	109			
ANA 2	185	0.50	900	5.95	30	10	60	41	5	2.5	4.3	20	1.7	1	600	1	1050	1.2	0.65	1 x 600mm dia pipe	42	42	6	2	50				
ANA 3	270	0.50	525	13.68	30	14	60	56	7	2.9	5.6	20	2.2	1	1050	1	1050	1.2	0.80	1 x 1050mm dia pipe	120	120	13	7	139				
ANA 4	70	1.00	1050	13.33	30	14	60	56	7	1.6	3.7	20	5.9	1	900	1	1350	1.5	1.00	1 x 900mm dia pipe	26	26	10	2	37				
ANA 5	225	0.50	750	13.33	30	14	60	56	7	2.0	3.7	20	2.1	1	750	1	1050	1.2	0.77	1 x 750mm dia pipe	65	65	8	4	77				
ANA 6	85	1.00	1350	13.33	30	14	60	56	7	0.9	3.7	20	10.1	1	1050	1	1650	2.3	1.00	1 x 1050mm dia pipe	38	38	13	3	53				
					total area	62.5 ha											\$388	\$56	\$22	\$466									
AOA 1	80	0.20	571	5.71	30	10	60	40	5	0.2	1.5	20	1.1	1	1050	1	1050	1.2	0.66	1 x 1050mm dia pipe	35	35	13	2	51				
					total area	5.7 ha											\$35	\$13	\$2	\$51									
APA 1	215	0.35	600	4.80	30	9	60	38	5	3.9	5.2	20	0.9	1	600	1	825	1.2	0.46	1 x 600mm dia pipe	49	49	6	3	58				
APA 2	225	0.35	750	5.63	30	10	60	39	5	3.1	5.3	20	1.9	1	750	1	1050	1.2	0.81	1 x 750mm dia pipe	65	65	8	4	77				
APA 3	270	0.20	1050	9.40	30	12	60	49	6	2.9	4.6	20	1.6	1	600	1	1200	1.2	0.89	1 x 600mm dia pipe	61	61	6	3	71				
APA 4	360	0.35	1350	9.37	30	12	60	49	6	2.4	5.5	20	5.1	1	1650	1	1650	2.0	1.00	1 x 1650mm dia pipe	159	159	13	9	181				
APA 5	210	1.00	750	5.56	30	10	60	40	5	3.2	4.8	20	1.1	1	750	1	1500	2.9	1.00	1 x 1500mm dia pipe	188	188	13	10	211				
APA 6	265	0.35	1350	12.86	30	14	60	55	7	1.1	3.5	20	8.0	1	1500	2	1800	3.9	1.00	1 x 1500mm dia pipe	60	60	21	4	85				
APA 7	85	0.20	2 x 1350	7.88	25	11	5	70	46	6	0.2	1.8	20	8.9	1	1500	2	1800	3.9	1.00	1 x 1500mm dia pipe	\$582	\$67	\$32	\$682				
					total area	55.1 ha											\$211	\$58	\$13	\$282									
AQA 1	110	0.50	324	3.24	30	6	10	60	33	4	1.9	3.2	25	0.7	1	675	1	675	1.2	0.34	1 x 675mm dia pipe	28	28	7	2	37			
AQA 2	125	0.50	449	4.49	30	9	10	60	37	5	1.3	3.2	20	1.5	1	1050	1	1050	1.2	0.59	1 x 1050mm dia pipe	55	55	13	3	72			
AQA 3	305	0.50	525	2.97	30	8	10	60	32	4	2.2	3.7	20	0.6	1	450	1	675	1.2	0.30	1 x 450mm dia pipe	51	51	5	3	59			
AQA 4	110	0.50	375	4.36	30	9	10	60	36	5	0.7	3.0	20	2.9	1	1200	1	1200	1.2	0.99	1 x 1200mm dia pipe	58	58	15	4	77			
AQA 5	30	0.50	204	2.04	30	7	10	60	27	3	0.2	2.0	20	5.3	1	1350	1	1350	1.3	1.00	1 x 1350mm dia pipe	18	18	18	2	38			
					total area	17.1 ha											\$211	\$58	\$13	\$282									
ARA 1	170	1.50	375	1.30	30	6	10	60	23	3	3.8	5.2	15	0.3	1	375	1	450	1.2	0.13	1 x 375mm dia pipe	24	24	4	1	30			
ARA 2	60	2.00	450	1.62	30	6	10	60	25	3	1.2	5.2	25	0.7	1	375	1	525	1.2	0.21	1 x 375mm dia pipe	9	9	4	1	13			
					total area	2.9 ha											\$33	\$8	\$2	\$43									
ASA 1	30	5.00	0.62	0.62	30	6	10	60	17	3	1.5	4.8	25	0.2	1	375	1	375	1.2	0.07	1 x 375mm dia pipe	4	4	4	0	9			
					total area	0.6 ha											\$4	\$4	\$0	\$9									
AUA 1	30	5.00	1.72	1.72	30	6	10	60	26	3	1.5	4.6	20	0.4	1	375	1	375	1.2	0.11	1 x 375mm dia pipe	4	4	4	0	9			
					total area	1.7 ha											\$4	\$4	\$0	\$9									
AUA 2	590	0.30	693	6.93	30	11	10	60	44	5	3.9	5.3	20	1.3	1	1050	1	1050	1.2	0.65	1 x 1050mm dia pipe	261	261	13	14	288			
AUA 3	200	0.30	600	13.73	30	14	10	60	56	7	2.2	5.8	20	3.4	1	1500	1	1500	1.6	1.00	1 x 1500mm dia pipe	142	142	21	8	171			
AUA 4	225	0.90	581	5.81	30	10	10	60	41	5	4.6	5.8	20	1.1	1	750	1	750	1.2	0.38	1 x 750mm dia pipe	65	65	8	4	77			
AUA 5	110	0.90	768	7.68	30	11	10	60	45	6	2.6	6.0	20	2.5	1	1050	1	1050	1.2	0.70	1 x 1050mm dia pipe	49	49	13	3	65			
AUA 6	175	0.30	399	3.99	30	9	10	60	35	4	1.6	4.2	20	6.3	1	1800	1	1800	2.5	1.00	1 x 1800mm dia pipe	161	161	26	9	196			
AUA 7	260	1.50	525	2.45	30	7	10	60	29	4	5.0	6.0	15	0.5	1	525	1	525	1.2	0.19	1 x 525mm dia pipe	4	4	4	0	9			
AUA 8	125	2.50	525	3.49	30	8	10	60	34	4	4.2	5.1	20	0.7	1	525	1	525	1.2	0.20	1 x 525mm dia pipe	235	235	23	13	270			
AUA 9	290	0.30	1050	7.93	30	11	10	60	46	6	1.1	4.9	20	8.6	1	1650	2	1500	3.2	1.00	1 x 1650mm dia pipe	235	235	23	13	270			
AUA 10	150	1.20	1050	2.74	30	8	10	60	31	4	3.7	4.6	20	0.6	1	525	1	525	1.2	0.23	1 x 525mm dia pipe	4	4	4	0	9			

**ILSAX RESULTS
1 IN 100 AEP FLOOD
EXISTING CATCHMENTS**

Network		Reach Details		Catchment Area Parameters							Levels		Hydrology 1 in 100 AEP		Proposed Upgrade Options			Proposed Upgrade Works		Cost Breakdown				Total Cost					
branch & reach	add hyd to	L (mi)	S (%)	existing condit. size (mm)	area (ha)	paved direct (%)	paved time (min)	paved sup. (%)	grass (%)	grass time (min)	grass lag (min)	invert level m(AHD)	ground level m(AHD)	critical duration (min)	flowrate (m ³ /s)	Extra Pipes no.	New Pipes no.	dia. (mm)	dia. (mm)	Open Channel w (m)	Open Channel d (m)	Description	Easement Width (m)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Re-location (\$1000)	Total Cost (\$1000)		
AUE 2	AUA	145	1.20	1200	2.62	30	8	10	60	30	4	1.9	4.8	20	1.1	1	675	1.2	1800	3.9	1.00	2 x 1500mm dia pipe	65	\$997	\$146	42	6	133	
AUA 5		60	0.30	1350	8.23	30	12	10	60	46	6	0.2	5.5	25	10.9	2	1500	2	1800	3.9	1.00	2 x 1500mm dia pipe	85	\$997	\$146	42	6	\$1,200	
					65.6	ba																							
AVA 1		130	0.50	450	1.93	30	7	10	60	27	3	4.2	6.5	15	0.4	1	450	1	600	1.2	0.23	1 x 450mm dia pipe	42			5	2	49	
AVB 1	AVA	100	0.50	600	3.65	30	9	10	60	35	4	3.0	5.5	20	1.2	1	600	1	825	1.2	0.50	1 x 600mm dia pipe	52			6	3	61	
AVA 2		215	0.50	750	0.88	30	6	10	60	19	3	2.6	5.0	15	0.2	1	450	1	450	1.2	0.15	1 x 450mm dia pipe	17			5	1	23	
AVC 1	AVA	130	0.50	335	3.35	30	8	10	60	33	4	1.2	4.6	20	0.7	1	675	1	900	1.2	0.53	1 x 900mm dia pipe	79			10	4	93	
AVA 3		110	0.50	283	2.83	30	8	10	60	31	4	0.6	5.0	25	2.5	1	1200	1	1200	1.2	0.88	1 x 1200mm dia pipe	33			7	2	42	
					12.3	ba																							
AW 1		250	0.50	450	1.93	30	7	10	60	27	3	4.2	6.5	15	0.4	1	450	1	600	1.2	0.23	1 x 450mm dia pipe	42			5	2	49	
AW 2		230	0.50	600	3.65	30	9	10	60	35	4	3.0	5.5	20	1.2	1	600	1	825	1.2	0.50	1 x 600mm dia pipe	52			6	3	61	
AW 1	AW	160	0.50	750	0.88	30	6	10	60	19	3	2.6	5.0	15	0.2	1	450	1	450	1.2	0.15	1 x 450mm dia pipe	17			5	1	23	
AW 3		140	0.50	335	3.35	30	8	10	60	33	4	1.2	4.6	20	0.7	1	675	1	900	1.2	0.53	1 x 900mm dia pipe	79			10	4	93	
AW 4		225	0.50	900	4.98	20	9	5	75	34	4	1.1	4.0	20	2.7	1	825	1	1200	1.2	0.93	1 x 825mm dia pipe	73			9	4	86	
					17.0	ba																							
					113.5	ba																							

SUMMARY OF 2(6) AREA 2(6) AREA = 413 ba
2(6) AREA COST = 413 / 1137 x 23,698 K = \$8,608 K
total area 113.5 ba \$18,735 \$1,835 \$1,128 \$23,698

