

GOSFORD CITY COUNCIL

MIDDLE CREEK, PEARL BEACH FLOOD STUDY

Final Report DECEMBER 2003

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1 SUMMARY

Gosford City Council commissioned Patterson Britton & Partners to undertake an investigation into the flooding of the Middle Creek catchment at Pearl Beach.

The '*Floodplain Development Manual*' (*Ref.*) outlines the steps involved in the floodplain management process (*refer below*). No detailed flood study has been undertaken for the Middle Creek catchment. Therefore, the first step that needs to be carried out in this process is the Flood Study, which will include detailed flood modelling for the purposes of carrying out the Floodplain Management Study and Plan.



The floodplain management plan for the study area will then address the existing, future, and residual flood and environmental problems, in accordance with the NSW Government's Flood Policy as detailed in NSW Government's *Floodplain Development Manual*.

Specifically, the objectives of this flood study were to:

- define flood behaviour along Middle Creek for the 20%, 5%, 1% AEP and an extreme event using hydrologic and hydraulic models; and
- present the flood behaviour for Middle Creek in a clear and concise manner, including flood flows, velocities and levels for the above events.

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The approach adopted for this flood study was:

- collect and collate background information;
- assemble and calibrate hydrological model of catchment;
- review and modify existing hydraulic model, refine and calibrate as necessary; and
- define design flood flows, levels, velocities and extents for three scenarios, these being:
- 1. Existing conditions;
- 2. Flood Mitigation Works Proposed culvert extension upstream of Diamond Road; and
- 3. Ultimate Flood Mitigation Works Proposed culvert extension upstream of Diamond Road, and channel widening works between chainage 270.0 and 405.0.

The findings of the flood study were that the proposed culverts works currently under construction upstream of Diamond Road would effectively alleviate inundation of existing properties along Middle Creek between Chainage 0.0 and 270.0 up to and including a 1% AEP flood event.

Also it was found that the proposed channel works, combined with the culvert works will effectively ensure flood free conditions for all existing properties along Middle Creek at Pearl Beach.

As no detailed coastal analysis has been undertaken, Council has advised that a flood level of RL 4.0 m AHD be adopted for locations between the beach, up to a point where the flood level is 4.0 m AHD from catchment flows. Council's advice regarding coastal effects has been based on the recent analysis of the nearby Green Point Creek outlet. These investigations indicated that a flood level of RL 4.05 m was appropriate for the hindrance to flood flows caused by the beach berm and dune at this and other similar creek outlets to Pearl Beach.

During an extreme event (*twice the 1% AEP flow*), it is estimated that for the ultimate flood mitigation works scenario, up to 9 sheds and garages and 1 residence would be inundated.

Estimated flood flows, levels, velocities and extents for the various events are presented in this report.

2.1 STUDY AREA

The study area for this investigation is the Middle Creek channel between the western end of Emerald Street, and the outlet into Broken Bay near Pearl Parade, Pearl Beach. Middle Creek flows between the properties fronting Pearl Beach Drive and Emerald Avenue. The study area is shown in **Figure 1**.

2.2 BACKGROUND

Recent investigations of Middle Creek have been undertaken for Council by various consultants. These include a flood investigation dated April 1989 by Giammarco Civil & Structural Engineering Pty Ltd, and hydrological and hydraulic modelling prepared by Boyden & Partners as part of their recent design works for the creek. These more recent investigations provided the base hydraulic model for the study, although it has been modified to suit existing conditions.

2.3 DATA

The following background data was used to undertake this study:

Topographic Maps

- Broken Bay 2782-4-A (1:2000) Orthophotomap *Central Mapping Authority*, Dept of Lands, 1974;
- Broken Bay 2782-1-C (1:2000) Orthophotomap *Central Mapping Authority*, Dept of Lands, 1974

<u>Design Drawings</u>

 Drainage Improvement Works, Middle Creek Catchment, Pearl Beach – File No. 920.14.18, Dwg No.s 14/38/00 to 14/38/15 inclusive – Boyden & Partners Pty Ltd for Gosford City Council, May 1998

Hydraulic Models

• HEC-RAS model: 98047.prj, Boyden & Partners Pty Ltd for Gosford City Council, May 1998

Local Zoning Plan

• Zoning Plan for Pearl Beach, Gosford City Council, 1998.

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2.4 ELEMENTS OF STUDY

The elements of this study include:

- *review of existing data*, including the previous investigations listed above in terms of the suitability of the survey data for hydraulic modelling, and a comprehensive review of the hydrological and hydraulic modelling;
- *Hydrologic Modelling*, establishment of RAFTS rainfall/runoff model of the Middle Creek catchment including the study area to estimate flows under existing catchment conditions for the 1%, 5% and 20% AEP events as well as an extreme flood event;
- *Hydraulic Modelling*, utilisation of the HEC-RAS hydraulic model from the previous investigations undertaken by Boyden & Partners, with modifications to identify flood behaviour for existing and post flood mitigation works scenarios (*3 Scenarios in total*) and flood levels for the design events listed above; and
- Produce a bound report describing the methodology and results of the flood study, in accordance with the NSW Government Floodplain Development Manual and the headings outlined in the brief.

3.1 RAFTS HYDROLOGIC MODEL

RAFTS is a non-linear rainfall/runoff program developed by WP software, in Canberra. RAFTS can be used to estimate peak flows for catchments, using actual storm events, or design rainfall data derived from AR&R, 1987 (*Ref.*).

To undertake the hydrologic modelling, a RAFTS rainfall/runoff model was established for the Middle Creek catchment. This model was used to estimate flows under existing catchment condition for the 1%, 5% and 20% AEP events as well as an extreme flood event. All hydrologic analyses were undertaken in accordance with "Australian Rainfall and Runoff – A Guide to Flood Estimation", (Volumes I and II, IEAust, 1987).

RAFTS was chosen for this investigation because it has the following attributes:

- it can account for spatial and temporal variation in storm rainfall across a catchment;
- it can accommodate variations in catchment characteristics;
- it can be used to estimate discharge hydrographs at any location within the catchment; and
- it has been widely used across NSW.

3.2 MODEL SET-UP

The catchment was divided into a number of subcatchments based on the topography, land use, road and the existing drainage system layout. Estimates of existing peak design flows were derived for input into the hydraulic model.

Each subcatchment has parameters defined including area, weighted average catchment slope, the percentage of impervious area, and lag time to the next downstream sub-catchment. A summary of adopted subcatchment parameters is enclosed in **Appendix A**.

A total of 13 subcatchments were identified for the RAFTS model, with a total catchment area of approximately 60 ha. The catchment breakdown is shown on **Figure 1**, including the RAFTS model network layout.

3.3 CALIBRATION

As no stream gauge exists in the catchment, there was no possibility of a true storm calibration, however comparison was made with the rational formula calculations for "small to medium sized rural catchments", as outlined in ARR 1987 (*Ref.*). The results of this comparison are shown below in **Table 3.1**.

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Average Recurrence Interval (AEP)	Peak discharge estimated using Rational Method (m ³ /s)	Peak discharge estimated using RAFTS model (m ³ /s)
20%	0.0	12.2
5%	14.7	16.1
1%	24.0	20.2
extreme (2 x 1% AEP)	N/A *	41.2

Table 3.1	· Com	narison	of Poak	Flow	Results	hetween	RAFTS	and	Rational	Meth	м
I able 3.1	. Com	par 15011	UI I Cak	T IUW	results	Detween	NALIS	anu	National	IVICUIU	JU

Note: * : The rational method is only intended to be used up to and including the 100 year ARI event.

As can be seen from the results tabulated above, the RAFTS results are slightly higher than those derived using the rational formula for events more frequent than the 1% AEP event, however, the rational method overestimates the peak 1% AEP peak flow, compared to the RAFTS model. This effect could be caused by storage effects in the RAFTS model, which tend to reduce the differences in peak flow estimates for different recurrence interval storm events.

The rational method calculations gave results that were comparable to the RAFTS model results. The RAFTS model results were adopted because the methodology is more comprehensive than the rational method, taking into account subcatchment slope, roughness, impervious percentage and lag times.

3.4 DESIGN STORM SIMULATION RESULTS

The RAFTS model, once "calibrated" was used to simulate runoff generated using design storm rainfall intensities and temporal patterns for the study area. Storm rainfall data was generated by applying the principals of rainfall intensity estimation and design temporal distributions outlined in AR&R (*Ref.*).

A range of storm durations were considered and modelled to establish the critical storm duration for the catchment. The critical storm duration corresponds to the maximum peak discharge generated by the hydrologic model for the most downstream node within the catchment.

A critical duration of 2 hours was determined for the catchment.

Using a critical duration of 2 hours and the corresponding rainfall intensities and design temporal patterns, peak discharges and discharge hydrographs were generated for the range of flood frequencies.

Peak catchment discharges at the outlet to the catchment as determined using the hydrologic model are listed in **Tables 3.1** above. Discharge hydrographs for the downstream end of the catchment are shown in **Appendix B**.

4 HYDRAULIC MODELLING

4.1 REVIEW OF EXISTING HEC-RAS MODEL

HEC-RAS is a steady-state backwater program developed by US Army Corp of Engineers Hydrological Engineering Centre (*HEC*). HEC-RAS can be used to estimate peak flood levels in open channels, taking into account the effects of bridges, culverts and other hydraulic controls. (*Ref. - HEC-RAS User Manual*)

The existing HEC-RAS model set up by Boyden and Partners Pty Ltd for their drainage improvement works was reviewed for technical correctness, and application for the purposes of a flood study. The modelling was found to be comprehensive, including the inclusion of culvert works associated with the proposed improvements just upstream of the existing Diamond Road culverts.

A plan of the model cross sections is shown in Figure 2.

Some minor modifications were made to the extent of a number of the model cross sections, to allow the modelling of the extreme $(2x \ 1\% \ AEP)$ event. This involved examining the design plan contours, or simply extrapolating cross section grades up to the extreme flood level, where additional contour information was not available on the plans.

4.2 MODEL CALIBRATION

The hydraulic model was tested by comparing predicted flood levels with the flood levels determined in the previous investigation. The changes to the cross sections for the purposes of modelling the extreme event, outlined above, had a very minor effect on the design 1% AEP flood levels derived by Boyden & Partners.

4.3 MODEL SCENARIOS

The hydraulic model was run for a number of scenarios, based on the proposed flood mitigation works proposed by Gosford City Council. Three scenarios were identified, as follows:

- 1. Existing conditions;
- 2. Flood Mitigation Works Proposed culvert extension upstream of Diamond Road; and
- 3. Ultimate Flood Mitigation Works Proposed culvert extension upstream of Diamond Road, and channel widening works between chainage 270.0 and 405.0.