APPENDIX B

ILSAX RESULTS
1 IN 100 AEP FLOOD
PROPOSED CATCHMENTS

Network	Reach Details				Catchment Area Parameters						Levels			Hydrology I in 100 AEP			Proposed Upgrade Options				Proposed Upgrade Works			Cost Breakdown			Total Cost (\$1000)				
	add to	hybrid	L (%)	S (%)	existing conduit size (mm)	area (ha)	paved direct (%)	paved time (min)	paved sup. (%)	grass (%)	grass time (min)	grass lag (min)	invert level (m)	ground level (m)	critical duration (min)	flowrate (m ³ /s)	Extra no. (mm)	Pipes no. (mm)	New Pipes no. (mm)	Open Channel w (m)	d (m)	Description	Easement Width (m)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)					
NA 1			355	0.30	no existing	6.31	60	11	20	20	42	5	4.1	5.0	20	2.3	1	1200	1	1200	1.2	1.00	1 x 1200mm dia pipe	-	187	18	10	215			
NA 2			265	0.40	525mm pipe	5.74	60	10	20	41	5	3.0	3.9	20	4.2	1	1500	1	1500	1.7	1.00	1 x 1500mm dia pipe	-	188	21	10	219				
NA 3			305	0.40	no existing	3.79	60	9	20	35	4	3.6	4.5	20	1.5	1	1050	1	1050	1.2	0.65	1 x 1050mm dia pipe	-	135	13	7	156				
NA 4			190	0.30	no existing	3.22	60	8	20	33	4	2.3	3.2	25	2.7	1	1350	1	1350	1.4	1.00	1 x 1350mm dia pipe	-	80	18	5	102				
NA 5			210	0.50	no existing	3.56	60	8	20	28	34	4	2.0	3.0	25	8.1	1	1800	1	1800	2.5	1.00	2.5m wide channel	5.5	186	42	11	239			
NA 6			700	0.45	no existing	7.23	60	11	20	28	44	6	4.1	5.1	20	2.6	1	1200	1	1200	1.2	0.95	1 x 1200mm dia pipe	22.6	369	15	19	403			
NA 7			115	0.50	no existing	5.16	60	10	20	39	5	0.9	3.5	20	12.3	2	1500	2	1500	3.5	1.00	3.5m wide channel	-	116	57	9	182				
NA 8			345	0.50	no existing	5.99	60	10	20	40	5	4.5	5.5	20	2.0	1	1050	1	1050	1.2	0.74	1 x 1050mm dia pipe	-	153	13	8	174				
NA 9			110	0.50	no existing	3.33	60	6	20	20	20	2	2.8	5.0	25	3.4	1	1350	1	1350	1.3	1.00	1 x 1350mm dia pipe	-	67	18	4	90			
NA 10			345	0.70	no existing	3.22	60	8	20	33	4	4.6	5.5	20	2.0	1.3	825	1	825	1.2	0.47	1 x 825mm dia pipe	-	112	9	6	128				
NA 11			115	0.50	750mm pipe	5.16	60	10	20	39	5	2.2	5.0	25	6.5	1	1500	1	1500	2.1	1.00	1 x 1500mm dia pipe	-	93	23	6	122				
NA 12			350	0.50	no existing	4.25	60	9	20	36	5	3.4	5.5	20	1.6	1	1050	1	1050	1.2	0.63	1 x 1050mm dia pipe	-	155	13	8	176				
NA 13			265	0.50	no existing	6.08	60	10	20	20	41	5	1.7	4.1	25	10.3	2	1500	2	1500	3.1	1.00	3.1m wide channel	20.4	252	30	15	317			
NA 14			55	0.60	2 x 1350mm pipes	3.79	60	9	20	20	35	4	0.3	1.4	25	23.7	2	1500	3	1650	5.6	1.00	5.6m wide channel	8.6	70	86	8	164			
						total area		66.2 ha		2(b) area		66.2 ha		2(b) cost		\$2,488								\$2,164		\$396		\$128		\$2,688	
OA 1			590	0.15	no existing	12.40	50	14	20	30	54	7	4.4	5.4	20	3.3	1	1650	1	1650	2.0	1.00	2.0m wide channel	5.0	404	31	26	541			
OA 2			420	0.20	no existing	27.45	55	18	20	25	73	9	3.5	5.6	25	10.1	2	1800	2	1800	4.4	1.00	4.4m wide channel	17.5	469	65	27	561			
OA 3			390	0.20	no existing	19.87	60	16	20	20	65	8	2.7	6.2	25	15.5	3	1650	3	1650	6.3	1.00	6.3m wide channel	36.7	530	97	31	658			
OA 4			210	0.25	no existing	13.78	50	14	20	30	57	7	1.9	4.8	25	18.5	3	1650	3	1650	6.6	1.00	6.6m wide channel	29.6	295	102	20	417			
OA 5			240	0.28	no existing	5.28	50	10	15	35	39	5	4.0	5.0	20	1.7	1	1200	1	1200	1.2	0.93	1 x 1200mm dia pipe	-	126	15	7	149			
OB 1			140	0.65	no existing	4.02	55	9	20	25	35	4	3.5	5.5	25	3.0	1	1200	1	1200	1.2	0.91	1 x 1200mm dia pipe	-	74	15	4	93			
OB 2			320	0.40	no existing	2.64	50	8	15	35	30	4	3.9	5.0	20	0.9	1	825	1	825	1.2	0.45	1 x 825mm dia pipe	-	104	9	6	119			
OB 3			155	0.80	no existing	3.68	55	9	20	25	34	4	2.6	5.0	20	5.0	1	1350	1	1350	1.5	1.00	1 x 1350mm dia pipe	-	95	18	6	119			
OA 5			170	0.80	no existing	8.50	55	12	20	25	47	6	1.4	5.5	60	24.8	3	1650	3	1650	5.2	1.00	5.2m wide channel	42.9	208	79	14	301			
						total area		97.6 ha		2(b) area		90.5 ha		2(b) cost		\$2,741								\$2,385		\$431		\$141		\$2,957	
PA 1			180	0.20	no existing	6.20	50	10	10	40	42	5	4.4	5.4	20	1.9	1	1200	1	1200	1.2	1.00	1.2m wide channel	4.2	129	20	7	157			
PA 2			220	0.20	no existing	9.42	50	12	10	40	49	6	4.1	5.8	20	4.5	1	1650	1	1650	2.3	1.00	2.3m wide channel	10.9	188	37	11	236			
PA 3			425	0.20	no existing	11.72	50	13	10	40	53	7	3.6	5.8	20	7.5	2	1500	2	1500	3.4	1.00	3.4m wide channel	17.3	424	56	24	504			
PA 4			260	0.65	no existing	24.35	50	18	10	40	70	9	2.8	6.0	20	12.8	2	1500	2	1500	3.3	1.00	3.3m wide channel	29.8	255	55	15	325			
PA 5			110	1.00	no existing	11.03	50	13	10	40	52	6	1.1	6.8	25	15.5	2	1500	2	1500	3.2	1.00	3.2m wide channel	50.0	107	53	8	168			
						total area		62.7 ha		2(b) area		22.2 ha		2(b) cost		\$492								\$1,103		\$221		\$66		\$1,391	
OA 1			148	0.50	750mm pipe	5.24	50	10	10	40	39	5	1.3	2.3	20	1.6	1	750	1	1050	1.2	0.63	1 x 750mm dia pipe	-	40	6	2	49			
OA 2			100	0.60	900mm pipe	2.69	50	8	10	40	30	4	0.6	1.9	20	2.4	1	600	1	1050	1.2	0.79	1 x 600mm dia pipe	-	23	6	1	30			
						total area		7.9 ha		2(b) area		0.0 ha		2(b) cost		90								\$63		\$12		\$4		\$78	
						TOTAL SUMMARY		1029 ha		2(b) area		413 ha		2(b) cost		\$12,484								\$24,038		\$4,281		\$1,416		\$29,735	

APPENDIX C

-C1-

APPENDIX C

**RECOMMENDED SHORT/MEDIUM TERM UPGRADING OF
EXISTING DRAINAGE SYSTEM
FOR EXISTING LEVEL OF DEVELOPMENT**

**RECOMMENDED UPGRADING
SHORT TO MEDIUM TERM
EXISTING CATCHMENTS**

Network		Reach Details			Hydrology Peak Flow		Proposed Upgrade Options		Proposed Culverts	Cost Breakdown			Total Cost
branch & reach	add hyd to	L (m)	S (%)	existing conduit size (mm)	1 in 10 AEP (m ³ /s)	1 in 100 AEP (m ³ /s)	1 in 10 AEP	1 in 100 AEP	Size (mm)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)	(\$1000)
AA 1		335	0.20		675	1.4	2.1	1 x 825mm dia pipe	825	109	9	6	124
AB 1	AA	230	0.50		600	0.4	0.7						
AA 2		290	0.20	2 x 750	2.9	4.5		1 x 1200mm dia pipe	1200	153	15	8	176
AA 3		185	0.20	2 x 900	3.5	5.7		1 x 1350mm dia pipe	1350	113	18	7	138
AC 1	AA	190	0.50		750	0.6	0.9						
AA 4		130	0.20	2 x 900	4.4	7.3		3.3m wide channel	3300 BC	129	50	9	187
AA 5		220	0.20	2 x 900	4.7	7.7		3.5m wide channel	3500 BC	222	55	14	290
AD 1		585	0.20		600	1.5	2.3	1 x 1050mm dia pipe	1050	259	13	14	286
AE 1	AD	130	0.20		600	1.0	1.5	1 x 1050mm dia pipe	1050	58	13	4	74
AD 2		255	0.20	1050	2.8	4.5		1 x 1350mm dia pipe	1350	156	18	9	183
AF 1	AD	90	1.00	1050	0.5	0.8							
AD 3		355	0.20	1200	3.1	5.2		1 x 1350mm dia pipe	1350	217	18	12	247
AG 1	AD	190	0.20	oc	1.5	2.3			3900 BC		64	1	65
AD 4	AA	205	0.20	oc	4.6	7.7							
AA 6		490	0.10	oc	9.7	16.1							
AH 1	AA	310	3.80		900	0.9	1.4						
AJ 1		60	0.20		750	0.2	0.5						
AJ 1	AI	40	0.30		750	0.4	0.6						
AK 1	AI	50	0.40		825	0.6	0.9						
AL 1		180	0.15		975	0.7	1.0						
AL 2		120	0.15		1275	1.1	1.8						
AL 3		110	0.15		1500	1.8	2.8						
AL 4		100	0.15		1950	2.2	3.5						
AL 5		290	0.15		1800	2.5	4.1						
AL 6	AI	50	0.15		1800	2.8	4.6						
AI 2		40	0.50	2 x 1350	0.4	1.3							
AM 1	AI	60	0.50		600	0.4	0.6						
AN 1	AI	130	0.27		825	0.6	1.0						
AI 3		10	0.50	3 x 1350	0.9	2.0							
AI 4		20	0.50	3 x 1350	0.8	2.0							
AI 5		20	0.50	3 x 1350	0.5	1.7							
AI 6		10	0.50	3 x 1350	0.5	1.5							
AI 7		10	0.50	3 x 1350	0.5	1.5							
AI 8		30	0.30	2x1050 + 825	0.5	1.5							
AO 1	AI	30	1.00		900	0.2	0.5						
AI 9	AA	80	0.30	2x1050 + 825	0.5	1.5	1 x 1050mm dia pipe	1050	35	13	3	51	
AA 7		250	0.10	oc	10.6	18.2		13800 BC		200	7	207	
AP 1		120	5.00		900	1.9	3.0	1 x 600mm dia pipe		27	1	29	
AO 1	AP	200	5.00		750	1.1	1.7	1 x 525mm dia pipe		39	2	41	
AP 2		290	1.00	1200	3.6	5.5		1 x 1350mm dia pipe	675	177	6	9	193
AR 1		95	5.00			0.7	1.1	1 x 750mm dia pipe	450	27	6	2	35
AR 2	AP	215	1.80	1200	1.3	2.1							
AP 3		185	1.00	oc	6.0	9.3		3.4m wide channel		183	31	11	225
AP 4	AA	215	0.20	1800	5.9	9.3		2 x 1800mm dia pipe		395	23	21	439
AA 8		230	0.10	oc	15.2	27.0			22000 BC		305	10	315
AS 1	AA	470	1.70		600	0.6	1.0	1 x 675mm dia pipe		121	4	6	131
AA 9		290	0.10	oc	16.0	29.1			23000 BC		317	10	327
AT 1		335	0.50	oc	0.4	0.6							
AT 2	AA	245	0.50	oc	1.3	2.2							
AA 10		175	0.10	oc	17.9	32.8							
AU 1		710	0.40		750	0.4	0.7						
AU 2	AA	220	0.40	1500	1.1	1.8							
AV 1	AA	235	1.00		0.2	0.4	1 x 375mm dia pipe	375	68	4	4	76	
AA 11		60	0.10	4 x 1500	18.7	34.7							
CATCHMENT A SUMMARY										\$2,489	\$1,182	\$168	\$3,839
BA 1		100	0.40		0.6	1.0	1 x 675mm dia pipe	675	26	7	2	34	
BB 1	BA	90	0.40		0.7	1.1	1 x 825mm dia pipe	825	29	9	2	40	
BA 2		125	0.40		1.7	2.7	1 x 1200mm dia pipe	1200	66	15	4	85	
BA 3		65	0.40		2.1	3.3	1 x 1050mm dia pipe	1050	29	13	2	44	
BA 4		60	0.40		2.3	3.7	1 x 1050mm dia pipe	1050	27	13	2	42	
BA 5		60	0.40		2.7	4.3	1 x 1200mm dia pipe	1200	32	15	2	49	
BA 6		70	0.40		3.1	5.0	1 x 1200mm dia pipe	1200	37	15	3	54	
BA 7		140	0.40		3.4	5.5	1 x 1350mm dia pipe	1350	86	18	5	109	
BA 8		240	0.10		3.7	6.0	2.6m wide channel	1800	214	26	12	252	
BA 9		165	0.40		4.1	6.8	1.5m wide channel	1800	125	26	8	159	
CATCHMENT B SUMMARY										\$669	\$157	\$41	\$868

**RECOMMENDED UPGRADING
SHORT TO MEDIUM TERM
EXISTING CATCHMENTS**

Network		Reach Details			Hydrology Peak Flow		Proposed Upgrade Options		Proposed Culverts	Cost Breakdown			Total Cost
branch & reach	add hyd to	L (m)	S (%)	existing conduit size (mm)	1 in 10 AEP (m ³ /s)	1 in 100 AEP (m ³ /s)	1 in 10 AEP	1 in 100 AEP	Size (mm)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)	(\$1000)
CA 1		165	0.50	750	0.4	0.6							
CA 2		225	0.50	900	0.8	1.3							
CA 3		75	0.50	1050	1.5	2.5							
CATCHMENT C SUMMARY													
DA 1		70	0.20	675	0.5	0.8	1 x 375mm dia pipe		375	10	4	1	15
DA 2		65	0.20	675	0.7	1.1	1 x 525mm dia pipe		525	13	6	1	20
CATCHMENT D SUMMARY										\$23	\$10	\$2	\$34
EA 1		60	0.50	750	0.3	0.5							
CATCHMENT E SUMMARY													
FA 1		70	0.10	1050	0.5	0.8							
FB 1	FA	80	0.1		0.2	0.3	1 x 600mm dia pipe		600	18	6	1	25
FA 2		145	0.1	1050	0.9	1.4							
FA 3		10	0.10	1050	1.2	1.8	1 x 600mm dia pipe		600	2	6	0	9
CATCHMENT F SUMMARY										\$20	\$12	\$2	\$34
GA 1		50	0.50	525	0.1	0.3							
GA 2		20	0.50	525	0.2	0.4							
CATCHMENT G SUMMARY													
HA 1		40	1.00		0.2	0.4	1 x 375mm dia pipe		375		4	0	4
CATCHMENT H SUMMARY											\$4	\$0	\$4
IA 1		90	0.10	600	0.7	1.1		1 x 1050mm dia pipe	1050	40	13	3	56
IB 1	IA	155	1.00	450	0.7	1.1	1 x 450mm dia pipe		450	26	5	2	32
IA 2		285	0.10	900	1.9	2.9		1 x 1500mm dia pipe	1500	202	21	11	234
IA 3		185	0.10		2.7	4.3		2 x 1500mm dia pipe	2x1500	262	42	15	319
IA 4		90	0.10		3.0	4.9		2 x 1500mm dia pipe	2x1500	127	42	8	178
IC 1	IA	170	0.80	450	0.4	0.6	1 x 375mm dia pipe		375	24	4	1	30
IA 5		165	0.10	oc	3.6	5.9	2.5m wide channel		675	146	7	8	161
IA 6		160	0.10	oc	4.1	6.9	2.8m wide channel			147		7	154
IA 7		100	0.10	oc	4.1	7.1	2.8m wide channel			92		5	96
ID 1	IA	260	0.25		0.1	0.2	1 x 375mm dia pipe		375	67	4	4	74
IE 1	IA	105	0.75		0.1	0.2	1 x 375mm dia pipe		375	27	4	2	33
IA 8		115	0.10	oc	4.3	7.6	2.9m wide channel			107		5	112
IA 9		20	0.10	2x1000x1800	4.3	7.8		1 x 1200mm dia pipe	1200	11	15	1	27
CATCHMENT I SUMMARY										\$1,277	\$157	\$72	\$1,506
JA 1		140	0.30		0.2	0.4	1 x 525mm dia pipe		525	27	6	2	35
JA 2		20	0.30		0.5	0.8	1 x 675mm dia pipe		675	5	7	1	13
CATCHMENT J SUMMARY										\$33	\$13	\$2	\$48
KA 1		80	0.15		0.1	0.1	1 x 375mm dia pipe		375		4	0	4
CATCHMENT K SUMMARY											\$4	\$0	\$4
LA 1		70	0.20		0.1	0.3	1 x 450mm dia pipe		450	12	5	1	18
CATCHMENT L SUMMARY										\$12	\$5	\$1	\$18
MA 1		70	0.20		0.3	0.5	1 x 600mm dia pipe		600	16	6	1	23
MA 2		120	0.20		0.4	0.7	1 x 675mm dia pipe		675	31	7	2	40
MB 1		115	0.10	375	0.1	0.1					4	0	4
MA 3		80	0.10	450	0.5	0.9	1 x 750mm dia pipe		750	23	9	2	34
CATCHMENT M SUMMARY										\$70	\$26	\$5	\$101

**RECOMMENDED UPGRADING
SHORT TO MEDIUM TERM
EXISTING CATCHMENTS**

Network		Reach Details			Hydrology Peak Flow		Proposed Upgrade Options		Proposed Culverts	Cost Breakdown			Total Cost
branch & reach	add hyd to	L (m)	S (%)	existing conduit size (mm)	1 in 10 AEP (m ³ /s)	1 in 100 AEP (m ³ /s)	1 in 10 AEP (m ³ /s)	1 in 100 AEP (m ³ /s)	Size (mm)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)	(\$1000)
NA 1		110	0.30		0.1	0.3	1 x 375mm dia pipe		375		4	0	4
NA 2		88	0.30		0.2	0.5	1 x 450mm dia pipe		450	15	5	1	21
NA 3		58	0.30		0.2	0.6	1 x 525mm dia pipe		525	11	6	1	18
NB 1		57	0.15		0.1	0.3	1 x 450mm dia pipe		450	10	5	1	15
NB 2		68	0.15		0.2	0.5	1 x 525mm dia pipe		525	13	6	1	20
NC 1	NB	36	0.15		0.2	0.1	1 x 525mm dia pipe		525	7	6	1	14
NB 3	NA	53	0.15		0.5	0.7	1 x 750mm dia pipe		750	15	8	1	24
NA 4		46	0.10		0.8	1.4	1 x 1050mm dia pipe		1050	20	13	2	35
ND 1		77	0.80		0.1	0.2	1 x 375mm dia pipe		375	34	4	2	40
ND 2	NA	38	0.80		0.3	0.6	1 x 450mm dia pipe		450	6	5	1	12
NA 5		131	0.10		1.2	2.1	1 x 1200mm dia pipe		1200	69	15	4	88
NE 1	NA	114	0.15	375	0.1	0.2					4	0	4
NA 6		104	0.10		1.4	2.4	1 x 1200mm dia pipe		1200	55	15	3	73
CATCHMENT N SUMMARY										\$256	\$96	\$18	\$370
OA 1		120	0.80	750	0.3	0.6							
OA 2		155	0.80	750	0.4	0.7							
OB 1	OA	150	0.10	375	0.1	0.2	1 x 375mm dia pipe		375	21	4	1	27
OA 3		80	0.20		0.7	1.3	1 x 825mm dia pipe		825	26	9	2	37
CATCHMENT O SUMMARY										\$48	\$13	\$3	\$64
PA 1		100	0.50		1.4	2.1	1 x 1050mm dia pipe		1050	44	13	3	60
CATCHMENT P SUMMARY										\$44	\$13	\$3	\$60
QA 1		195	0.70		0.3	0.5	1 x 450mm dia pipe		450	33	5	2	39
QB 1	QA	95	0.40	675	0.2	0.4							
QA 2		130	0.40	675	0.6	1.1							
QA 3		175	0.40	750	1.0	2.1	1 x 1050mm dia pipe		1050	78	13	5	95
QA 4		190	0.40	900	1.2	2.7	1 x 825mm dia pipe		825	62	9	4	74
QC 1		135	0.35	750	0.7	1.1							
QC 2	QA	195	0.35	750	1.0	1.6	1 x 750mm dia pipe		750	56	8	3	68
QA 5		115	0.10	2 x 900	2.3	4.4	1 x 1500mm dia pipe		1500	81	21	5	108
QD 1		185	0.50	900	0.5	0.8							
QD 2	QA	305	0.70	1050	0.7	1.1							
QE 1		255	0.50	600	0.7	1.2	1 x 375mm dia pipe		375	36	4	2	42
QF 1	QE	90	1.00	525	0.3	0.5							
QE 2	QA	310	0.50		1.9	3.0	1 x 1050mm dia pipe		1050	137	13	8	158
QA 6		405	0.10	open channel	5.2	9.3	3.4m wide channel			401		20	421
QA 7		120	0.10	1050 + 1200	5.8	10.2	2 x 1800mm dia pipe	2x1800	220	52	14	286	
CATCHMENT Q SUMMARY										\$1 105	\$125	\$62	\$1 292
RA 1		135	0.30	750	0.8	1.3	1 x 750mm dia pipe		750	39	8	2	49
RA 2		80	0.50	900	1.0	1.6	1 x 600mm dia pipe		600	18	6	1	25
CATCHMENT R SUMMARY										\$57	\$14	\$4	\$75
SA 1		285	0.25		0.6	0.9	1 x 750mm dia pipe		750	82	8	5	95
SA 2		345	0.25		1.8	2.9	1 x 1200mm dia pipe		1200	182	15	10	207
SA 3		325	0.25		3.3	5.2	1 x 1650mm dia pipe		1650	263	23	14	300
SA 4		200	0.25	900	4.1	6.6	1 x 1800mm dia pipe		1800	184	26	10	220
SB 1		250	0.50		0.3	0.5	1 x 600mm dia pipe		600	57	6	3	66
SB 2	SA	100	0.50		0.5	0.9	1 x 750mm dia pipe		750	29	8	2	39
SA 5		195	0.25	1200	5.0	8.0	1 x 1800mm dia pipe		1800	179	26	10	215
SA 6		160	0.25	1350	5.1	8.3	1 x 1650mm dia pipe		1650	129	23	8	160
SC 1		175	0.20		0.4	0.7	1 x 675mm dia pipe		675	45	7	3	55
SC 2		235	0.20		1.2	1.9	1 x 1050mm dia pipe		1050	104	13	6	123
SC 3		240	0.20		2.1	3.3	1 x 1350mm dia pipe		1350	147	18	8	173
SC 4		90	0.20		3.0	4.9	1 x 1500mm dia pipe		1500	64	21	4	89
SC 5		280	0.20	1200	3.7	6.0	1 x 1200mm dia pipe		1200	148	15	8	171
SC 6		110	0.20	1350	4.6	7.4	1 x 1200mm dia pipe		1200	58	15	4	77
SC 7	SA	135	0.20	1500	4.7	7.6	1 x 1500mm dia pipe		1500	96	21	6	122
SA 7		85	0.25	1800	10.0	16.2	2 x 1500mm dia pipe	2x1500	120	42	8	8	170
SA 8		130	0.25	1800	10.1	16.4	1 x 1650mm dia pipe		1800	105	23	6	135
SD 1	SA	220	0.70	675	0.5	0.8			600				
SA 9		220	0.25	1800	10.7	17.6	1 x 1650mm dia pipe		1800	178	23	10	211
SA 10		60	0.25	1800	10.9	18.1	2 x 1500mm dia pipe	2x1500	85	42	6	6	133
CATCHMENT S SUMMARY										\$2 254	\$375	\$131	\$2 761

**RECOMMENDED UPGRADING
SHORT TO MEDIUM TERM
EXISTING CATCHMENTS**

Network		Reach Details			Hydrology Peak Flow		Proposed Upgrade Options		Proposed Culverts	Cost Breakdown			Total Cost
branch & reach	add hyd to	L (m)	S (%)	existing conduit size (mm)	1 in 10 AEP (m ³ /s)	1 in 100 AEP (m ³ /s)	1 in 10 AEP (m ³ /s)	1 in 100 AEP (m ³ /s)	Size (mm)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)	(\$1000)
TA 1		285	0.30	600	1.5	2.3	1 x 900mm dia pipe		900	105	10	6	120
TA 2		170	0.30	600	2.1	3.2	1 x 1050mm dia pipe		1050	75	13	4	93
TB 1	TA	50	2.00	600	0.3	0.5							
TA 3		120	0.30		2.9	4.7	1 x 1650mm dia pipe		1650	97	23	6	126
TA 4		30	0.30		3.0	4.9	1 x 1650mm dia pipe		1650	24	23	2	50
CATCHMENT T SUMMARY										\$301	\$69	\$19	\$389
UA 1		65	0.50	450 + 600	0.5	0.7							
CATCHMENT U SUMMARY													
VA 1		75	1.00	525	0.2	0.4							
CATCHMENT V SUMMARY													
WA 1		25	5.00	375	0.1	0.2							
CATCHMENT W SUMMARY													
XA 1		50	5.00	2 x 525	1.7	2.6							
CATCHMENT X SUMMARY													
YA 1		70	5.00		0.8	1.2	1 x 450mm dia pipe		450	12	5	1	18
CATCHMENT Y SUMMARY										\$12	\$5	\$1	\$18
AAA 1		200	0.25	525	0.2	0.4							
AAA 2		280	0.25	675	0.9	1.4	1 x 600mm dia pipe		600	64	6	3	73
AAA 3		110	0.25	1050	1.3	2.2							
AAA 4		145	0.25	1200	1.6	2.7							
AAA 5		205	0.25	1350	2.0	3.2							
AAA 6		150	0.25	1350	2.1	3.9							
AAB 1	AAA	190	1.00	600	0.2	0.3							
AAA 7		320	0.25	1350	2.4	4.7							
AAA 8		280	0.25	1500	2.4	5.1							
AAA 9		50	0.25	1500	2.7	5.8							
AAC 1		110	0.50	675	0.3	0.6							
AAD 1	AAC	130	0.50	750	0.8	1.3							
AAC 2		140	0.50	900	1.4	2.2							
AAC 3	AAA	100	0.50	1050	1.6	2.8							
AAA 10		70	0.25	2x1200+1050	3.8	8.0							
CATCHMENT AA SUMMARY										\$64	\$6	\$3	\$73
ABA 1		70	1.00	750	0.2	0.4							
CATCHMENT AB SUMMARY													
ACA 1		100	0.50		0.6	0.9	1 x 675mm dia pipe		675	26	7	2	34
CATCHMENT AC SUMMARY										\$26	\$7	\$2	\$34
ADA 1		70	0.50	525	0.2	0.4							
CATCHMENT AD SUMMARY													
AEA 1		75	0.30		0.3	0.5	1 x 600mm dia pipe		600	17	6	1	24
CATCHMENT AE SUMMARY										\$17	\$6	\$1	\$24
AFA 1		60	0.50		0.2	0.3	1 x 450mm dia pipe		450	10	5	1	16
CATCHMENT AF SUMMARY										\$10	\$5	\$1	\$16
AGA 1		60	0.50		0.2	0.4	1 x 450mm dia pipe		450	10	5	1	16
CATCHMENT AG SUMMARY										\$10	\$5	\$1	\$16
AHA 1		80	0.30		0.1	0.2	1 x 375mm dia pipe		375	4	0	4	4
CATCHMENT AH SUMMARY										\$4	\$0	\$4	\$4
AIA 1		70	0.30		0.9	1.4	1 x 825mm dia pipe		825	23	9	2	33
CATCHMENT AI SUMMARY										\$23	\$9	\$2	\$33