Case 2 above was examined at Council's request because the culvert works upstream of Diamond Road were in the process of being constructed, when the modelling for this study was finalised.

4.4 COASTAL EFFECTS

The beach berm and dune at creek outlets hinder flood flows until the crest is overtopped and a channel is eroded across the beach by flood flows. No detailed coastal analysis has been undertaken of the dune and berm levels at the outlet of Middle Creek, but recent experience at Green Point Creek, which discharges across the southern end of Pearl Beach, has been taken into account in determining these coastal effects.

Council has drawn on a recent flooding investigation for Green Point Creek, where a flood level of RL 4.05 m was adopted, due to coastal effects. Also, Council recently recorded a flood level of approximately RL 4.0 m AHD near the culvert under Coral Crescent for Pearl Beach Lagoon.

In light of this information Council has requested that a flood level of RL 4.0 m AHD be adopted in the creek for areas from the outlet up to a point where the predicted flood level in the creek is RL 4.0 m AHD from the catchment flows alone.

This coastal backwater effect is incorporated into the reported design storm results in **Section 4.5**, and the flood profiles and extents shown in the **Figures**.

4.5 DESIGN STORM RESULTS

The estimated peak flood level results for the above three scenarios are presented in **Tables 4.1** to **4.3**. It should be noted that these results include an allowance for a downstream level water surface profile at RL 4.0 m AHD, as nominated by Council *(Refer Section 4.4)*.

Longitudinal section profiles of these flood levels are provided for Scenarios 2 and 3 in **Figures 4** and **6** respectively. Flood extents, contours and velocity vectors are shown for the 1% AEP event in **Figures 3** and **5**.

Peak flood levels and velocities for all scenarios and storm events are shown in the hydraulic model results listing in **Appendix C**.

The results of the modelling indicate that the proposed culvert works upstream of Diamond Road will reduce the estimated flood levels between chainage 220.0 and 370.0. The most significant reduction in estimated flood level occurs just upstream of the proposed culverts, where flood levels for the 1% AEP event are reduced by up to one metre.

The modelling also shows that a further reduction in estimated flood levels is achieved by the proposed channel improvement works. Estimated flood levels for the 1% AEP event are reduced between chainage 280.0 and 420.0, again by as much as one metre from the flood levels predicted for the existing conditions scenario.

4.6 POTENTIALLY FLOOD AFFECTED PROPERTIES

Table 4.4 lists various locations along Middle Creek where the estimated 1% AEP water surface level, plus 500 mm freeboard for Scenario 2 comes close to or exceeds surveyed building floor levels.

The results indicate that the proposed culvert works will effectively ensure flood free conditions for existing properties along Middle Creek between chainages 0.0 and 270.0 at Pearl Beach. However, immediately upstream of the proposed culvert works, the flood profile will rise quite steeply to around chainage 315.0, where the profile settles back into a typical normal flow.

This steeper profile is believed to be caused by a gradually varied profile transitioning from the slower, deeper flow of the existing channel, down through the more hydraulically efficient culvert extension at Diamond Road.

Table 4.5 compares the same floor levels to the estimated 1% AEP water surface levels (*plus 500 mm freeboard*) for Scenario 3.

These results indicate that the proposed channel and culvert works will effectively ensure flood free conditions for all existing properties along Middle Creek at Pearl Beach.

The flood profile upstream of the extended Diamond Road culverts continues upstream at a lower level, as a result of the channel improvement works proposed under this scenario. However, once the channel transitions back to the existing cross section at chainage 405.0, the profile rises quit steeply, back to the level in the original channel.

Adequate development controls by Council should ensure that future development along the creek is also flood free up to and including the 1% AEP event.

Creek Chainage (m)20% AEP5% AEP1% AEPExtreme Event $(2 x 1\% AEP)$ 04.004.004.004.00154.004.004.004.00304.004.004.004.00454.004.004.004.00604.004.004.004.00754.004.004.004.00904.004.004.004.001054.004.004.004.001204.004.004.004.00
Chainage (m)20% AEP5% AEP1% AEPExtreme Event $(2 x 1% AEP)$ 04.004.004.004.00154.004.004.004.00304.004.004.004.00454.004.004.004.00604.004.004.004.00754.004.004.004.00904.004.004.004.001054.004.004.004.001204.004.004.004.00
(m) $(2 \times 1\% AEP)$ 04.004.004.00154.004.004.00304.004.004.00454.004.004.00604.004.004.00604.004.004.00754.004.004.00904.004.004.001054.004.004.001204.004.004.00
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1.00 1.00 T.00 T.00
135 4.00 4.00 4.00 4.77
150 4.00 4.00 4.07 4.85
207 4.15 4.39 4.61 5.38
221 4.27 4.56 4.96 6.26
222 4.25 4.54 4.96 6.15
240 4.73 4.97 5.36 6.36
255 5.58 5.83 6.10 6.52
270 5.87 6.11 6.36 6.79
284 6.01 6.25 6.50 6.95
285 6.02 6.25 6.51 6.96
300 6.09 6.31 6.57 7.04
315 6.19 6.41 6.67 7.17
330 6.24 6.47 6.72 7.24
345 6.44 6.67 6.93 7.46
359 6.53 6.71 6.95 7.49
360 6.54 6.73 6.97 7.50
375 6.67 6.83 7.10 7.59
390 7.08 7.34 7.49 7.79
399 7.20 7.45 7.60 7.92
400 7.17 7.42 7.54 7.74
405 7.25 7.48 7.63 8.01
420 7.37 7.55 7.71 8.25
435 7.78 7.97 8.21 8.59
450 7.87 8.06 8.29 8.72
465 7.89 8.07 8.30 8.71
480 7.92 8.10 8.33 8.78
495 8.04 8.22 8.43 8.85 510 9.10 9.27 9.46 9.02
510 8.10 8.2/ 8.46 8.92 525 8.20 8.46 8.60 0.12
323 8.29 8.40 8.08 9.13 540 9.51 9.60 9.97 0.20
340 8.51 8.09 8.87 9.28 555 8.55 8.72 8.01 0.21
333 8.35 8.72 8.91 9.31 570 8.60 8.75 8.01 0.20
570 6.00 6.75 6.91 9.29 585 8.74 8.00 0.10 0.56
303 0.74 0.70 9.10 9.30 600 8.85 0.02 0.21 0.62
000 0.03 9.02 9.21 9.02 615 0.05 0.10 0.37 0.90

<u>Table 4.1</u>: Estimated peak water surface levels for 20%, 5%, 1% AEP and extreme events for Scenario 1 (*Existing conditions*)

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Middle		Estimated Peak W	ater Surface Levels	
Creek				
Chainage	20% AEP	5% AEP	1% AEP	Extreme Event
(m)		0,01111		$(2 \times 1\% AEP)$
0	4.00	4.00	4.00	4.00
15	4.00	4.00	4.00	4.00
30	4.00	4.00	4.00	4.00
45	4.00	4.00	4.00	4.00
60	4.00	4.00	4.00	4.00
75	4.00	4.00	4.00	4.00
90	4.00	4.00	4.00	4.00
105	4.00	4.00	4.00	4.00
120	4.00	4.00	4.00	4.04
135	4.00	4.00	4.00	4.77
150	4.00	4.00	4.07	4.85
207	4.15	4.39	4.61	5.38
221	4.27	4.56	4.96	6.28
222	4.25	4.54	4.96	6.26
270	4.39	4.71	5.36	6.44
284	5.27	5.45	5.68	6.30
285	5.46	5.66	5.90	6.49
300	5.80	5.99	6.23	6.82
315	6.01	6.21	6.47	7.05
330	6.11	6.31	6.56	7.14
345	6.37	6.60	6.86	7.41
359	6.49	6.68	6.89	7.45
360	6.51	6.71	6.91	7.46
375	6.65	6.82	7.09	7.56
390	7.07	7.34	7.49	7.79
399	7.20	7.45	7.60	7.92
400	7.17	7.41	7.54	7.75
405	7.25	7.48	7.63	8.01
420	7.37	7.55	7.71	8.25
435	7.78	7.97	8.21	8.59
450	7.87	8.06	8.29	8.72
465	7.89	8.07	8.30	8.71
480	7.92	8.10	8.33	8.78
495	8.04	8.22	8.43	8.85
510	8.10	8.27	8.46	8.92
525	8.29	8.46	8.68	9.13
540	0.31	0.09 9 7 2	0.0/	9.28
535 570	8.33 8.60	8.72 9.75	8.91 8.01	9.31
505	0.0U 0.71	0./3	0.71	9.29
505	0./4	0.90	9.10 0.21	9.30
615	0.05	9.02	0.37	9.02
015	9.05	1.17	1.51	9.00

<u>Table 4.2</u>: Estimated peak water surface levels for 20%, 5%, 1% AEP and extreme events for Scenario 2 (*Culvert works without channel works*)

Middle		Estimated Peak W	ater Surface Levels	
Creek				
Chainage	20% AEP	5% AEP	1% AEP	Extreme Event
(m)				(2 x 1% AEP)
0	4.00	4.00	4.00	4.00
15	4.00	4.00	4.00	4.00
30	4.00	4.00	4.00	4.00
45	4.00	4.00	4.00	4.00
60	4.00	4.00	4.00	4.00
75	4.00	4.00	4.00	4.00
90	4.00	4.00	4.00	4.00
105	4.00	4.00	4.00	4.01
120	3.31	3.49	3.67	4.04
135	3.52	3.74	3.96	4.77
150	3.62	3.85	4.08	4.85
207	4.15	4.39	4.61	5.38
221	4.27	4.56	4.98	6.18
222	4.25	4.54	4.96	6.15
270	4.39	4.71	5.36	6.31
284	4.49	4.81	5.43	6.36
285	4.49	4.66	5.27	6.18
300	4.95	5.16	5.49	6.34
315	5.12	5.33	5.65	6.48
330	5.25	5.47	5.78	6.59
345	5.38	5.62	5.94	6.77
359	5.50	5.75	6.08	6.96
360	5.43	5.60	5.87	6.82
375	5.89	6.10	6.36	7.12
390	6.05	6.27	6.55	7.32
399	5.27	5.39	5.55	1.42
400	5.86	6.00	6.1/	6.69
405	6.8/	7.07	/.58	8.32
420	/.40	/.60	/./4	8.42
455	7.81	/.99	8.21	8.04 9.76
450	7.90	8.08	8.29	8./0 9.76
403	7.92	8.10	8.31	8.70
460	7.90	0.14 9.25	8.30 8.45	0.04
493	8.07 8.15	0.23 8.22	8.4 <i>3</i> 8.5 <i>4</i>	0.02
525	8.1 <i>3</i> 8.21	0.52	8.34 8.72	9.02
540	8.51	0.40 8 73	8.72	9.19
555	0. <i>39</i> 8.61	8.75 8.75	8.90	9.32
570	8.64	8.75	8 Q/	9.35
585	8 78	8.95	0.74	9.61
600	8 90	9.11	932	9.71
615	9.07	9.24	9 44	9.83
015	2.07	<i>7.4</i> T	2.77	2.00