**RECOMMENDED UPGRADING
SHORT TO MEDIUM TERM
EXISTING CATCHMENTS**

Network		Reach Details			Hydrology Peak Flow		Proposed Upgrade Options		Proposed Culverts	Cost Breakdown			Total Cost
branch & reach	add hyd to	L (m)	S (%)	existing conduit size (mm)	1 in 10 AEP (m3/s)	1 in 100 AEP (m3/s)	1 in 10 AEP (m3/s)	1 in 100 AEP (m3/s)	Size (mm)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)	(\$1000)
AJA 1		65	0.50		0.8	1.2	1 x 750mm dia pipe		750	19	8	1	28
CATCHMENT AJ SUMMARY										\$19	\$8	\$1	\$28
AKA 1		35	5.00		0.1	0.1	1 x 375mm dia pipe		375		4	0	4
CATCHMENT AK SUMMARY											\$4	\$0	\$4
ALA 1		100	2.50		0.7	1.2	1 x 525mm dia pipe		525	20	6	1	27
CATCHMENT AL SUMMARY										\$20	\$6	\$1	\$27
AM 1		90	3.00		0.6	1.0	1 x 450mm dia pipe		450	15	5	1	21
CATCHMENT AM SUMMARY										\$15	\$5	\$1	\$21
ANA 1		430	0.50	375	0.4	0.6	1 x 375mm dia pipe		375	61	4	3	69
ANA 2		185	0.50	900	1.1	1.7							
ANB 1	ANA	270	0.50	525	0.6	2.2	1 x 450mm dia pipe		450	45	5	3	53
ANA 3		70	1.00	1050	2.3	5.9	1 x 900mm dia pipe		900	26	10	2	37
ANC 1	ANA	225	0.50	750	0.8	2.1							
ANA 4		85	1.00	1350	3.6	10.1	1 x 1050mm dia pipe		1050	38	13	3	53
CATCHMENT AN SUMMARY										\$170	\$32	\$10	\$212
AOA 1		80	0.20		0.7	1.1	1 x 825mm dia pipe		825	26	9	2	37
CATCHMENT AO SUMMARY										\$26	\$9	\$2	\$37
APA 1		215	0.35	600	0.6	0.9	1 x 600mm dia pipe		600	49	6	3	58
APA 2		225	0.35	750	1.2	1.9	1 x 750mm dia pipe		750	65	8	4	77
APB 1	APA	270	0.20	1050	1.0	1.6	1 x 600mm dia pipe		600	61	6	3	71
APA 3		360	0.35	1350	3.2	5.1	1 x 1050mm dia pipe		1050	159	13	9	181
APC 1	APA	210	1.00	750	0.7	1.1							
APA 4		265	0.35	1350	5.0	8.0	1 x 1500mm dia pipe		1500	188	13	10	211
APA 5		85	0.20	2 x 1350	5.6	8.9	1 x 1500mm dia pipe		1500	60	21	4	85
CATCHMENT AP SUMMARY										\$582	\$67	\$32	\$682
AQA 1		110	0.50		0.4	0.7	1 x 525mm dia pipe		525	22	6	1	29
AQA 2		125	0.50		1.0	1.5	1 x 750mm dia pipe		750	41	9	2	52
AQB 1	AQA	305	0.50	525	0.4	0.6							
AQA 3		110	0.50	375	1.8	2.9	1 x 1050mm dia pipe		1050	49	13	3	65
AQA 4		30	0.50		2.1	3.3	1 x 1350mm dia pipe		1350	18	18	2	38
CATCHMENT AQ SUMMARY										\$129	\$46	\$9	\$184
ARA 1		170	1.50	375	0.2	0.3							
ARA 2		60	2.00	450	0.4	0.7							
CATCHMENT AR SUMMARY													
ASA 1		30	5.00		0.1	0.2	1 x 375mm dia pipe		375		4	0	4
CATCHMENT AS SUMMARY											\$4	\$0	\$4
ATA 1		30	5.00		0.2	0.4	1 x 375mm dia pipe		375		4	0	4
CATCHMENT AT SUMMARY											\$4	\$0	\$4
AUA 1		590	0.30		0.8	1.3	1 x 825mm dia pipe		825	192	9	10	211
AUA 2		200	0.30	600	2.1	3.4	1 x 1050mm dia pipe		1050	89	13	5	107
AUB 1		225	0.90		0.7	1.1	1 x 600mm dia pipe		600	51	6	3	60
AUB 2	AUA	110	0.90		1.6	2.5	1 x 825mm dia pipe		825	36	9	2	47
AUA 3		175	0.30		4.0	6.3	1 x 1500mm dia pipe		1500	124	21	7	152
AUC 1	AUA	260	1.50	525	0.3	0.5							
AU 1	AUA	125	2.50	525	0.4	0.7							
AUA 4		290	0.30	1050	5.5	8.6	1 x 1350mm dia pipe		1350	177	18	10	205
AUE 1		150	1.20	1050	0.3	0.6							
AUE 2	AUA	145	1.20	1200	0.7	1.1							
AUA 5		60	0.30	1350	6.8	10.9	2 x 1500mm dia pipe	2x1500	85	42	6	133	
CATCHMENT AU SUMMARY										\$754	\$118	\$44	\$916

**RECOMMENDED UPGRADING
SHORT TO MEDIUM TERM
EXISTING CATCHMENTS**

Network		Reach Details			Hydrology Peak Flow		Proposed Upgrade Options		Proposed Culverts	Cost Breakdown			Total Cost
branch & reach	add hyd to	L (m)	S (%)	existing conduit size (mm)	1 in 10 AEP (m3/s)	1 in 100 AEP (m3/s)	1 in 10 AEP	1 in 100 AEP	Size (mm)	Trunk Drainage (\$1000)	Proposed Culverts (\$1000)	Services Relocation (\$1000)	(\$1000)
AVA 1		130	0.50		0.5	0.7	1 x 600mm dia pipe		600	30	6	2	37
AVB 1	AVA	100	0.50		0.1	0.2	1 x 375mm dia pipe		375	14	4	1	19
AVA 2		215	0.50		0.8	1.3	1 x 750mm dia pipe		750	62	8	4	74
AVC 1	AVA	130	0.50		0.4	0.7	1 x 600mm dia pipe		600	30	6	2	37
AVA 3		110	0.50		1.5	2.5	1 x 1200mm dia pipe		1200	58	15	4	77
CATCHMENT AV SUMMARY										\$193	\$39	\$12	\$244
AW 1		250	0.50	450	0.3	0.4	1 x 375mm dia pipe		375	36	4	2	42
AW 2		230	0.50	600	0.7	1.2	1 x 375mm dia pipe		375	33	4	2	39
AW 1	AW	160	0.50	750	0.1	0.2							
AW 3		140	0.50	750	1.3	2.2	1 x 525mm dia pipe		525	27	6	2	35
AW 4		225	0.50	900	1.6	2.7	1 x 600mm dia pipe		600	51	6	3	60
CATCHMENT AW SUMMARY										\$147	\$20	\$8	\$176
TOTAL CATCHMENT SUMMARY										\$10,875	\$2,684	\$663	\$14,222

SUMMARY OF 2(b) AREA 2(b) AREA = 413 ha

2(b) AREA C = 413 / 1137 x 14,222 K = \$ 5,166 K

APPENDIX D

APPENDIX D

SECTION 94 DRAINAGE CONTRIBUTION CALCULATIONS

Existing Conditions - see Appendix C

TRUNK DRAINAGE

The 2(b) area drainage cost was proportioned from the total drainage cost by the percentage 2(b) area for the whole Peninsula.

Data: Total Area = 1137 ha
2(b) Area = 413 ha
Total Cost = \$14,222,000

2(b) Area Cost = $14,222,000 \times (413 / 1137)$
= \$5,166,000

Future Conditions - see Appendix B

The 2(b) area drainage cost in this case was calculated within the spreadsheet (see Appendix B). The cost was calculated separately for each catchment area, A to Q. Each of these were then totalled to give an overall estimate of 2(b) area cost.

2(b) Area Cost = \$12,484,000 ✓

Section 94 Contributions

The costs of drainage works in only the 2(b) areas were calculated for each of the two schemes above. The estimate of Section 94 Contributions is equal to the difference in cost of the two schemes, proportioned over the total area zoned 2(b).

[2(b) Cost difference / 2(b) Area] = $(12,484,000 - 5,166,000) / 413 = \$17,720 / \text{ha}$

MINOR DRAINAGE

Typical 3 hectare residential area was selected and the 1 in 10 AEP drainage works estimated and costed for zoning 2(a) and 2(b). The results are shown below:

Residential 2(a)

Pipe Size	Length	Amount	
375	60	8 580	
450	95	15 865	
600	110	24 970	
		<hr/>	
		\$49 415	
Equivalent to		\$16 471	per hectare
Services Adjustment (say 3%)		494	
		<hr/>	
		\$16 965	
	Say	\$17 000	per hectare

Residential 2(b)

Pipe Size	Length	Amount	
450	90	15 030	
525	60	11 760	
675	95	24 415	
825	110	35 860	
		<hr/>	
		\$87 065	
Equivalent to		\$29 021	per hectare
Services Adjustment (say 3%)		870	
		<hr/>	
		\$29 891	
	Say	\$30 000	per hectare

Section 94 contribution is thus (\$30 000 minus \$17 000) \$13 000 per hectare.

APPENDIX E

OPEN CHANNEL SURVEY CATCHMENT A



CROSS SECTION 1

CHAINAGE	R.L.	REMARKS
00.0	1.18	
10.0	1.33	TB
12.2	-0.22	EC
16.5	-0.30	IN
20.0	-0.24	EC
21.2	0.04	BOTT RW
21.4	0.96	TOP RW
23.9	1.03	FC
27.8	1.09	

CROSS SECTION 2

CHAINAGE	R.L.	REMARKS
00.0	1.18	FC
06.4	1.18	TB
08.8	-0.16	EC
11.1	-0.44	IN
13.3	-0.23	EC
15.8	1.42	TB
18.5	1.66	FC

CROSS SECTION 3

CHAINAGE	R.L.	REMARKS
00.0	1.62	
11.6	0.98	TB
12.4	0.00	EC
15.8	-0.44	IN
18.3	0.03	EC
18.7	0.69	TB
31.3	1.22	

CROSS SECTION 4

CHAINAGE	R.L.	REMARKS
00.0	1.58	FC
02.3	1.40	TB
05.4	0.47	
07.5	0.64	TB
08.3	0.09	EC
10.6	-0.10	IN
13.4	0.06	EC
14.0	0.54	TB
17.9	2.05	TB
26.7	1.78	

CROSS SECTION 5

CHAINAGE	R.L.	REMARKS
00.0	3.06	
09.8	2.90	TB
17.0	1.04	
21.3	0.64	TB
21.8	0.18	EC
24.7	0.15	IN
27.1	0.18	EC
29.3	1.78	TB
36.9	2.56	
47.4	2.15	

CROSS SECTION 6

CHAINAGE	R.L.	REMARKS
00.0	3.13	
11.5	2.48	TB
16.4	0.90	TB
17.3	0.36	EC
21.1	0.25	IN
26.3	0.66	EC
28.1	2.36	TB
43.8	3.29	

CROSS SECTION 7

CHAINAGE	R.L.	REMARKS
00.0	3.22	TK
10.1	1.93	TB
11.2	1.14	EC
12.8	0.27	IN
18.1	0.62	IN
21.2	1.08	EC
22.6	1.93	TB
30.3	3.29	TK

CROSS SECTION 8

CHAINAGE	R.L.	REMARKS
00.0	3.42	FC
02.6	3.40	
06.7	2.95	TB
10.5	1.90	TB
11.1	1.16	EC
14.1	0.48	IN
19.2	0.96	EC
19.4	1.47	TB
21.8	2.06	
26.7	3.27	FC