Table 4.3: Estimated peak water surface levels for 20%, 5%, 1% AEP and extreme events for Scenario 3 (Ultimate case: Culvert and channel works)

5	cenario 2	
Approximate Middle Creek Chainage (m)	Estimated Peak Water Surface Level for 1% AEP + 500 mm freeboard for Scenario 2 (m, AHD)	Surveyed Building Floor Level adjacent to creek (m, AHD)
230	5.66	No. 36 Diamond Rd - Carport FL* RL 5.63
265	5.90	No. 36 Diamond Rd - Shed FL RL 5.74
375 "	7.60	No. 14 Emerald Rd - Shed FL RL 6.87 and No. 14 Emerald Rd - Cottage FL RL 6.75
490	8.90	No. 28 Emerald Rd – Shed FL RL 9.02

Table 4.4:Comparison of Estimated 1% AEP Peak Flood with building floor levels for
Scenario 2

Note: * : FL refers to surveyed Floor Level

Table 4.5:Comparison of Estimated 1% AEP Peak Flood with building floor levels for
Scenario 3

Approximate Middle Creek Chainage (m)	Estimated Peak Water Surface Level for 1% AEP + 500 mm freeboard for Scenario 3 (m, AHD)	Surveyed Building Floor Level adjacent to creek (m, AHD)
230	5.50	No. 36 Diamond Rd - Carport FL* RL 5.63
265	5.80	No. 36 Diamond Rd - Shed FL RL 5.74
375	6.86 "	No. 14 Emerald Rd - Shed FL RL 6.87 and No. 14 Emerald Rd - Cottage FL RL 6.75
490	8.90	No. 28 Emerald Rd – Shed FL RL 9.02

Note: * : FL refers to surveyed Floor Level

4.7 SENSITIVITY TESTING

Parameters having a significant influence on the model predictions for flood levels were varied in order to test the sensitivity of the results to the likelihood of inundation of habitable floor levels. The parameter values tested were:-

- flow 1.2 times the adopted 1% AEP discharge estimates; and
- Manning's "n" channel and overbank roughness raised by 0.02, ie. 0.055 increased to 0.075.

The sensitivity testing runs were undertaken using the Scenario 3, model (*Ultimate Flood Mitigation Works*), the result sof which are are presented in **Appendix D**.

The increase in flow by 20% resulted in an increase in predicted 1% AEP flood level by up to 500mm. The increase in Manning's "n" roughness by 0.02 (*with original peak flow rates*) had a similar effect, with predicted 1% AEP flood levels being raised by up to 500mm higher than the adopted 1% AEP flood levels.

The sensitivity testing suggests that there is adequate freeboard to habitable floor levels to account for reasonable inaccuracies in the modelling.

4.8 EFFECT OF EXTREME FLOOD

An extreme flood (*flow twice the estimated 1% AEP peak flow rate*) for the ultimate flood mitigation works scenario would cause inundation of up to 9 sheds and garages and 1 residence along the creek.

5 **REFERENCES**

- Australian Rainfall & Runoff (AR&R), Institute of Engineers Australia, 1987
- Runoff and Flow Simulation (RAFTS), User Manual V2.8, WP Software, June 1992
- HEC-RAS River Analysis System, User Manual V2.0, US Army Corp of Engineers, April 1997
- Drainage Improvement Works Middle Creek Catchment, Pearl Beach, Boyden & Partners Pty Ltd for Gosford City Council, May 1998.
- Floodplain Development Manual, NSW Public Works Department, December 1986

FIGURES

Patterson Britton & Partners



J3144

FIGURE 1



J3144/R1958

HEC-RAS MODEL CHANNEL CROSS SECTION AND CHAINAGE.

APPROXIMATE CREEK THALWEG.

EXISTING GROUND CONTOUR LEVEL.

LOT No.

BUILDING FLOOR LEVEL.

FLOW DIRECTION.

20 m 0 1:1000 (A3)

MIDDLE CREEK FLOOD STUDY PLAN OF HEC-RAS HYDRAULIC MODEL CROSS SECTIONS.



J3144/R1958

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FIGURE 3

HEC-RAS MODEL CHANNEL CROSS SECTION

ESTIMATED PEAK FLOOD CONTOUR -- 6.5 FOR 1% AEP DESIGN EVENT (m AHD).

> ESTIMATED MAXIMUM CHANNEL VELOCITY (m/s)

APPROXIMATE CREEK THALWEG

EXISTING GROUND CONTOUR LEVEL

LOT No.

BUILDING FLOOR LEVEL.

AREA ASSUMED TO BE INUNDATED DUE TO COASTAL DUNE EFFECTS -RL 4.0m AHD.

20 m 1:1000 (A3)

MIDDLE CREEK FLOOD STUDY 1% AEP EVENT, PEAK WATER **LEVELS AND VELOCITIES FOR SCENARIO 2: PROPOSED** CULVERT WORKS ONLY, WITHOUT CHANNEL WORKS.

												EXISTIN	IG 3 CELL	ø1800 RCP-			Ś	PRECEN 2 CELL	TLY COMPLI 2400x180	ETED 0 RCBC		
	-	FLOOD	LEVELS ALO	ng middle cr	EEK ARE CON	TROLLED BY	COASTAL EFFE	CTS, FLOOD L	<u>EVEL = 4.0m</u>	AHD, UNLESS	NOTED OTHER	RWISE			\sum		1 -/				· · 7	=
			·	·	·····	T	· · · · · · · · · · · · · · · · · · ·		r						<u> </u>							
											-		-									
									41.2				· _ · · · · · · · · · · · · · · · · · ·					37.6				
									20.2 16.1	- - -					· · ·			20.1 14.8				
FLOW	(m³/s) -			<u> </u>				<u> </u>				·····			-			142	-,		<u>, , , , , , , , , , , , , , , , , , , </u>	
CHAN	NEL -				·····			NA	TURAL (0.7-3	.8)		····						0.5%		NA	JRAL (0.4 -	.7%)
	-												-				-				א '	
	L-5.000							· · · · · · · · ·														
2x1% AE	P TWL	6.00	7.00	00.4	0	4 00	00.4	4.01	7.04	4.77	4.85	2.08	5.26	17 5	5.38		S.15	5.36	16.0	6.3	6.62	7.05
ESTIMATE		00	00	00	00	00	.00	00.	00.	8	90.	15.	.46	17	61	86 150 150	150	960	36	88	36	.47
FXISTING		~ ~ ~	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	~				~ ~ ~	~ ~			~ ~									<u></u>	
5% AEP	TWL	4.0	0.4	0.4	4	40	4.0	0.4	4.0	0.4	4.0	0.4	4.2	2		400 200	S. 5	S. 3	5.1	5.4	5.9	6.21
EXISTING 20% AEP	TWL	4.00	00.4	4.00	00.4	00.7	4.00	00.4	4.00	00.4	4.00	4.00	00.4	4.16 4.16	4.15	4.25 4.610 4.650	4.720	4.800	6E.4	5.27	5.80 5.80	6.01
EXISTING	19	28	97	e e	237	5	941	504	634	512	808	966	006	951	690	780	890	026	040	80	950	050
CHANNEL	INVERT 📮					`	6						1.1	· · · · · · · · · · · · · · · · · · ·	5			<u>^</u>	ň			
CHAINAGE	0.00	15.00	00.06	45.00	00.08	75.00	90.00	105.00	120.00	135.00	150.00	164.2	178.50	197. S0	207.6	220-2 222-2 222-2	240.0	255.00	270.01	284.0	300.00	315.00
	S	CALES: HOR	Z - 1:500 V	ERT - 1:100												· · · · · · · · ·					····	·····



MIDDLE CREEK FLOOD STUDY LONGITUDINAL SECTION SHOWING 20%, 5%, 1% AEP AND EXTREME **FLOOD PROFILES FOR SCENARIO 2 : PROPOSED CULVERT WORKS ONLY, WITHOUT CHANNEL WORKS**

10 20 30 HORIZONTAL A3 1:1000 40 50 m

VERTICAL A3 1:200 0 2 4 6 8 10 m

FIGURE 4



J3144/R1958

3

HEC-RAS MODEL CHANNEL CROSS SECTION

ESTIMATED PEAK FLOOD CONTOUR -- 6.5 FOR 1% AEP DESIGN EVENT (m AHD).

> ESTIMATED MAXIMUM CHANNEL VELOCITY (m/s)

> APPROXIMATE CREEK THALWEG

EXISTING GROUND CONTOUR LEVEL

LOT No.

BUILDING FLOOR LEVEL.

AREA ASSUMED TO BE INUNDATED DUE TO COASTAL DUNE EFFECTS -RL 4.0m AHD.