CROSS SECTION 9

CHAINAGE	R.L.	REMARKS
00.0	3.87	
09.1	3.74	TB
14.7	3.01	
17.6	2.78	TB
19.6	1.13	EC
22.9	0.51	IN
28.0	1.15	EC
30.3	1.53	
31.8	2.09	TB
34.4	2.24	
37.7	3.42	FC

CROSS SECTION 10

CHAINAGE	R.L.	REMARKS
00.0	3.44	
09.0	3.65	
11.2	3.48	TB
14.9	1.51	BB
18.0	1.17	EC
20.3	1.07	IN
23.0	1.18	EC
26.4	1.34	BB
33.0	3.25	FC

CROSS SECTION 11

CHAINAGE	R.L.	REMARKS
00.0	3.74	FC
04.7	3.50	TB
07.8	2.08	BB
12.8	2.00	TB
13.6	1.54	EC
16.3	1.28	IN
18.3	1.42	EC
20.9	3.42	TB
27.3	3.89	FC

CROSS SECTION 12

CHAINAGE	R.L.	REMARKS
00.0	3.78	FC
04.8	4.32	TB
08.0	1.81	BB
09.3	1.16	IN
11.3	1.50	IN
13.0	1.72	BB
16.1	4.32	TB
21.5	4.36	
27.6	3.71	

CROSS SECTION 13

CHAINAGE	R.L.	REMARKS
00.0	4.06	FC
02.7	3.81	TB
05.2	2.32	TB
05.8	1.68	EC
07.1	1.51	IN
08.3	1.65	EC
11.8	2.19	BB
13.4	3.64	TB
16.1	3.92	

CROSS SECTION 14

CHAINAGE	R.L.	REMARKS
00.0	4.04	FC
04.6	4.17	TB
06.8	2.51	BB
08.2	2.35	
08.8	1.94	EC
09.8	1.79	IN
12.3	1.95	EC
15.5	4.50	TB
26.4	3.86	FC

CROSS SECTION 15

CHAINAGE	R.L.	REMARKS
00.0	4.23	FC
01.7	3.87	TB
03.6	2.49	TB
03.9	2.07	EC
06.1	1.98	IN
10.1	2.39	BB
11.0	3.59	TB
14.0	4.15	
19.9	4.01	FC

CROSS SECTION 16

CHAINAGE	R.L.	REMARKS
00.0	5.52	
06.5	5.32	TB
11.9	2.56	BB
13.4	2.28	EC
15.4	2.12	IN
17.8	2.37	EC
19.6	3.76	TB
24.0	4.50	
31.7	4.04	

CROSS SECTION 17

CHAINAGE	R.L.	REMARKS
00.0	5.12	FC
06.0	5.05	
08.7	4.31	TB
09.7	2.95	BB
11.1	2.33	EC
13.8	2.11	IN
16.3	2.44	EC
17.5	2.85	BB
19.9	4.38	
24.9	5.33	TB
30.9	4.46	

CROSS SECTION 18

CHAINAGE	R.L.	REMARKS
00.0	5.03	FC
04.0	5.34	
09.3	5.35	TB
13.5	4.02	
14.7	2.70	EC
17.2	2.35	IN
20.0	2.62	EC
22.1	3.41	
23.8	4.65	TB
29.4	4.98	
30.6	5.43	
35.5	4.75	

CROSS SECTION 19

CHAINAGE	R.L.	REMARKS
00.0	5.08	FC
07.0	4.81	
08.8	4.14	TB
10.4	2.59	EC
13.4	2.26	IN
15.9	2.65	EC
17.3	4.12	TB
19.1	4.21	FC

CROSS SECTION 20

CHAINAGE	R.L.	REMARKS
00.0	5.33	FC
06.9	5.19	TB
09.0	3.83	
09.8	2.99	EC
12.6	2.83	IN
14.9	2.90	EC
16.6	3.90	TB
18.5	4.38	FC

CROSS SECTION 21

CHAINAGE	R.L.	REMARKS
00.0	5.83	
06.4	5.49	FC
08.1	5.27	TB
10.8	3.18	EC
14.1	2.66	IN
15.5	2.90	EC
17.9	4.36	TB
20.3	4.56	FC

CROSS SECTION 22

CHAINAGE	R.L.	REMARKS
00.0	4.82	EDGE SPOIL HEAP
10.2	5.07	FC / EDGE SPOIL HEAP
12.7	5.34	
15.7	4.80	TB
17.0	3.21	EC
18.9	3.21	IN
20.8	3.21	EC
21.9	4.62	TB
24.2	4.58	FC

CROSS SECTION 23

CHAINAGE	R.L.	REMARKS
00.0	5.10	FC
07.9	4.42	TB
09.0	2.93	EC
10.7	2.67	IN
12.8	3.12	EC
15.3	3.19	BB
18.0	5.06	TB
26.0	5.12	FC

CROSS SECTION 24

CHAINAGE	R.L.	REMARKS
00.0	3.59	
06.1	3.32	TB
07.0	2.81	EC
10.0	2.44	IN
11.8	2.79	EC
14.2	4.24	TB
20.7	4.36	
31.4	5.04	

CROSS SECTION 25

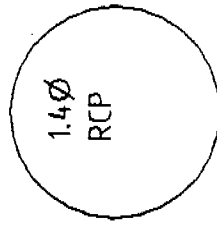
CHAINAGE	R.L.	REMARKS
00.0	4.18	FC
06.7	3.06	
09.1	2.69	TB
09.6	2.13	EC
11.0	2.06	IN
12.8	2.18	EC
14.8	3.51	TB
19.0	3.86	FC

CULVERT UNDER WOY-WOY ROAD

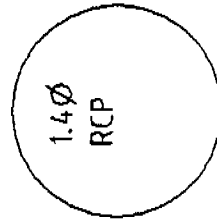
LEVEL ON ROAD 1.68

R.L. 1.46

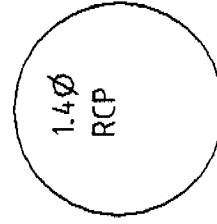
R.L. 1.51



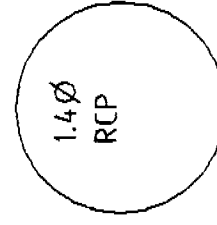
I.L. -0.32
I.L. -0.45(UP)



I.L. -0.32
I.L. -0.47(UP)



I.L. -0.34
I.L. -0.47(UP)

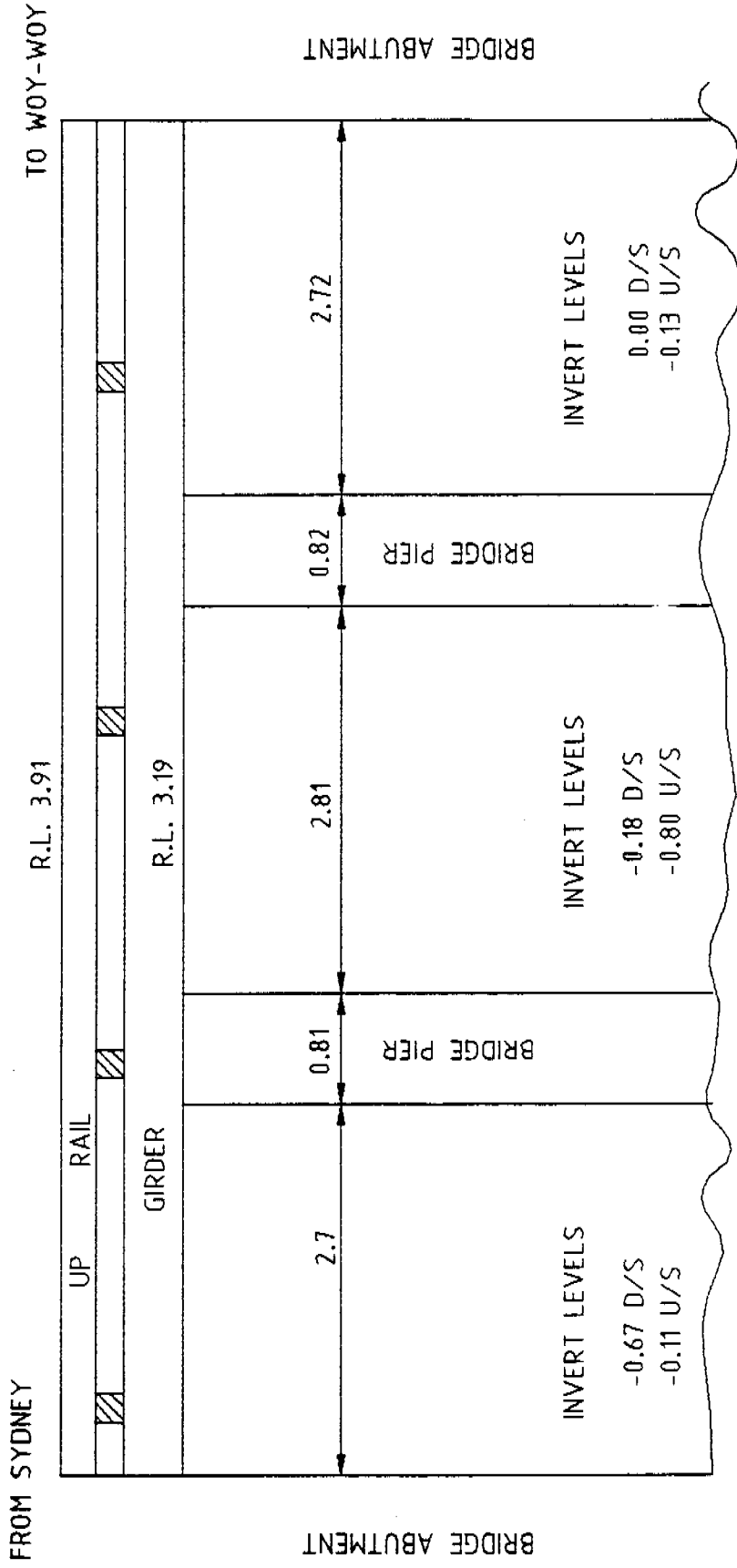


I.L. -0.37
I.L. -0.48(UP)

PIPE ABT 12.6m LONG

LOOKING UPSTREAM

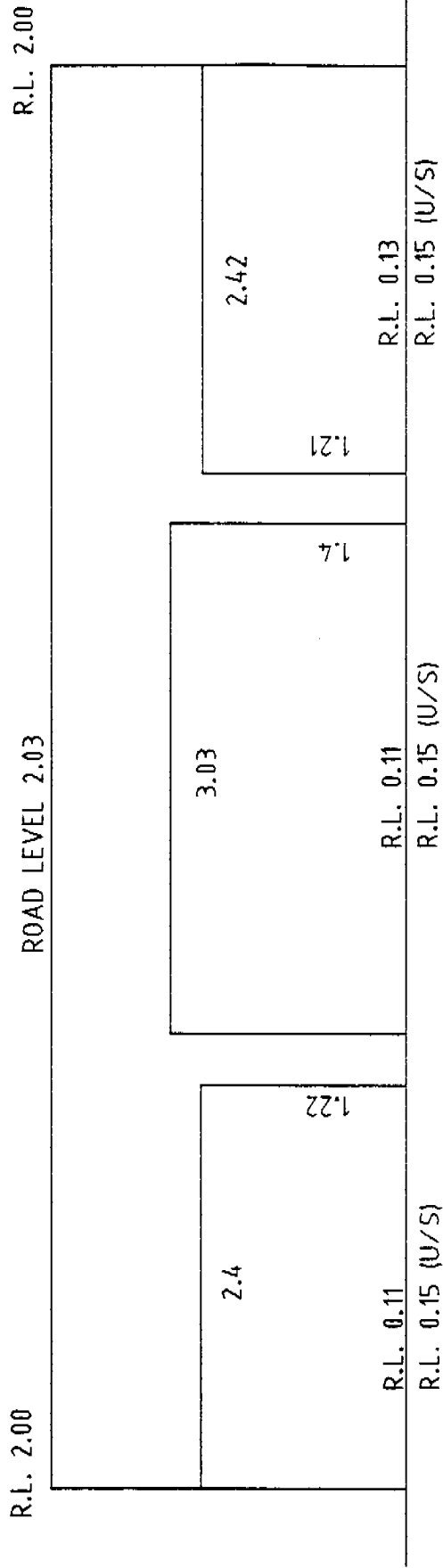
RAILWAY UNDERBRIDGE



D/S = DOWN STREAM
 U/S = UP STREAM

LOOKING DOWNSTREAM

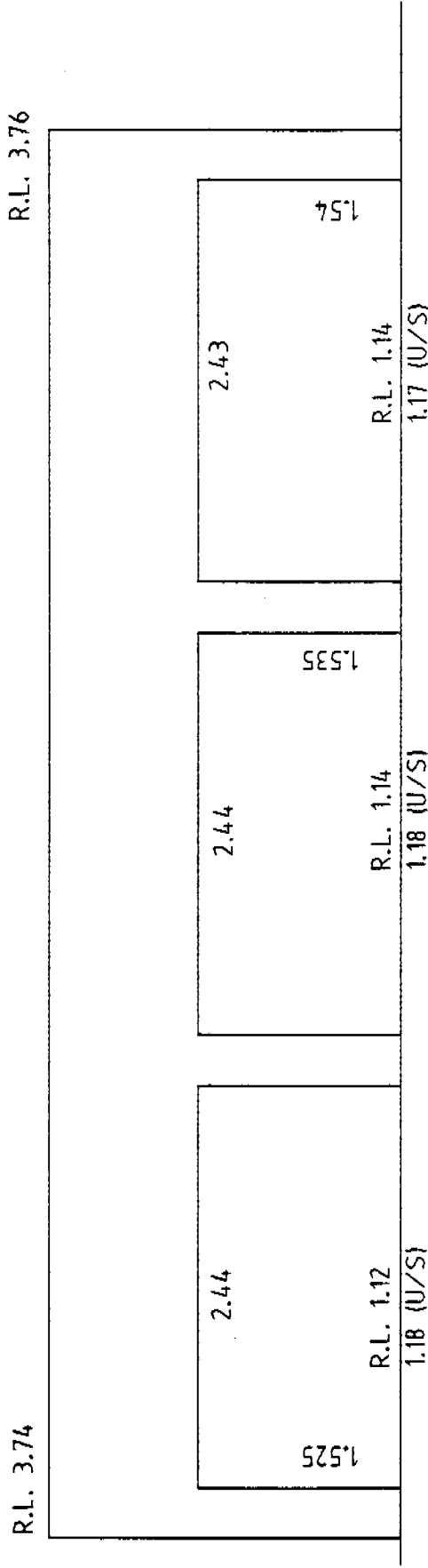
CULVERT UNDER RAILWAY STREET



LOOKING UPSTREAM

CULVERT UNDER NAMBUCCA DRIVE

3.55 NTH TOP OF KERB (TK)
3.55 CENTER LINE ROAD
3.55 STH TOP OF KERB (TK)

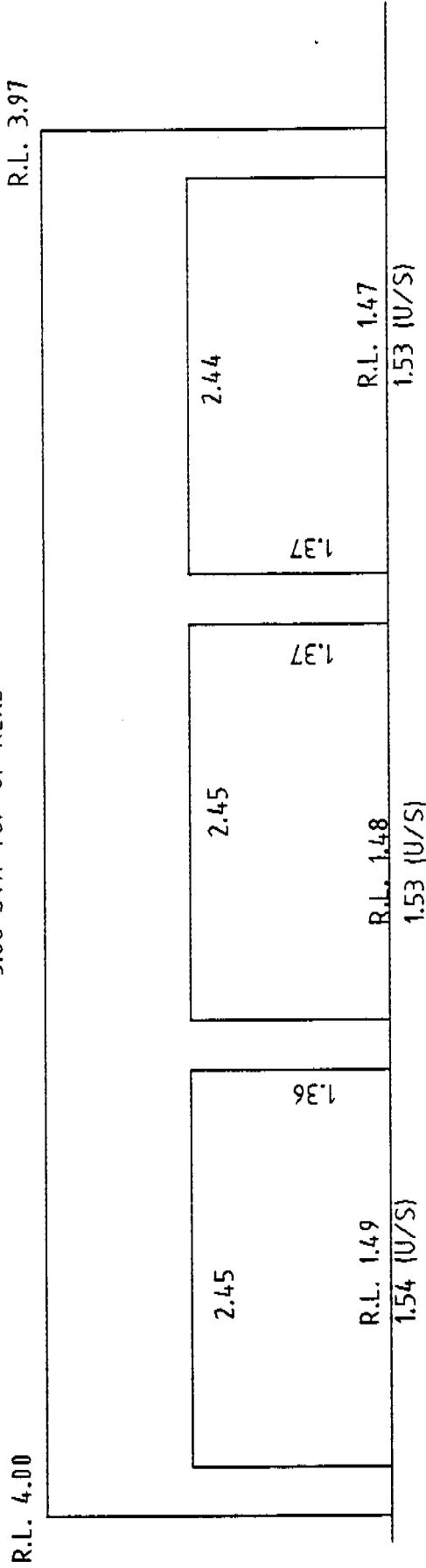


(CULVERT ABT. 17.1m LONG)

LOOKING UPSTREAM

CULVERT UNDER DULKARA ROAD

3.78 NTH TOP OF KERB
3.80 CENTER LINE ROAD
3.80 STH TOP OF KERB

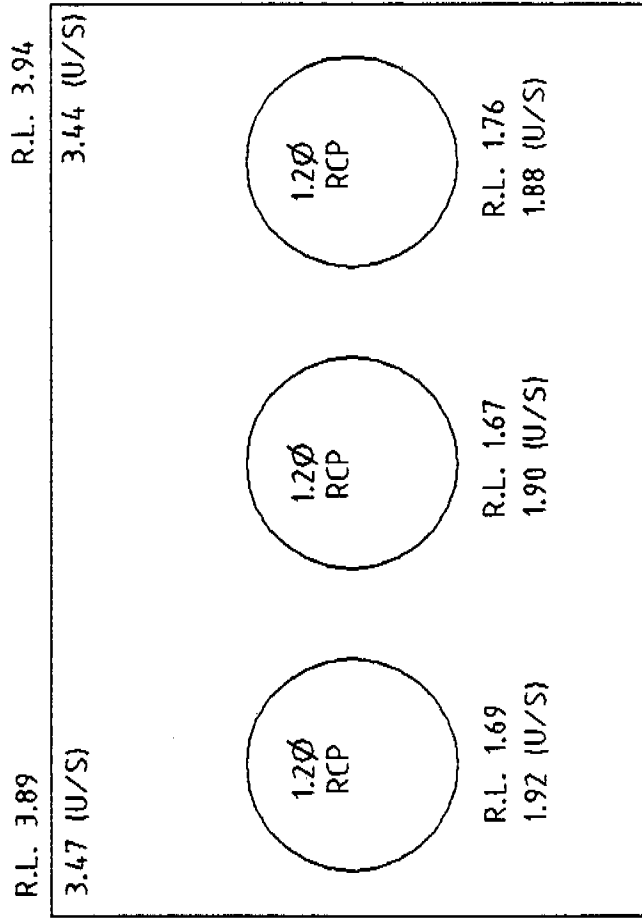


(CULVERT ABT. 17.1m LONG)