> 20 m 1:1000 (A3)

MIDDLE CREEK FLOOD STUDY 1% AEP EVENT, PEAK WATER LEVELS AND VELOCITIES FOR SCENARIO 3 **ULTIMATE FLOOD MITIGATION WORKS, INCLUDING CULVERT AND CHANNEL UPGRADING**

ļ	FLOOD LEVELS ALONG MID	DLE CREEK ARE CONTROLLED B	<u>(COASTAL EFFECTS, F</u>	1000 LEVEL = 4.0m /	hd, unless noted of	EXISTING 3 CELL HERWISE	@1800 RCP		-RECENTLY COM 2400x1800 RC	IPLETED 2 CELL BC	
-				41.2					37.6		
FLOW (m³/s)				20.2 16.1 12.2				· · · · · · · · · · · · · · · · · · ·	20.1 14.8 11.2		
CHANNEL GRADE			NAT	URAL (0.7-3.8%)					0.5%	0.6%	
DATUM RL-5.000 EXTREME EVENT 2x1% AEP TWL	4,00 4,00 4,00	4.00	4.00	4.04	.85	26 08	77 9		360		
ESTIMATED ESTIMATED 1% AEP TWL EXISTING	0 4,00	0 4·00 4·00	4-00 r	000.7	4.08 4	4.31 5	1997 1997	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	5.150 5. 5.360 5.	5.36 6.1 5.27 6.1 5.27 6.1 5.49 6.3	5.65
5% AEP TWL - EXISTING	4.00 4.0	<u>4.00 4.0</u>	4.00 4.01	4.00	4.00 4.00	4.00 4.08	<u>86.,4 x1</u> 86.,4 x1	25 5130 550 55130 55130 55130	.720 5.150 800 5.360	<u>39 4.71</u> 49 4.66 49 4.66 85 5.16	2 5.33
EXISTING CHANNEL INVERT	00 1.187 00 1.166 00 1.146	00 1.237 00 1.537 00 1.153	00 0.941 00 1.504	00 1.634	00 1.512 00 1.808	25 1.866 50 1.800	2 2.139 4	29 2780 4 23 2780 4 23 2292 4 20 2.820 4	00 2.890 4 	0 3.040 4 3.000 4 3.000 4 3.510 4 0 3.710 4	3.800 5.
		- · · I ·	7/								
PROPOSED CHANNEL WORKS	37.6 20.1 14.8 112						37.6 20.1 14.8				
CHANNEL GRADE	0.6%						11.2 NATURAL (0.5	-2.3%			
DATUM RL-3.000 EXTREME EVENT 2x1% AEP TWL	6, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2, 2,		7.42 6.69 8.32 8.32 8.42	8.64	8.76	986 8	8.91 9.02	9.19 2.32	134	13 17 EB	
ESTIMATED 1% AEP TWL EXISTING 5% AEP TWI	5.47 5.78 1.62 5.94 .75 6.08	9E.0 0L.	<u>139</u> 5.55 07 6.17 07 7.58 60 7.74	99 8.21	08 8.29 10 8.31	14 8.36 7 7	42-18 C2	3 8,90 52	<u>5 8.93 1</u> 2 8.94 9	5 <u>9.15</u> 9.32 9.32 9.44, <u>9</u>	
EXISTING 20% AEP TWL	0 22 2 23 2 23 2 25 2 2 2 2	e 7 602 88 2 602 88 2	0 5.27 5 0 5.87 7 6.87 7 7.40 7	7.81	7.90 B	7.96 8.	8,15 6, 0	8.31 8. 8.59 8.7	8.61 8.7 8.64 8.7 8.64 8.7	8.78 8.9 8.90 9.11 9.07 9.24	
CHANNEL INVERT	330.00 3.89 330.00 3.89 355.00 3.97 359.00 4.05 359.00 4.05	£977 00.02E	399.00 4.76 400.00 5.26 405.00 5.74 405.00 5.75 420.00 5.756	<u> 435.00 5.662</u>	450.00 5.700 .65.00 5.674		10.00 6.517	25.00 <u>6.534</u> 40.00 6.757	55.00 6.715 0.00 6.921	0,00 7.123 0,00 7.109 5.00 7.321	

HORIZONTAL A3 1:1000 0 10 20 30 40 50 m

VERTICAL A3 1:200 2 4 6 8 10 m

J3144/R1958

MIDDLE CREEK FLOOD STUDY LONGITUDINAL SECTION SHOWING 20%, 5%, 1% AEP AND EXTREME FLOOD PROFILES FOR SCENARIO 3 : ULTIMATE FLOOD MITIGATION WORKS, INCLUDING CULVERT AND CHANNEL UPGRADING

APPENDIX A – RAFTS MODEL INPUT PARAMETERS

Patterson Britton & Partners

Run started at: 26th August 1998 17:27:07

ROUTING INCREMENT (MINS)	at 1	.60	
STORM DURATION (MINS)	=	120.	
RETURN PERIOD (YRS)	=	100.	
BX	-	.5000	
TOTAL OF FIRST SUB-AREAS	(HA)	=	56.04
TOTAL OF SECOND SUB-AREAS	G (HA)	=	3.51
TOTAL OF ALL SUB-AREAS (H	IA)	=	59.55

SUMMARY OF	CATCHMENT	AND	RAINFALL	DATA
------------	-----------	-----	----------	------

Link	Catch	. Areą	Slo	ope	<pre>% Impe:</pre>	rvious	P	ern	E	3	Link
Label	#1	#2	#1	#2	#1	#2	#1	#2	#1	#2	No.
	(hect	tares)	(१	ቴ)		(웅)					
7.0	9.540	.000	18.00	.0000	5.000	.0000	.060	.00	.0154	.0000	1.000
6.0	8.930	.000	18.00	.0000	5.000	.0000	.060	.00	.0149	.0000	1.001
5.1	2.900	.000	29.00	.0000	5.000	.0000	.060	.00	.0066	.0000	2.000
5.0	8.300	.000	23.00	.0000	5.000	.0000	.060	.00	.0127	.0000	1.002
4.11	3.680	.000	20.00	.0000	5.000	.0000	.060	.00	.0089	.0000	3.000
4.12	2.500	.000	30.00	.0000	5.000	.0000	.060	.00	.0060	.0000	4.000
4.1	1.760	.000	23.00	.0000	5.000	.0000	.060	.00	.0057	.0000	3.001
4.0	3.040	.000	15.30	.0000	5.000	.0000	.060	.00	.0092	.0000	1.003
3.0	5.330	.00100	20.00	20.00	5.000	100.0	.035	.015	.0071	0.000	1.004
2.1	2.060	.00100	6.000	6.000	5.000	100.0	.035	.015	.0079	0.000	5.000
2.2	4.800	1.720	10.00	10.00	5.000	100.0	.035	.015	.0095	.0003	6.000
2.0	2.060	1.370	1.200	1.200	5.000	100.0	.025	.015	.0139	.0008	1.005
1.0	1.140	.4200	1.500	1.500	5.000	100.0	.025	.015	.0091	.0004	1.006
Outlet	.00100	.000	1.000	.0000	5.000	.0000	.025	.00	.0003	.0000	1.007

Average Init. Loss	Cont. Loss	Excess Rain	Peak	Time Lin	k
Intensity #1 #2	#1 #2	#1 #2	Inflow	to La	q
(mm/h) (mm)	(mm/h)	(mm)	(m^3/s)	Peak (min	s)
56.075 10.00 .0000	1.500 .0000	99.272 .000	5.253	36.00 5.50	0
56.075 10.00 .0000	1.500 .0000	99.272 .000	9.185	40.20 4.00	0
56.075 10.00 .0000	1.500 .0000	99.272 .000	1.781	34.80 5.50	0
56.075 10.00 .0000	1.500 .0000	99.272 .000	12.745	40.20 6.00	0
56.075 10.00 .0000	1.500 .0000	99.272 .000	2.186	35.40 3.50	0
56.075 10.00 .0000	1.500 .0000	99.272 .000	1.541	34.80 3.500	0
56.075 10.00 .0000	1.500 .0000	99.272 .000	4.520	38.40 6.000	0
56.075 10.00 .0000	1.500 .0000	99.272 .000	17.840	45.00 6.000	С
56.075 10.00 .0000	1.500 .0000	99.272 111.96	18.488	51.00 18.00	С
56.075 10.00 .0000	1.500 .0000	99.272 111.96	1.215	35.40 3.000	С
56.075 10.00 .0000	1.500 .0000	99.272 111.96	3.882	34.80 3.000	С
56.075 10.00 .0000	1.500 .0000	99.272 111.96	20.040	69.00 8.000	5
56.075 10.00 .0000	1.500 .0000	99.272 111.96	20.206	76.80 1.000	C
56.075 10.00 .0000	1.500 .0000	99.272 .000	20.206	78.00 .0000)
	Average Init. Loss Intensity #1 #2 (mm/h) (mm) 56.075 10.00 .0000 56.075 10.00 .0000	AverageInit. LossCont. LossIntensity#1#2#1#2(mm/h)(mm)(mm/h)56.07510.00.000056.07510.00.00001.500.000056.07510.00.00001.500.000056.07510.00.00001.500.000056.07510.00.00001.500.000056.07510.00.00001.500.000056.07510.00.00001.500.000056.07510.00.00001.500.000056.07510.00.00001.500.000056.07510.00.00001.500.000056.07510.00.00001.500.000056.07510.00.00001.500.000056.07510.00.00001.500.000056.07510.00.00001.500.000056.07510.00.00001.500.000056.07510.00.00001.500.000056.07510.00.00001.500.0000	Average Init. Loss Cont. Loss Excess Rain Intensity #1 #2 #1 #2 #1 #2 (mm/h) (mm) (mm/h) (mm) (mm/h) (mm) 56.075 10.00 .0000 1.500 .0000 99.272 .000 56.075 10.00 .0000 1.500 .0000 99.272 .000 56.075 10.00 .0000 1.500 .0000 99.272 .000 56.075 10.00 .0000 1.500 .0000 99.272 .000 56.075 10.00 .0000 1.500 .0000 99.272 .000 56.075 10.00 .0000 1.500 .0000 99.272 .000 56.075 10.00 .0000 1.500 .0000 99.272 .000 56.075 10.00 .0000 1.500 .0000 99.272 .000 56.075 10.00 .0000 1.500 .0000 99.272<	AverageInit. LossCont. LossExcess RainPeakIntensity#1#2#1#2#1#2Inflow(mm/h)(mm)(mm/h)(mm)(mm/h)(mm)(m^3/s)56.07510.00.00001.500.000099.272.0005.25356.07510.00.00001.500.000099.272.0009.18556.07510.00.00001.500.000099.272.0001.78156.07510.00.00001.500.000099.272.00012.74556.07510.00.00001.500.000099.272.0001.54156.07510.00.00001.500.000099.272.0004.52056.07510.00.00001.500.000099.272.00017.84056.07510.00.00001.500.000099.272111.9618.48856.07510.00.00001.500.000099.272111.961.21556.07510.00.00001.500.000099.272111.963.88256.07510.00.00001.500.000099.272111.9620.20656.07510.00.00001.500.000099.272111.9620.20656.07510.00.00001.500.000099.272.00020.20656.07510.00.00001.500.000099.272.00020.206	AverageInit. LossCont. LossExcess RainPeakTimeLinIntensity#1#2#1#2#1#2InflowtoLa(mm/h)(mm)(mm/h)(mm)(mm/h)(mm)(m^3/s)Peak(min.56.07510.00.00001.500.000099.272.0005.25336.005.50056.07510.00.00001.500.000099.272.00091.8540.204.00056.07510.00.00001.500.000099.272.0001.78134.805.50056.07510.00.00001.500.000099.272.00012.74540.206.00056.07510.00.00001.500.000099.272.0001.54134.803.50056.07510.00.00001.500.000099.272.0001.54134.803.50056.07510.00.00001.500.000099.272.0001.54134.803.50056.07510.00.00001.500.000099.272.0001.54134.803.50056.07510.00.00001.500.000099.272.00017.84045.006.00056.07510.00.00001.500.000099.272111.9618.48851.0018.0056.07510.00.00001.500.000099.272111.963.88234.803.000