LOOKING UPSTREAM

CULVERT UNDER VERON ROAD

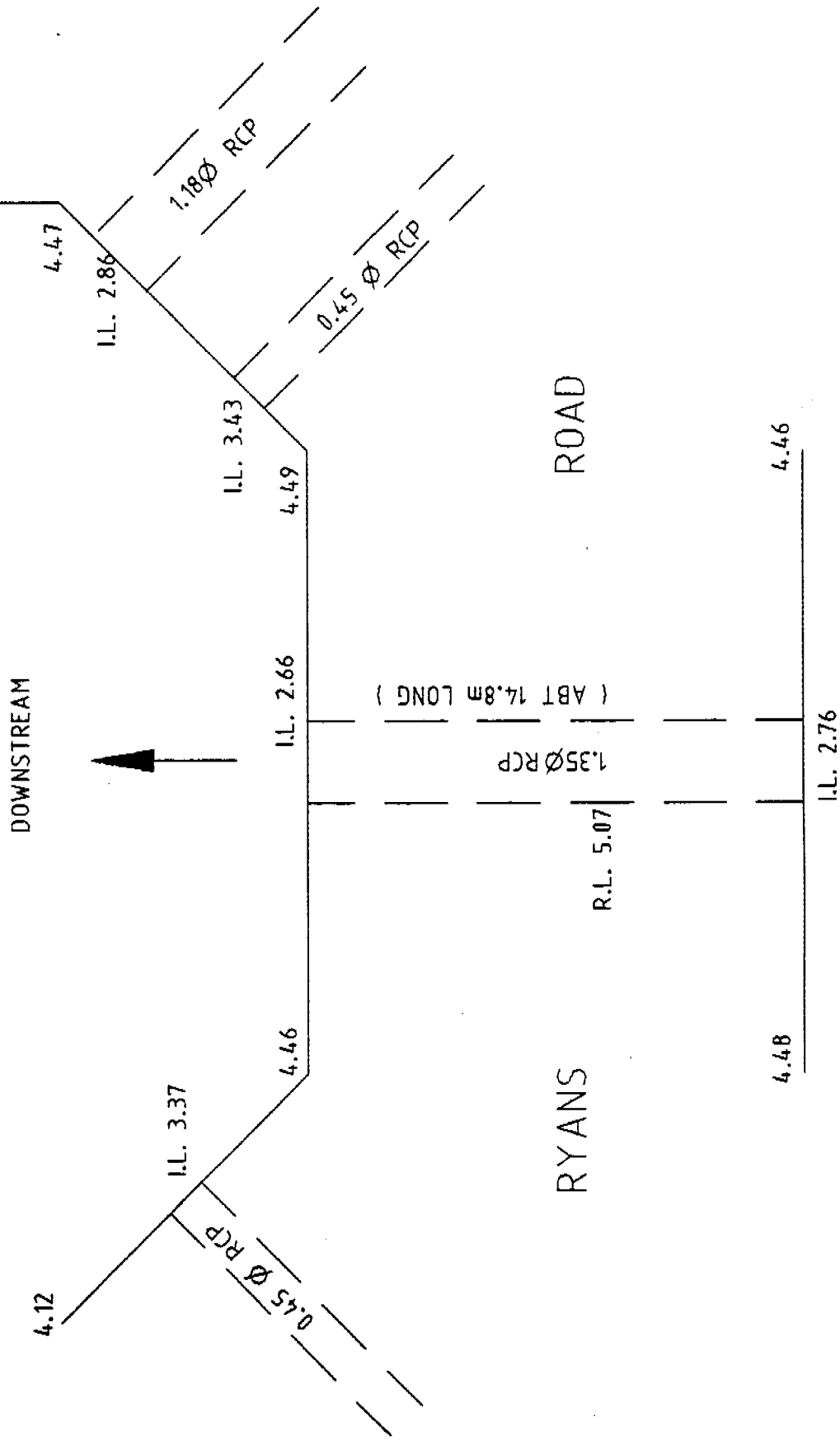
3.84 NTH TOP OF KERB
3.89 CENTER LINE ROAD
3.89 STH TOP OF KERB



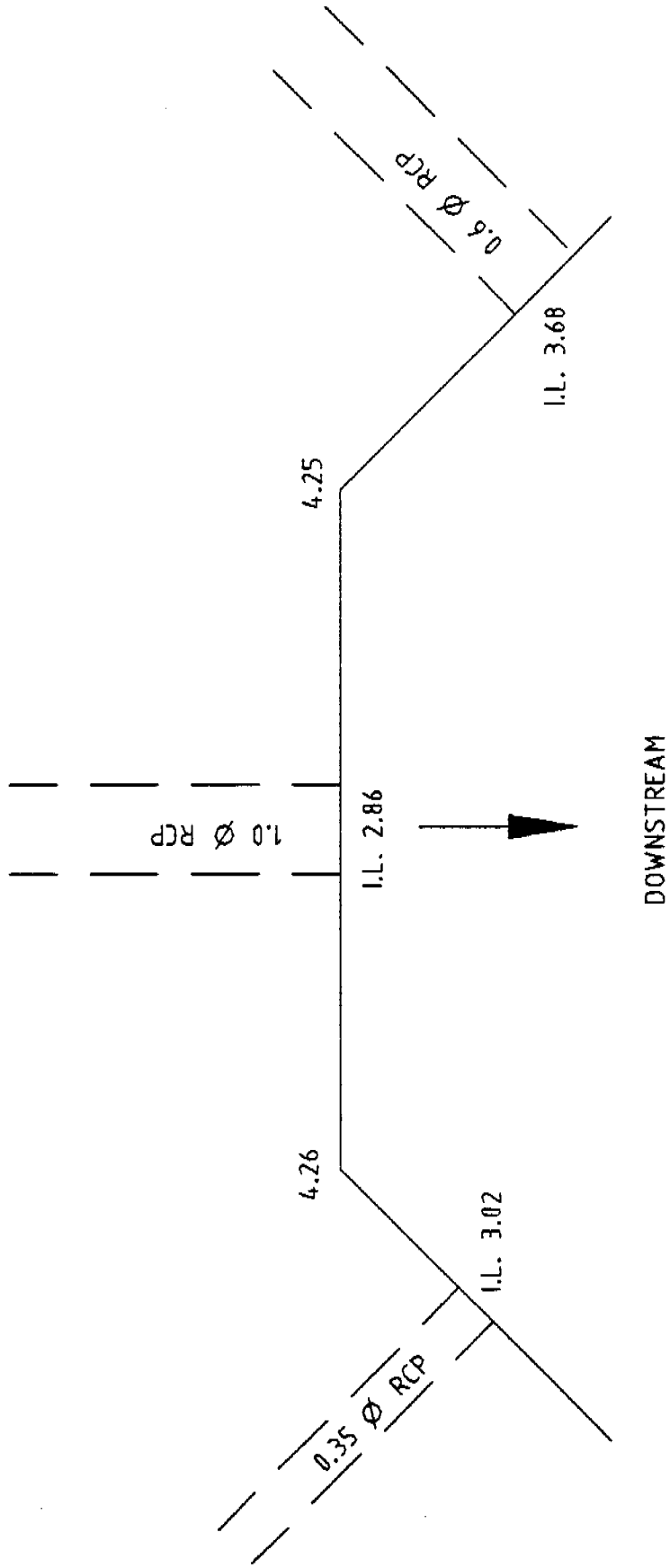
{ CULVERT ABT 22.0m LONG }

LOOKING UPSTREAM

CULVERT UNDER RYANS ROAD { PLAN }

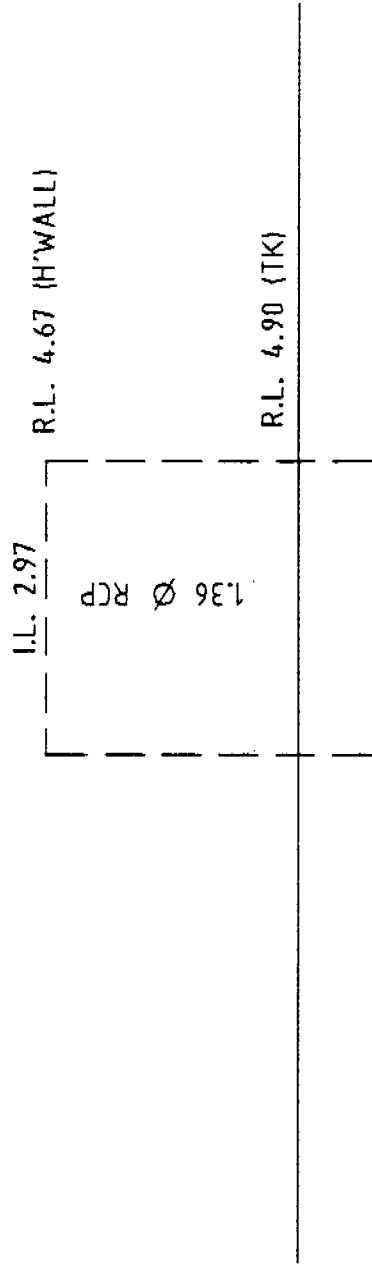
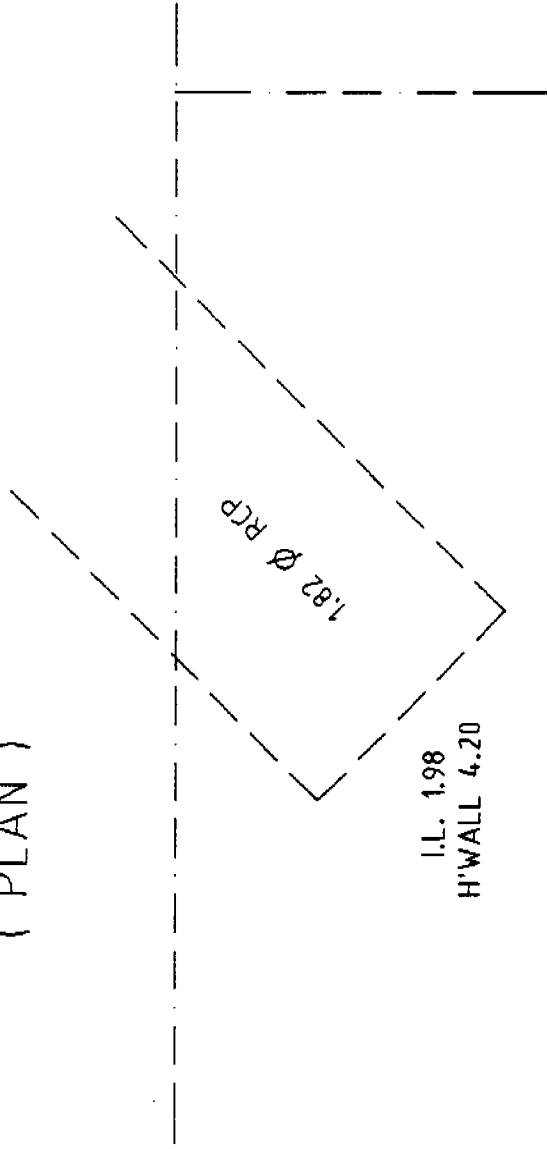


END OF EAST CHANNEL
(PLAN)



WESTERN CHANNEL

(PLAN)



VERON

ROAD

APPENDIX F



GOSFORD CITY COUNCIL

49 MANN STREET, GOSFORD, N.S.W. 2250

P.O. BOX 21, GOSFORD. DX 7211 GOSFORD

Telephone: (043) 25 8857

Please Quote: Mr C Slater:sw 109547 & 920.30.01
20 December 1991

Webb, McKeown & Associates Pty Ltd
117 York Street
SYDNEY NSW 2000

Attention: Mr I.J. Tye

Woy Woy, Umina, Ettalong, Peninsula Drainage Strategy Study - Determination of Council's Future Expenditure

Dear Sir

Following the project meeting called to discuss Council's likely works program and standards of work for the future in the study area if no further development occurred we wish to advise as follows:-

- o The criteria for developing a program of works in the study area should be primarily to remove flood waters from buildings. This strategy is applied across the city area to determine the highest priorities for drainage and flood mitigation works. Works proposals in this area would compete for available funding with other similarly affected areas of the city. Works to the same standards as new proposals are not an achievable short term goal within available funding. However wherever possible, the element of the works in the proposed program should be able to be readily incorporated in a suitable long term scheme. Applicable standards are set out below.
- o Residential streets with overflow to reserves or pathways, or bypass along the street - 10 year design flood average recurrence interval (DFARI).
- o Set minimum floor levels for low blocks of land, low points and low areas where surface flow may enter houses.
- o Major System Problems - rectification by staged works in problem areas using 50 year DFARI and minimum floor levels to achieve a standard where new buildings will have ½ metre free board over a 100 year DFARI. Proposals should be curtailed where benefits are low or marginal.
- o Current undrained areas without extensive problems of buildings flooding may be managed with minor drainage works and minimum floor levels.

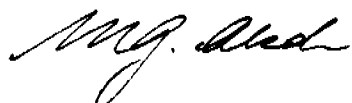
"The Scenic Central Coast just One Hour north of Sydney"

Webb, McKeown & Associates Pty Ltd

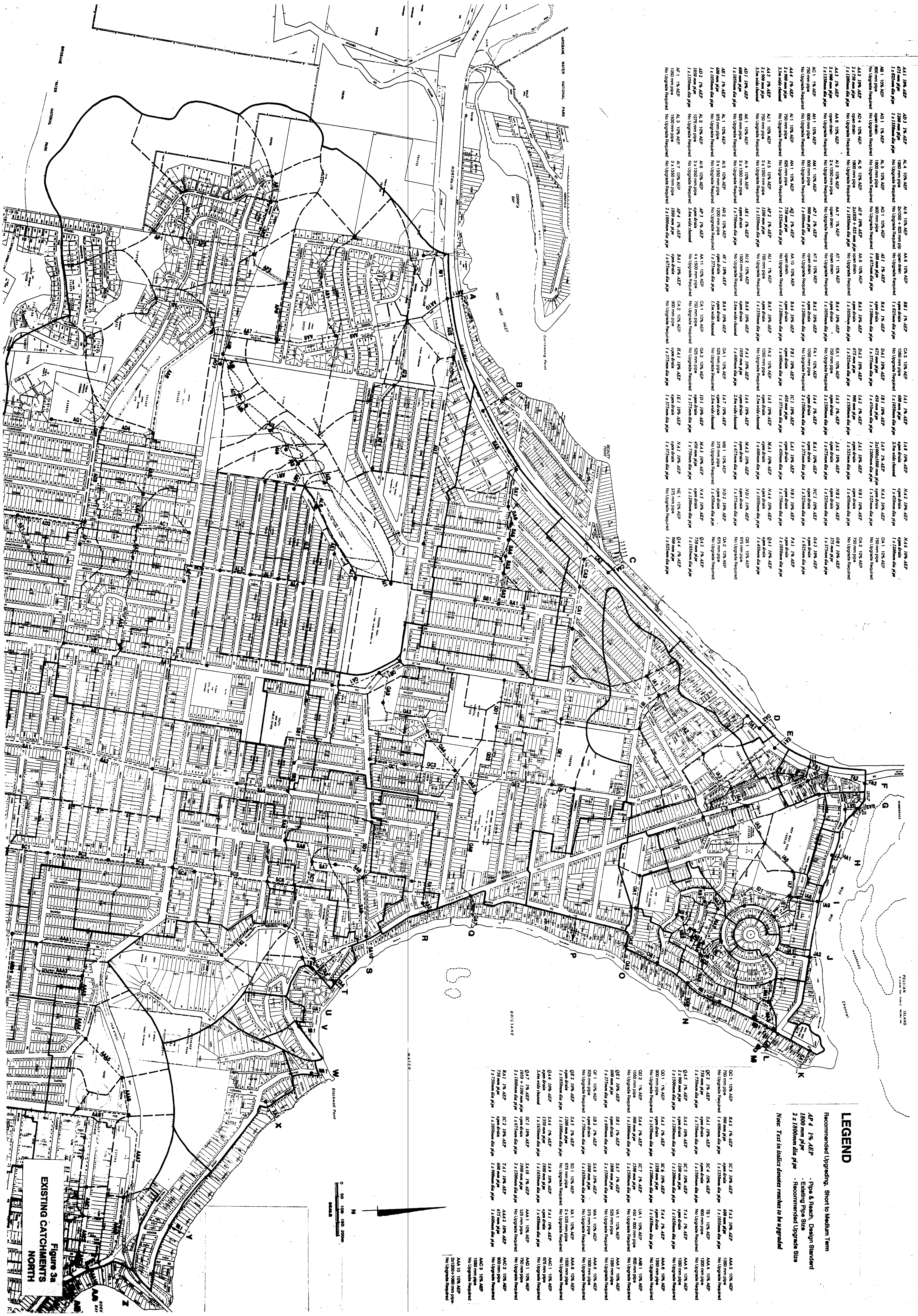
20 December 1991

Please apply these criteria in determining the estimated cost of the works required for the case with no further development.

Yours faithfully



M G Alsop
Program Manager - Environment



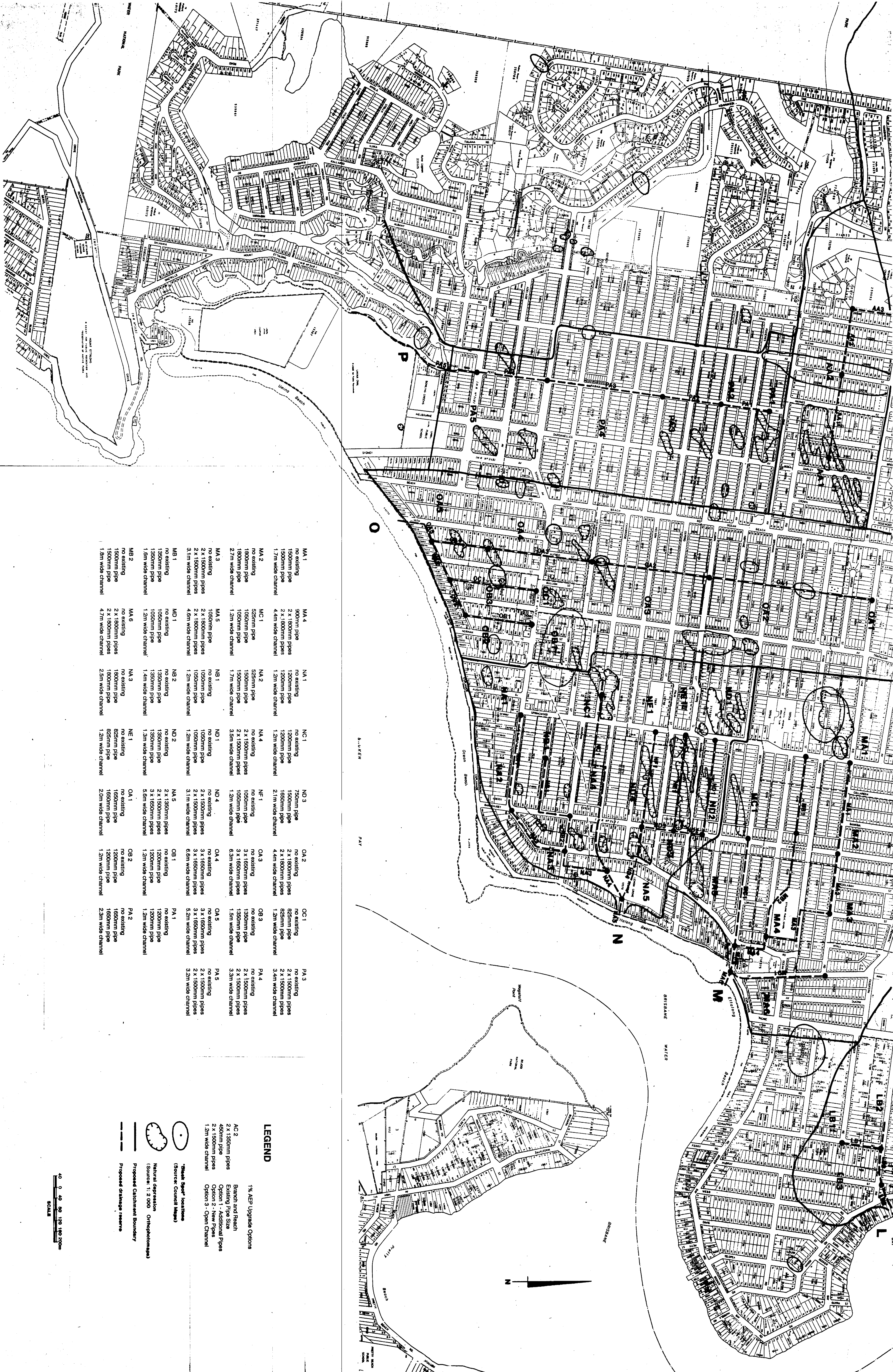
LEGEND

- Recommended Upgrading, Short to Medium Term
- AP 4 1% AEP
- 1800 mm dia pipe
- Pipe & Reach, Design Standard
- Existing Pipe Size
- 2 x 1800mm dia pipe
- Recommended Upgrade Size