Run completed at: 26th August 1998 17:27:40

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APPENDIX B – RAFTS DISCHARGE HYDROGRAPHS

Patterson Britton & Partners



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APPENDIX C – HEC-RAS HYDRAULIC MODEL RESULTS LISTING

Patterson Britton & Partners

HEC-RAS Plan: Existing River: WATERCOURSE Reach: WATERCOURSE

	Reach River Sta	SIQ Total	Min Ch El	WS Eleve	I-CHWS	E.G. Elev	EG Slopa	Mel Chnies	Flow Area	Co Width	Emude # Ch
	11-11-11-12-12-12-12-12-12-12-12-12-12-1	(m3/s)	3.52 (m) 31 ÷	(m)o	(m)	(m)	(m/m)	(m/s)	(m2)	<u>(m)</u>	CONTRACTOR OF
	WATERCOURSE 615	11.20	7.25	9.05		9.09	0.004467	1.11	15.77	27.80	0.32
	WATERCOURSE 615	14.80	7.25	9,19		9.24	0.004269	1.14	20.02	29.78	0.32
	WATERCOURSE 615	20.10	7.25	9.37		9.41	0.004213	1.20	25.45	32.13	0.32
	WATERCOURSE, #-1616	37.60	7.25	9.80		9.85	0.004035	1.32	40.06	36.91	0.32
	WATERCOURSE600	11.20	7.03	8.85		8.98	0.011569	1 73	8.81	10.76	0.50
	WATERCOURSE	14.80	7.03	9.02		9.14	0.010307	1.73	12.27	10.70	0.50
	WATERCOURSE	20.10	7.03	9.21		0.17	0.010505	1.75	16 47	20.09	0.48
	WATERCOURSE	37.60	7.03	9.62		9.52	0.0000000	2.04	10.17	20.91	0.47
_	MATEROCORDEX 000	01.00	7.00	0.02		8.70	0.010272	2.04	25.04	24.68	0.49
		11.20	7.04	0.74			0.007000		10.01		
	WATERCOURSE	11.20	7.04	0.74		8.64	0.007030	1.48	10.34	20.58	0.40
	WATERCOURSE \$585	14.80	7.04	8.90		9.00	0.006922	1.58	14.41	28.95	0.41
-	WATERCOURSE 1985	20.10	7.04	9.10		9.20	0.006330	1.64	21.11	39.02	0.40
	WATERCOURSE 585	37.60	7.04	9.56		9.63	0.004368	1.59	41.12	43.85	0.34
	A CONTRACTOR OF										•
	WATERCOURSE 570	11.20	6.91	8.60		8.71	0.010450	1.60	8.67	13.37	0.50
	WATERCOURSE 1570	14.80	6.91	8.75		8.87	0.011033	1.74	10.82	15.19	0.52
	WATERCOURSE	20.10	6.91	8.91		9.06	0.012099	1.93	13.53	17.22	0.55
	WATERCOURSE 570	37.60	6.91	9.29		9.51	0.014885	2.39	20.95	21.83	0.63
	The second second second second										
	WATERCOURSE 1555	11.20	6.70	8.55		8.59	0.004029	0.94	14.32	29.69	0.32
	WATERCOURSE 1555	14.80	6.70	8.72		8.76	0.003430	0.96	19.62	33.21	0.02
	WATERCOURSE 1455	20.10	6.70	8.91		8.95	0.003123	1.00	25.81	33.63	0.00
	WATERCOURSE \$ 555	37.60	6.70	9.31		9.37	0.003258	1.00	39.61	34.25	0.29
	TRATEROOUNDE 1999		0.70		·		0.00200	1.21		J4.20	0.31
_	MATERICAL INC.	11 20	6 74	8.64		8 54	0 003365	0.07	19.00	20.04	
	WATERCOOKSEN ID-ID-ID-ID-ID-ID-ID-ID-ID-ID-ID-ID-ID-I	14.80	6.74	8.60		8 71	0.003205	0.01	24.62	34.01	0.28
	WATERCOORSE	14.00	6.74	0.03		8.00	0.002025	1.05	24.02	34.01	0.26
	WATERCOURSE 1940	20.10	0.74	0.07		0.90	0.002739	1.05	31.33	30.01	0.26
	WATERCOURSE 5401	37.60	0.74	9.20		9.32	0.002890	1.20	40.30	44.14	0.28
_	Line Transformer and the second		0.50				0.040070	1.00			
	WATERCOURSE 1 525	11.20	6.53	8.29		8.44	0.010373	1.82	8.00	15.19	0.51
	WATERCOURSE 525	14.80	6.53	8.46		8.62	0.010072	1.95	11.00	19.97	0.51
	WATERCOURSE 525	20.10	6.53	8.68		8.82	0.008312	1.94	17.21	33.08	0.48
_	WATERCOURSE # 525	37.60	6.53	9.13		9.25	0.006310	1.99	33.75	40.06	0.43
	A STATE AND AND A STATE OF										
	WATERCOURSE 510	11.20	6.48	8.10		8.27	0.012241	1.89	7.39	12.73	0.55
	WATERCOURSE 510	14.80	6.48	8.27		8.45	0.012310	2.06	9.71	16.23	0.56
_	WATERCOURSE 510	20,10	6.48	8.46		8.66	0.011955	2.22	13.41	21.25	0.57
	WATERCOURSE 510	37.60	6.48	8.92		9.12	0.010334	2.45	25.74	32.57	0.55
										i	
	MATERCOURSE 495	11.20	6.28	8.04		8.13	0.005662	1.33	10.60	18.29	0.39
	WATCHOOD AND A MARKED AND A MAR	14.80	6.28	8.22		8.31	0.005257	1.41	14.07	20.28	0.38
-	WATERCOURSE 444	20.10	6.28	8 43		8.52	0.005106	1.52	18.47	22.56	0.39
	WATERCOURSE #90	27.60	6.28	8.85		8 99	0.005974	1.93	29.40	29 72	0.43
	WATERCOURSE 1495		0.20				0.000314				
		44.00	6.04	7.02		8.03	0.007300	1.54	9 15	15 90	0.42
_	WATERCOURSE 400.1	11.20	6.04	9.40		9.00	0.007114	1.64	12.01	23.66	0.42
	WATERCOURSE 480	14.80	0.04	8.10		0.21	0.007114	1.04	40.00	20.00	0.42
	WATERCOURSE 480	20.10	6.04	0.33		0.44	0.006072	1.00	10.09	21.19	0.40
	WATERCOURSE 4480 1	37.60	6.04	8.78		6.89	0.005940	1.91	32.70		
_											
	WATERCOURSE 465 41	11.20	5.66	7.89		7.94	0.003081	1.03	13.15	17.91	0.28
	WATERCOURSE 465	14.80	5.66	8.07		8.13	0.003244	1.12	16.51	19.14	0.30
	WATERCOURSE 465	20.10	5.66	8.30		8.36	0.003382	1.22	20,98	20.66	0.31
	WATERCOURSE 465	37.60	5.66	8.71		8.81	0.004703	1.62	30.02	23.79	0.37
-	CALIFORNIA DE LA CALIFICACIÓN DE LA CALIFORNIA DE LA CALIFICALIFICAL DE LA CALIFICALIFICAL DE LA CALIFICAL										
	WATERCOURSE 450 450	11.20	5.69	7.87		7.89	0.002228	0.92	20.93	35.48	0.24
	WATERCOURSE	14.80	5.69	8.06		8.08	0.001945	0.93	28.17	41.01	0.23
	WATERCOURSE 450	20.10	5.69	8.29		8.31	0.001749	0.96	39.07	51.66	0.22
_	WATERCOURSE	37.60	5.69	8.72		8.75	0.001693	1.08	63.02	59.11	0.22
	WATERCOURSE 435	11.20	5.66	7.78		7.84	0,005735	1.29	12.47	17.33	0.35
	WATERCOURSE 435	14.80	5.66	7.97		8.03	0.005339	1.31	15.93	18.61	0.34
	WATERCOURSE	20 10	5.66	8.21		8.27	0.005109	1.35	20.41	20.14	0.34
	WATCOCOLOGE	37 60	5.66	8.59		8.69	0.007113	1.74	28.74	22.71	0.41
	MAILER COORSES										
		11 20	5 65	7 37		7 65	0.033464	2.51	5.36	7.96	0.73
_	WATERCOURSE 4205 AT	11.20	5.05	7.51		7 85	0.031088	2.64	6.98	9.34	0.71
_	WATERCOURSE, 420	14.80	5.00			P 07	0.027380	3.04	8.51	10 48	0.77
	WATERCOURSE 42011	20.10	5.05	1.11		0.07	0.001002	2.00	18.55	75 75	0.77
	WATERCOURSE // 420	37.60	5.65	8.25		0.49	0.028714	2.40	10.00		
							0.000007			40.00	
	WATERCOURSE 405	11.20	5.35	7.25		1.37	0.008387	1./1	9.15	13.00	0.45;
	WATERCOURSE 405	14.80	5.35	7.48		7.59	0.007229	1.75	12.65	16.95	0.43!
	WATERCOURSE 24405	20.10	5.35	7.63		7.77	0.008724	2.03	15.28	18.95	0.48

HEC-RAS Plan: Existing River: WATERCOURSE Reach: WATERCOURSE (Continued)

	Réach Ruersia	Q rotal	MINGNE	Wise elsy	CHUW.S	EGLERV	E.G. Slope	Vel Chnij	Flow Area	Top Width a	Froude # Chi
		. (m3/s)	(m) 5.05	(m)	(m),	(n)	(m/m);	(m/s)	(m2)	(m)	10,252
_	WATERONIESE	20.10	5.35	7.42		7.55	0.008708	1.87	11.63	16.11	0.47
	WATERCOURSE	37.60	5.35	7.54		1.12	0.011336	2.24	13.60	17.70	0.54
				7.14		0.13	0.022090	3,37	17.61	20.57	0.77
	WATERCOURSE 1399	11.20	5.07	7.20		7.30	0.007256	1.51	9 76	14 31	0.20
	WATERCOURSE 399	14.80	5.07	7.45		7.53	0.005458	1.46	16.44	31.17	0.35
	WATERCOURSE 399	20.10	5.07	7.60		7.68	0.005831	1.59	21.13	32.69	0.37
	WATERCOURSE 399	37.60	5.07	7.92		8.03	0.007188	1.96	32.30	36.05	0.42
_		14.20	E 07	7.00							
	MATER/COURSE 300	14.80	5.07	7.08		7.21	0.010725	1.74	8.14	12.75	0.47
	WATERCOURSE 390	20.10	5.07	7.49		7.40	0.008741	1.72	13.23	30.09	0.43
	WATERCOURSE 390	37.60	5.07	7.79		7.95	0.010752	2.30	27.57	31.55	0.45
_	and the second second second second second										
	WATERCOURSE SYE	11.20	4.42	6.67		6.94	0.022312	2.34	5.02	4.80	0.62
	WATERCOURSE 1875	14.80	4.42	6.83	6.56	7.17	0.024639	2.64	7.04	20.99	0.67
	WATERCOURSE 10/6	20.10	4.42	7.10		7.30	0.015740	2.34	14.59	32.01	0.55
	WATERCODESE 1375	37,60	4.42	7.59		1./1	0.008757	2.04	30.44	32.49	0.42
	WATERCOURSE 200	11.20	4.49	6.54		6.67	0.011010	1.62	6.01	6.12	
	WATERCOURSE 1360	14.80	4.49	6.73		6.88	0.012293	1.02	9.72	0.42 20.61	0.50
_	WATERCOURSE 860	20.10	4.49	6.97		7.10	0.010144	1.74	15.61	30.67	0.51
	WATERCOURSE 360	37.60	4.49	7.50		7.59	0.005854	1.65	33.14	33.71	0.38
	WATERCOURSE 359	11.20	4.49	6.53		6.66	0.012369	1.64	6.82	6.38	0.51
	WATERCOURSE 3593	14.80	4.49	6./1		6.87	0.012962	1.79	9.37	20.02	0.53
	WATERCOURSE 358	37.60	4.45	7 49		7.09	0.010666	1.77	15.19	29.86	0.49
						1.00	0.000000	1.00	52.84		0,39
_	WATERCOURSE 345	11.20	4.32	6.44		6.49	0.004637	1,21	13.74	23.29	0.33
	WATERCOURSE 4345	14.80	4.32	6.67		6.71	0.003437	1.16	19.34	25.06	0.29
	WATERCOURSE 345	20.10	4.32	6.93		6.97	0.002869	1.17	26.12	27.15	0.27
	WATERCOURSE 345 THOM	37.60	4.32	7.46		7.51	0.002852	1.38	41.70	31.63	0.29
		11.20	4.09	6.04			0.000550	4.57			
	WATERCOOKSE 330	14.80	4.20	6.47		6.57	0.008307	1.57	7.44	8.84	0.43
	WATERCOURSE 330	20.10	4.28	6.72		6.88	0.007933	1.72	9.00	12.90	0.44
	WATERCOURSE 330	37.60	4.28	7.24		7.43	0.008048	2.21	25.53	27.73	0.44
	To apply a series of the serie										
	WATERCOURSE 315	11.20	4.05	6.19		6.26	0.004911	1.26	9.37	10.40	0.35
	WATERCOURSER 315	14.80	4.05	6.41		6.51	0.004768	1.38	12.14	13.84	0,35
_	WATERCOURSE 315	20.10	4.05	6,67		6.78	0.004738	1.52	16.24	17.83	0.36
	WATERCOURSE 41315	37.60	4.05	/.1/		7.32	0.005723	1.95	26.88	25.75	0.41
	WATERCAURSE	11.20	3.95	6.09		6.18	0.005578	1 38	8 78	9.73	0.37
	WATERCOURSE 300	14.80	3.95	6.31		6.43	0.005572	1.52	11.36	13.41	0.38
_	WATERCOURSE 300	20.10	3.95	6.57		6.70	0.005566	1.68	15.44	18.18	0.39
	WATERCOURSE 11 300	37.60	3.95	7.04		7.23	0.007079	2.19	26.60	33.09	0.45
_	WATERCOURSE 285	11.20	4.00	6.02		6.10	0.004958	1.27	9.44	11.36	0.35
	WATERCOURSE 285	14.80	4.00	6.25		6.34	0.004669	1.38	12.46	14.70	0.35
	WATERCOORSE 289	20.10	4.00	6.96		7 11	0.004501	1.96	27.49	28.13	0.30
							0.000111			20.10	
	WATERCOURSE 284	11.20	4.00	6.01		6,10	0.005028	1.28	9.38	11.28	0.36
	WATERCOURSE 284	14.80	4.00	6.25		6.34	0.004725	1.38	12.39	14.63	0.35
	WATERCOURSE 284	20.10	4.00	6.50		6.61	0.004611	1,51	16.72	18.78	0.36
	WATERCOURSE 284	37.60	4.00	6.95		7.11	0.005786	1.96	27.32	28.03	0.42
		44.00		5.07		6.04	0.0000000	4.05	7 70		
	WATERCOURSE 220	11.20	3.00	5.67		6.01	0.008447	1.00	10.63	9.93	0.42
	WATERCOURSE 270	20 10	3.88	6.36		6.51	0.008153	1.90	14.86	18.98	0.42
_	WATERCOURSE 270	37.60	3.88	6.79		7.01	0.010893	2.48	26.34	38.97	0.50
				··· ——							
	WATERCOURSE 255	11.20	3.73	5.58		5.80	0.015883	2.13	5.86	7.73	0.57
	WATERCOURSE 265	14.80	3.73	5.83		6.07	0.014075	2.24	8.29	11.77	0.55
	WATERCOURSE 255	20.10	3.73	6.10		6.33	0.012838	2.36	12.03	16.60	0.54
	WATERCOURSE 255	37.60	3.73	6.52		6.83	0.015469	2.95	22.29	35.92	0.62
		44.00	2 10	A 79	A 79	<u> </u>	0.064224	2 3 3 1	2 27		1 00
	WATERCOUKSE 240	14.80	3.10	4.73	4.73	5.29	0.065376	3.52	4 14	3.01	1.00
	WATERCOURSE 240 PCP	20.10	3.10	5.36	5,36	6.01	0.057099	3.58	5.66	4.75	0.98
	LASS AND A MARK AND AN				1	1		1	1	1	

HEC-RAS Plan: Existing River: WATERCOURSE Reach: WATERCOURSE (Continued)

	Roach International	G Totals	Minicin El	W/S/ERV	entry's	CCG ENV	EG Slope	Velichili	Flow Area	TopWitth	Froude # Chi
	WARPEN USE	20 10	2 0 <i>1</i>	5 03	(iii)):	(m)) ***	0.007570	- (m/s) 4 70	7 (m2)	(m)	
	WATER COURSE 225	37.60	2.84	6.36		5,10	0.007579	1.70	11.83	6.77	0.41
	Move Brite States					0.11	0.002100	1.45		51.95	0.27
	WATERCOURSE 222	11.20	2.80	4.25	3.62	4.38	0.006945	1.58	7.10	6.08	0.42
-	WATERCOURSE 222	14.80	2.80	4.54	3.78	4.69	0.007198	1.73	8.55	6.12	0.42
	WATERCOURSE 222	20.10	2.80	4.96	4.00	5.14	0.007254	1.89	10.65	6.17	0.42
	WATERCOURSE 222	37.60	2.80	6.26	4.62	6.42	0.004047	1.78	21.17	6.30	0.31
		11.20	2 70	4.97	2 55	4 97	0.000000	4.40	7.00		
-	WATERCOURSE 22	14.80	2.78	4.56	3.33	4.57	0.002920	1.40	0.57	6.78	0.37
	WATERCOURSE 22	20.10	2.79	4.98	3.91	5.13	0.002537	1.70	11.83	6.87	0.37
	WATERCOURSE 221	37.60	2.79	6.28	4.49	6.41	0.002951	1.58	23.82	7.00	0.27
-	WHERE WERE STREET, CARENDALLY										
	WATERCOURSE 24	Culvert									
		12 20	2 70	4 15	9.64	4 20	0.002704	4.50	- 7.00		
	WATERCOURSE 200	12.20	2.70	4.13	3.51	4.20	0.003724	1.00	7.82	7.00	0.41
	WATERCOURSE 207	20.20	2.70	4.61	3.83	4.81	0.004084	1.96	10.29	7.10	0.45
	WATERCOURSE 207	41.20	2.70	5.38	4.51	5.79	0.005479	2.85	14.45	7.43	0.56
_	WATERCOURSE 150	12.20	1.77	3.62		3.84	0.013059	2.08	5.87	5.46	0.64
	WATERCOURSE 150	16.10	1.77	3.85		4.10	0.013079	2.23	7.23	6.04	0.65
	WATERCOURSE 150	20.20	1.//	4.07		4.35	0.012809	2.34	14 09	6.59	0.65
	WATERCOORSE	41.20		+.05		0.20	0.012200	2.04	14.30	13.00	0.07
-	WATERCOURSE 185	12.20	1.41	3.52		3.66	0.007535	1.69	7.24	5.47	0.47
	WATERCOURSE 1 135	16.10	1.41	3.74		3.92	0.008556	1.89	8.52	5.99	0.51
	WATERCOURSE 135	20.20	1.41	3.96		4.17	0.009131	2.04	9.88	6.50	0.53
_	WATERCOURSE 1854 (1854)	41.20	1.41	4.77		5.06	0.008927	2.48	19.13	23.34	0.55
-		12.20	1 50	2 21		2.54	0.010618	1.07	6.01	6.06	0.57
	WATERCOURSE 120	16.10	1.50	3.49		3.75	0.012791	2.26	7.12	5.00	0.57
	WATERCOURSE 120	20.20	1.56	3.67		3.98	0.014164	2.49	8.12	5.67	0.66
-	WATERCOURSE 120	41.20	1.56	4.04	4.04	4.84	0.029171	3.98	10.48	6.84	0.97
	WATERCOURSE 105	12.20	1.49	3.13		3.28	0.007503	1.68	7.27	6.15	0.49
	WATERCOURSE 105	16.10	1.49	3.22		3.44	0.010657	2.05	7.84	6.34	0.59
-	WATERCOURSE 105	20.20	1.49	3.33		4 47	0.013289	2.30	13.54	11.77	0.00
	WATERCOURSE 105	41.20									
	WATERCOURSE 90	12.20	0.94	3.10		3.18	0.003383	1.27	9.61	5.90	0.32
-	WATERCOURSE 90 4	16.10	0.94	3.17		3.30	0.005263	1.61	10.02	5.98	0.40
	WATERCOURSE 90.4	20.20	0.94	3.25		3.44	0.007251	1.92	10.53	6.09	0.47
	WATERCOURSE 90	41.20	0.94	3.72		4.19	0.015120	3.04	13.61	7.87	0.68
	and the second second second	12.20	1.05	3 10		3 14	0.001513	0.91	13 43	8 72	0.23
-	WATERCOURSE 75	16.10	1.05	3.16		3.23	0.002331	1.15	14.04	8.85	0.29
	WATERCOURSE \$ 75	20.20	1.05	3.25		3.35	0.003162	1.36	14.81	9.02	0.34
	WATERCOURSE 75	41.20	1.05	3.78		3.99	0.005538	2.04	21.59	21.57	0.46
_											
	WATERCOURSE 60	12.20	1.44	3.02		3.09	0.003669	1.20	10.25	10.06	0.37
	WATERCOURSE 60	16.10	- 1.44	3.04		3.16	0.006080	1.00	10.41	10.08	0.48
	WATERCOURSE 60	41 20	1.44	3.28	3.22	3.80	0.020410	3.22	13.12	12.17	0.91
-	WATERCODRSE										
	WATERCOURSE 45	12.20	1.11	3.03		3.05	0.000613	0.68	21.21	19.96	0.16
	WATERCOURSE 45	16.10	1.11	3.06		3.09	0.001003	0.88	21.74	20.24	0.21
~	WATERCOURSE 45 45	20.20	1.11	3.09		3.14	0.001454	1.07	22.45	20.62	0.25
	WATERCOURSE 45	41.20	1.11	3.43		3.55	0.002870	1.69	30.26	27.31	0.37
		12 20	1 1 2	3.00		3.04	0.001449	0.84	14.60	11.27	0.23
	WATERCOURSE 30	16.10	1.12	3.00		3.06	0.002522	1.10	14.60	11.27	0.31
-	WATERCOURSE 30	20.20	1.12	3.00		3.10	0.003968	1.38	14.61	11.27	0.39
	WATERCOURSE 30	41.20	1.12	3.00		3.41	0.016568	2.82	14.58	11.27	0.79
	The second second second										
_	WATERCOURSE	12.20	1.15	3.00		3.02	0.000555	0.58	20.94	13.93	0.15
	WATERCOURSE 15	16.10	1.15	3.00		3.03	0.001521	0.77	20.94	13.93	0.25
	WATERCOURSES 15	<u> </u>	1.15	3.00		3.20	0.006324	1.97	20.94	13.93	0.51
	WATEROOURDE: 10 22	-71.20									
-	WATERCOURSE	12.20	1.14	3.00	1.73	3.01	0.000413	0.41	29.75	28.88	0.13
	WATERCOURSE	16 10	1,14	3.00	1.83	3.01	0.000719	0.54	29.75	28.88	0.17