Note: Text in *italics* denotes reaches to be upgraded

AA1 1% AEP	AA2 1% AEP	AA3 1% AEP	AA4 1% AEP	AA5 1% AEP	AA6 1% AEP	AA7 1% AEP	AA8 1% AEP	AA9 1% AEP	AA10 1% AEP	AA11 1% AEP	AA12 1% AEP	AA13 1% AEP	AA14 1% AEP	AA15 1% AEP	AA16 1% AEP	AA17 1% AEP	AA18 1% AEP	AA19 1% AEP	AA20 1% AEP	AA21 1% AEP	AA22 1% AEP	AA23 1% AEP	AA24 1% AEP	AA25 1% AEP	AA26 1% AEP	AA27 1% AEP	AA28 1% AEP	AA29 1% AEP	AA30 1% AEP	AA31 1% AEP	AA32 1% AEP	AA33 1% AEP	AA34 1% AEP	AA35 1% AEP	AA36 1% AEP	AA37 1% AEP	AA38 1% AEP	AA39 1% AEP	AA40 1% AEP	AA41 1% AEP	AA42 1% AEP	AA43 1% AEP	AA44 1% AEP	AA45 1% AEP	AA46 1% AEP	AA47 1% AEP	AA48 1% AEP	AA49 1% AEP	AA50 1% AEP	AA51 1% AEP	AA52 1% AEP	AA53 1% AEP	AA54 1% AEP	AA55 1% AEP	AA56 1% AEP	AA57 1% AEP	AA58 1% AEP	AA59 1% AEP	AA60 1% AEP	AA61 1% AEP	AA62 1% AEP	AA63 1% AEP	AA64 1% AEP	AA65 1% AEP	AA66 1% AEP	AA67 1% AEP	AA68 1% AEP	AA69 1% AEP	AA70 1% AEP	AA71 1% AEP	AA72 1% AEP	AA73 1% AEP	AA74 1% AEP	AA75 1% AEP	AA76 1% AEP	AA77 1% AEP	AA78 1% AEP	AA79 1% AEP	AA80 1% AEP	AA81 1% AEP	AA82 1% AEP	AA83 1% AEP	AA84 1% AEP	AA85 1% AEP	AA86 1% AEP	AA87 1% AEP	AA88 1% AEP	AA89 1% AEP	AA90 1% AEP	AA91 1% AEP	AA92 1% AEP	AA93 1% AEP	AA94 1% AEP	AA95 1% AEP	AA96 1% AEP	AA97 1% AEP	AA98 1% AEP	AA99 1% AEP	AA100 1% AEP
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Figure 3a
EXISTING CATCHMENTS
NORTH



MA 1	no existing 1500mm pipe 1500mm pipe 1.7m wide channel	MA 4	900mm pipe 2 x 1800mm pipes 2 x 1800mm pipes 4.4m wide channel	NA 1	no existing 1200mm pipe 1200mm pipe 1.2m wide channel	NC 1	no existing 1200mm pipe 1200mm pipe 1.2m wide channel	ND 3	750mm pipe 1500mm pipe 1500mm pipe 2.1m wide channel	OA 2	no existing 2 x 1800mm pipes 2 x 1800mm pipes 4.4m wide channel	OC 1	no existing 825mm pipe 825mm pipe 1.2m wide channel	PA 3	no existing 2 x 1500mm pipes 2 x 1500mm pipes 3.4m wide channel
MA 2	no existing 1800mm pipe 1800mm pipe 2.7m wide channel	MA 5	550mm pipe 1050mm pipe 1050mm pipe 1.2m wide channel	NA 2	550mm pipe 1500mm pipe 1500mm pipe 1.7m wide channel	NC 2	no existing 2 x 1500mm pipes 2 x 1500mm pipes 3.5m wide channel	ND 4	no existing 1050mm pipe 1050mm pipe 1.2m wide channel	OA 3	no existing 3 x 1650mm pipes 3 x 1650mm pipes 6.5m wide channel	OC 2	no existing 1200mm pipe 1200mm pipe 1.2m wide channel	PA 4	no existing 2 x 1500mm pipes 2 x 1500mm pipes 3.2m wide channel
MA 3	no existing 2 x 1500mm pipes 2 x 1500mm pipes 3.1m wide channel	MA 6	no existing 2 x 1800mm pipes 2 x 1800mm pipes 4.7m wide channel	NA 3	no existing 1350mm pipe 1350mm pipe 1.4m wide channel	NC 3	no existing 1050mm pipe 1050mm pipe 1.2m wide channel	ND 5	2 x 1350mm pipes 2 x 1500mm pipes 3 x 1650mm pipes 3.1m wide channel	OA 4	no existing 3 x 1650mm pipes 3 x 1650mm pipes 6.5m wide channel	OC 3	no existing 1200mm pipe 1200mm pipe 1.2m wide channel	PA 5	no existing 2 x 1500mm pipes 2 x 1500mm pipes 3.2m wide channel
MA 4	no existing 2 x 1500mm pipes 2 x 1500mm pipes 1.5m wide channel	MA 7	no existing 2 x 1800mm pipes 2 x 1800mm pipes 4.7m wide channel	NA 4	no existing 1350mm pipe 1350mm pipe 1.3m wide channel	NC 4	no existing 1050mm pipe 1050mm pipe 1.2m wide channel	ND 6	2 x 1350mm pipes 2 x 1500mm pipes 3 x 1650mm pipes 3.1m wide channel	OA 5	no existing 3 x 1650mm pipes 3 x 1650mm pipes 6.5m wide channel	OC 4	no existing 1200mm pipe 1200mm pipe 1.2m wide channel	PA 6	no existing 2 x 1500mm pipes 2 x 1500mm pipes 3.2m wide channel
MA 5	no existing 2 x 1500mm pipes 2 x 1500mm pipes 1.5m wide channel	MA 8	no existing 2 x 1800mm pipes 2 x 1800mm pipes 4.7m wide channel	NA 5	no existing 1350mm pipe 1350mm pipe 1.3m wide channel	NC 5	no existing 1050mm pipe 1050mm pipe 1.2m wide channel	ND 7	2 x 1350mm pipes 2 x 1500mm pipes 3 x 1650mm pipes 3.1m wide channel	OA 6	no existing 3 x 1650mm pipes 3 x 1650mm pipes 6.5m wide channel	OC 5	no existing 1200mm pipe 1200mm pipe 1.2m wide channel	PA 7	no existing 2 x 1500mm pipes 2 x 1500mm pipes 3.2m wide channel
MA 6	no existing 2 x 1500mm pipes 2 x 1500mm pipes 1.5m wide channel	MA 9	no existing 2 x 1800mm pipes 2 x 1800mm pipes 4.7m wide channel	NA 6	no existing 1350mm pipe 1350mm pipe 1.3m wide channel	NC 6	no existing 1050mm pipe 1050mm pipe 1.2m wide channel	ND 8	2 x 1350mm pipes 2 x 1500mm pipes 3 x 1650mm pipes 3.1m wide channel	OA 7	no existing 3 x 1650mm pipes 3 x 1650mm pipes 6.5m wide channel	OC 6	no existing 1200mm pipe 1200mm pipe 1.2m wide channel	PA 8	no existing 2 x 1500mm pipes 2 x 1500mm pipes 3.2m wide channel
MA 7	no existing 2 x 1500mm pipes 2 x 1500mm pipes 1.5m wide channel	MA 10	no existing 2 x 1800mm pipes 2 x 1800mm pipes 4.7m wide channel	NA 7	no existing 1350mm pipe 1350mm pipe 1.3m wide channel	NC 7	no existing 1050mm pipe 1050mm pipe 1.2m wide channel	ND 9	2 x 1350mm pipes 2 x 1500mm pipes 3 x 1650mm pipes 3.1m wide channel	OA 8	no existing 3 x 1650mm pipes 3 x 1650mm pipes 6.5m wide channel	OC 7	no existing 1200mm pipe 1200mm pipe 1.2m wide channel	PA 9	no existing 2 x 1500mm pipes 2 x 1500mm pipes 3.2m wide channel
MA 8	no existing 2 x 1500mm pipes 2 x 1500mm pipes 1.5m wide channel	MA 11	no existing 2 x 1800mm pipes 2 x 1800mm pipes 4.7m wide channel	NA 8	no existing 1350mm pipe 1350mm pipe 1.3m wide channel	NC 8	no existing 1050mm pipe 1050mm pipe 1.2m wide channel	ND 10	2 x 1350mm pipes 2 x 1500mm pipes 3 x 1650mm pipes 3.1m wide channel	OA 9	no existing 3 x 1650mm pipes 3 x 1650mm pipes 6.5m wide channel	OC 8	no existing 1200mm pipe 1200mm pipe 1.2m wide channel	PA 10	no existing 2 x 1500mm pipes 2 x 1500mm pipes 3.2m wide channel
MA 9	no existing 2 x 1500mm pipes 2 x 1500mm pipes 1.5m wide channel	MA 12	no existing 2 x 1800mm pipes 2 x 1800mm pipes 4.7m wide channel	NA 9	no existing 1350mm pipe 1350mm pipe 1.3m wide channel	NC 9	no existing 1050mm pipe 1050mm pipe 1.2m wide channel	ND 11	2 x 1350mm pipes 2 x 1500mm pipes 3 x 1650mm pipes 3.1m wide channel	OA 10	no existing 3 x 1650mm pipes 3 x 1650mm pipes 6.5m wide channel	OC 9	no existing 1200mm pipe 1200mm pipe 1.2m wide channel	PA 11	no existing 2 x 1500mm pipes 2 x 1500mm pipes 3.2m wide channel
MA 10	no existing 2 x 1500mm pipes 2 x 1500mm pipes 1.5m wide channel	MA 13	no existing 2 x 1800mm pipes 2 x 1800mm pipes 4.7m wide channel	NA 10	no existing 1350mm pipe 1350mm pipe 1.3m wide channel	NC 10	no existing 1050mm pipe 1050mm pipe 1.2m wide channel	ND 12	2 x 1350mm pipes 2 x 1500mm pipes 3 x 1650mm pipes 3.1m wide channel	OA 11	no existing 3 x 1650mm pipes 3 x 1650mm pipes 6.5m wide channel	OC 10	no existing 1200mm pipe 1200mm pipe 1.2m wide channel	PA 12	no existing 2 x 1500mm pipes 2 x 1500mm pipes 3.2m wide channel

- LEGEND**
- 1% AEP Upgrade Options
 - Branch and Reach
 - Existing pipe Size
 - Proposed pipe Size
 - Option 2 - New Pipes
 - Option 3 - Open Channel
 - AC 2
 - 450mm pipe
 - 2 x 1500mm pipes
 - 1.2m wide channel
 - Shale Spur locations (Source: Council Maps)
 - Natural depression (Orthophotomaps) (Source: 1:2,000 Orthophotomaps)
 - Proposed Catchment Boundary
 - Proposed drainage reserve

**Figure 4b
PROPOSED CATCHMENTS
SOUTH**

0 10 20 30 40 50 60 70 80 90 100 110 120 130 140 150 160 170 180 190 200
SCALE

End of Report