HEC-RAS Plan: Culv, No US River: WATERCOURSE Reach: WATERCOURSE

	Reach - Reach - River Sta	i - O Totali -	/ Min Ch El	WS ENV	CittWis	EG Elev	EG Slope	Vel Christ	Flow Areas	TONIATAN	10-2010-0000000000000000000000000000000
		(m3/s)	10+0 (m)+24	ത്രം	(m)	(m)	(m/m)	and the second s	1 Jun Mea	TOD AND U	rioude # Chi
	WATERCOURSE 615	11.20	7.25	9.05		0.00	0.004467	12	aurs) -		
-	WATERCOURSE	14.90	7.20	0.00		9.05	0.004467	1.11	15.77	27.80	0.32
	MATERCOOKSE	14.00	1.25	9.19		9.24	0.004269	1.14	20.02	29.78	0.32
	WATERCOURSE 610	20.10	7.25	9.37	1	9.41	0.004213	1.20	25.45	32 13	0.22
	WATERCOURSE 615	37.60	7.25	9.80		9.85	0.004035	1 32	40.06	02.10	0.32
	Contraction of the second s				· · · · · · · · · · · · · · · · · · ·			1.52	40.00	30.91	0.32
-	WATERCOURSE	11 20	7.02	0.05							
	TRATERCOODINGE DOW	11.20	7.03	6.65		8.98	0.011569	1.73	8.81	18.76	0.50
	WATERCOURSE 600	14.80	7.03	9.02		9.14	0.010307	1.73	12.27	20.69	0.49
	WATERCOURSE 600	20.10	7.03	9,21		9.32	0.009595	1 77	16 17	20.00	
	WATERCOURSE 1000	37.60	7.03	9.62		0.76	0.040070	1.77	10.17	20.91	0.47
_				0.02		9.70	0.010272	2.04	25.04	24.68	0.49
	A CONTRACTOR OF										
	WATERCOURSE 585	11.20	7.04	8.74		8.84	0.007030	1.48	10.34	20.58	0.40
	WATERCOURSE: 585	14.80	7.04	8.90		9.00	0.006922	1 58	14.41	29.05	0.40
	WATERCOURSE	20.10	7.04	9 10		0.20	0.006220	1.00	14.41	20.93	0.41
-	WATERCOURSE	37.60	7.04	0.50		0.20	0.000330	1.04	21.11	39.02	0.40
	WAILING DOUGLE TO SERVICE	57.00	7.04	9.50		9.63	0.004368	1.59	41.12	43.85	0.34
	en de la clara providente de la providente				[]						
	WATERCOURSE 570	11.20	6.91	8.60		8.71	0.010450	1.60	8.67	13 37	
	WATERCOURSE	14.80	6.91	8 75		8 87	0.011022	1.74	40.00		0.50
-		20.10	6.01	9.01		0.01	0.011033	1.74	10.82	15.19	0.52
	WATERCOOKSE 1970	20.10	0.91	0.91		9.06	0.012099	1.93	13.53	17.22	0.55
	WATERCOURSE 15/0	37.60	6.91	9.29		9.51	0.014885	2.39	20.95	21.83	0.63
	WATERCOURSE 555	11.20	6,70	8.55		8 59	0.004028	0.04	14 22	20.00	
-	WATERCOURSEL 555	14 80	6 70	8 72		0.70	0.000400	0.04	14.52	29.09	0.32
		14.00	0.70	0.72		0.70	0.003430	0.96	19.62	33.21	0.30
	WATERCOURSES	20.10	6.70	8.91		8.95	0.003123	1.00	25.81	33.63	0.29
	WATERCOURSE(2) 555	37.60	6.70	9.31		9.37	0.003258	1.21	39.61	34.25	0.31
	WATERCOURSENSIS	11 20	6.74	9.51		0.54	0.000005				
-	TRATERCOURCE 19-00	11.20	0.74	0.01		0,04	0.003265	0.97	18.98	30.24	0.28
	WATERCOURSE 540	14.80	6.74	8.69		8.71	0.002825	0.99	24.62	34.01	0.26
	WATERCOURSE 540	20.10	6.74	8.87		8.90	0.002739	1.05	31.33	38.01	0.26
	WATERCOURSE 1540 WATERCOURSE	37.60	6.74	9,28		9.32	0.002890	1 26	49.29	44.14	0.20
	A CONTRACT OF A CO								40.00	44.14	0.28
-	A STATE CONTRACTOR STREET										
	WATERCOURSE 525	11.20	6,53	8.29		8.44	0.010373	1.82	8.00	15.19	0.51
	WATERCOURSE 525	14.80	6.53	8.46	1	8.62	0.010072	1.95	11.00	19.97	0.51
	WATERCOURSE 525	20.10	6.53	8.68		8.82	0.008312	1 94	17.21	33.08	0.49
	MATERCOURSE IN 525	37.60	6.53	0.12		0.05	0.006240	1.04		33.00	0.40
_	MATERCOURSE 1970			5.10		8,23	0.006310	1.99	33.75	40.06	0.43
	A PARTY AND A PARTY AND A PARTY AND A										
	WATERCOURSE 510	11.20	6.48	8.10		8.27	0.012241	1.89	7.39	12.73	0.55
	WATERCOURSE 510	14.80	6.48	8.27		8.45	0.012310	2.06	9.71	16.23	0.56
	WATERCOURSE 510	20 10	6.48	8.46		0.66	0.011055	0.00		10.20	0.56
	WAILEROOD WE FIGURE	27.00	0.40	0.40		0.00	0.011955	2.22	13.41	21.25	0.57
	WATERCOURSE 12 510	37.60	6.48	8.92		9.12	0.010334	2.45	25.74	32.57	0.55
	A MARK AND A PARTY STREET										
	WATERCOURSE 495	11.20	6.28	8.04		8,13	0.005662	1.33	10.60	18 29	0.30
	MATERCOURSE 495	14.80	6.28	8 22		8.31	0.005257		14.07		
_	WATERCOOKSE	14.00	0.20	0.22		0.31	0.005257	1.41	14.07	20.28	0.38
	WATERCOURSE 4495	20.10	6.28	8,43		8.52	0.005106	1.52	18.47	22.56	0.39
	WATERCOURSE 4954	37.60	6.28	8.85		8.99	0.005974	1.93	29.40	29.72	0.43
	Werden in a standard and a serve										
	WATERCONPSES 480 AND	11 20	6.04	7 92		8.03	0.007308	1.54	0.15	45.00	
_	WATER COURCE	14.00	0.04	1.02		0.00	0.007398	1.04	9.15	15,90	0.42
	WATERCOURSE	14.80	0.04	8.10		8.21	0.007114	1.64	12,91	23.56	0.42
	WATERCOURSE 480 And	20.10	6.04	8.33	1	8.44	0.006072	1.66	18.89	27.79	0.40
	WATERCOURSE 480	37.60	6.04	8.78	.	8.89	0.005940	1.91	32.70	34.62	041
	And a second										
			E 00				0.00000.				
	WAIERCOURSE 402	11.20	5.00	1.89		7.94	0.003081	1.03	13,15	17.91	0.28
	WATERCOURSE 4465	14.80	5.66	8.07		8.13	0.003244	1.12	16.51	19.14	0.30
	WATERCOURSE 465	20.10	5.66	8.30		8.36	0.003382	1.22	20.98	20.66	0.31
	WATERCO IRSE 465	37.60	5.66	8 71		8.81	0.004703	1.62	30.02	23 70	0.27
_	WAILROOMWE						0.004703	1.02		23.19	0.37
]				
	WATERCOURSE 450 Kersel	11.20	5.69	7.87	1	7.89	0.002228	0.92	20.93	35.48	0.24
	WATERCOURSE 450%	14.80	5.69	8.06		8.08	0.001945	0.93	28.17	41.01	0.23
	WATERCOURSE 450	20.10	5.69	8.29		8.31	0.001749	0.96	39.07	51.66	0.22
		37.60	5.60	0.70		0.75	0.004002		00.07	01.00	
_	WATERCOOKSE 400	37.00	5.09	0.12		8.75	0.001693	1.08	63.02	59.11	0.22
	2月15日1月1日,1月1日日,1月1日日,1月1日日 1月1日日 - 1月1日日 -									1	
	WATERCOURSE 435	11.20	5.66	7.78		7.84	0.005734	1.29	12.47	17.33	0.35
	WATERCOI IPSE	14 80	5 66	7 97		8.03	0.005330	1 21	15.03	18.61	0.24
			E CO			0.00	0.005400				0.04
_	WATERCOURSE ADD	20.10	5.66	0.21		0.27	0.005109	1.35	20.41	20,14	0.34
	WATERCOURSE 435 435	37.60	5.66	8.59	Γ	8.69	0.007113	1.74	28.74	22.71	0.41
	Construction and the form										
	WATERCOURSE 420	11 20	5.65	7 37		7 65	0.033520	2.52	5 35	7 06	0.73
	NOVER THE REAL	11 00				7.05	0.000407	2.02	0.00		0.73
	WATERCOURSE 10/20	14.80	5.05	1.55		1.85	0.032137	2.65	6.97	9.33	0.72
	WATERCOURSE 420 420	20.10	5.65	7.71	I	8.07	0.037382	3.00	8.51	10.48	0.77
	WATERCOURSE 4201	37.60	5.65	8.25		8.49	0.029715	2.40	18.55	25.75	0.78
						 -					
	11500110		Encl				0.000.107				
_	WATERCOURSE 405	11.20	5.35	7.25		1.37	0.008425	1.72	9.14	13.84	0.45
	WATERCOURSE 405	14.80	5.35	7.48		7.59	0.007283	1.75	12.60	16.92	0.43

HEC-RAS Plan: Culv, No US River: WATERCOURSE Reach: WATERCOURSE (Continued)

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	Reach	River Sta	C Q Total	MinChel	WS. Eev	S CHIWS	CEG Elev	EG Slope	velichn)	FlowArea	Top Width	Froude # Chi
			(m3/s) 24	((m)), (m)	(m); 		(m)	(m/m), s	122 (ñvs)	(m2);	(m)	12.2136.00
	WATERCOURSE.	200	20.10	5.35	7.41		7.55	0.008798	1.88	11.5/	16.06	0.47
	WATERCOURSE	100	37.60	5.35	7.75		8.13	0.022059	3.37	17.62	20.57	0.54
	WATERCOURSE	309	11.20	5.07	7.20		7.30	0.007302	1.52	9.74	14.29	0.40
-	WATERCOURSE	399 328 84	14.80	5.07	7.45		7.53	0.005529	1.46	16.34	31.14	0.35
	WATERCOURSE	399	20.10	5.07	7.60		7.68	0.005831	1.59	21.13	32.69	0.37
	WATERCOURSE	399	37.60	5.07	7.92		8.03	0.007182	1.96	32.31	36.05	0.42
			14.00		7.07		7.04	0.040000				
	WATERCOURSE	390	11.20	5.07	7.07		7.21	0.010653	1.75	13.01	12.70	0.48
	WATERCOORSE	390	20.10	5.07	7.49		7.61	0.000432	1.74	17.60	31.55	0.45
	WATERCOURSE	391	37.60	5.07	7.79		7.95	0.010730	2.30	27.60	34.67	0.45
	the second se	A CONTRACTOR										
	WATERCOURSE	375	11.20	4.42	6.65		6.93	0.023326	2.37	4.94	4.75	0.64
	WATERCOURSE	8775	14.80	4.42	6.82	6.56	7.17	0.025643	2.68	6.79	19.91	0.68
	WATERCOURSE	375	20.10	4.42	7.09		7.30	0.016586	2.39	14.22	32.00	0.56
	WATERCOURSE	37/5	37.60	4.42	7.56		7.69	0.009546	2.11	29.53	32.46	0.44
					0.54		0.05	0.040007	4.07			
	WATERCOURSE	360	11.20	4.49	6.51		6.65	0.012937	1.67	6./1	6.33	0.52
	WATERCOURSE	360	20.10	4.48	6.01		7.07	0.013257	1.80	9.22	19.77	0.53
-	WATERCOURSE	960 1 200	37.60	4.49	7.46		7.56	0.006564	1.72	31.79	33.62	0.52
	WATERCOOKSE											
	WATERCOURSE	359	11.20	4.49	6.49	······································	6.64	0.013487	1.70	6.60	6.28	0.53
	WATERCOURSE	359	14.80	4.49	6.68		6.85	0.014411	1.86	8.69	18.81	0.55
	WATERCOURSE	359	20.10	4.49	6.89		7.06	0.013239	1.91	13.55	26.58	0.54
	WATERCOURSE	359	37.60	4.49	7.45		7.56	0.006693	1.73	31.57	33.60	0.41
	and the second second	Service and										
_	WATERCOURSE	345	11.20	4.32	6.37		6.44	0.006137	1.35	12.19	22.78	0.38
	WATERCOURSE	345	14.80	4.32	6.60		6.03	0.004361	1.27	24.20	24.03	0.33
	WATERCOURSE	345	20.10	4.32	7 41		7 47	0.003124	1.43	40.32	31.26	0.30
	WATERCOURSE	0451	57.00					0.000124			01.20	
_	MATERCONRSE	330	11.20	4.28	6.11		6.27	0.012335	1.76	6.41	6.67	0.51
	WATERCOURSE	330	14.80	4.28	6.31		6.50	0.012521	1.97	8.07	9.99	0.53
	WATERCOURSE	330	20.10	4.28	6.56		6.78	0.012057	2.15	11.14	14.62	0.53
	WATERCOURSE	330.00	37.60	4.28	7.14		7.37	0.010082	2.41	22.91	25.83	0.51
	Martin Martin	Contraction of										
	WATERCOURSE	915	11.20	4.05	6.01		6.12	0.008045	1.46	7.74	7.94	0.43
	WATERCOURSE	315	14.80	4.05	6.21		6.35	0.007932	1.62	9.69	10.81	0.44
	WATERCOURSE	815	20.10	4.05	6.47		6.63	0.007637	1.78	12.96	14.72	0.45
	WATERCOURSE	315	37.60	4.05	7.05		1.24	0.007242		24.02	23.09	0.40
			11 20	2 05	5.80		5.96	0.012323	1 77	6.41	6.80	0.53
	WATERCOURSE	12001212	11.20	3.95	5.99		6.19	0.012557	1.98	7.89	8.71	0.55
	WATERCOURSE	300	20.10	3.95	6.23		6.47	0.012640	2.22	10.28	11.83	0.56
	WATERCOURSE	300	37.60	3.95	6.82		7.10	0.011217	2.58	20.47	23.42	0.56
		a de la compañía de l										
	WATERCOURSE	2851	11.20	4.00	5.46		5.71	0.025510	2.20	5.09	5.81	0.75
	WATERCOURSE	285	14.80	4.00	5.66		5.94	0.025123	2.35	6.31	6.44	0.76
	WATERCOURSE	285	20.10	4.00	5.90		6.23	0.022473	2.54	8.18	9.63	0.74
	WATERCOURSE	285	37.60	4.00	6.49		. 6.87	0.016685	2.86	10.40	10.00	0.00
		dia da la companya da comp	11 20	4.00	5 27	5 27	5.66	0.047815	2.78	4.02	5.19	1.01
	WATERCOURSE	284	14.80	4.00	5.45	5.45	5.89	0.046247	2.95	5.02	5.77	1.01
	WATERCOORSE	284	20.10	4.00	5.68	5.68	6.18	0.043982	3.13	6.43	6.50	1.00
	WATERCOURSE	284	37.60	4.00	6.30	6.30	6.84	0.026439	3.35	13.25	15.53	0.84
	MATERICOUROUS											
_	WATERCOURSE	270 1 270	11.20	3.05	4.39	3.84	4.50	0.005729	1.47	7.64	7.79	0.43
	WATERCOURSE	270	14.80	3.05	4.71	3.99	4.83	0.004888	1.51	9.83	8.22	0.41
	WATERCOURSE	- 27/0	20.10	3.05	5.36	4.19	5.45	0.002976	1.37	14.63	9.08	0.32
	WATERCOURSE	270	37.60	3.05	6.44	4.74	6.53	0.001808	1.35	27.91	10.00	0.26
	No. of Contraction											
	WATERCOURSE	245 .	Culvert					<u> </u>				
	The sector of the	BIR COLUMN			4.05	360	4 20	0.006045	1 59	7 10	6.08	0.42
	WATERCOURSE	222	11.20	2.80	4.25	3.02	4.00	0.000345	1.30	8.55	6.12	0.42
	WATERCOURSE	244	20.10	2.00	4.04	4.00	5.14	0.007254	1.89	10.65	6.17	0.42
	WATERCOURSE	2002	37.60	2.80	6.26	4.62	6.42	0.004047	1.78	21.17	6.30	0.31
	WATERCOURSE							,,,, ===				
	MATEROOIDEE	221	11.20	2.79	4.27	3.55	4.37	0.002928	1.40	7.98	6.78	0.37
	MALERY COLOE		3	2.70	4.56	3 70	4.68	0.002793	1.55	9.57	6.82	0.37

HEC-RAS Plan: Culv, No US River: WATERCOURSE Reach: WATERCOURSE (Continued)

Reach	River Sta	- Ostotaliss	Min ChiEle	W.S. Elev.	CHWSS	EGIEIW	EGSlope	Vel Chnick	Eldw Area	Too Wath	Emude # Ch
		(m3/s)	(m)	(m)	(m)	(m)	£1.(m/m)	(m/s)	(m2)	(m)	
WATERCOURSE	20.9	Culvert									The providence of the second
WATERCOURSE	207	12.20	2.70	4.15	3.51	4.28	0.003724	1.56	7.82	7.06	0.4
WATERCOURSE	207	16.10	2.70	4.39	3.67	4.55	0.003902	1.77	9.10	7.13	0.43
WATERCOURSE	207	20.20	2.70	4.61	3.83	4.81	0.004084	1.96	10.29	7.20	0.4
WATERCOURSE	-207	41.20	2.70	5.38	4.51	5.79	0.005479	2.85	14.45	7.43	0.56
WATERCOURSES	150	12.20	1.77	3.62		3.84	0.013059	2.08	5.87	5.46	0.64
WATERCOURSE	(60)	16.10	1.77	3.85		4.10	0.013079	2.23	7.23	6.04	0.65
WATERCOURSE	160	20.20	1.77	4.07		4.35	0.012809	2.34	8.64	6.59	0.65
WATERCOURSE	150	41.20	1.77	4.85		5.26	0.012286	2.84	14.98	13.05	0.6
WATTERCOURSE	135	12.20	1.41	3.52		3.66	0.007535	1.69	7.24	5.47	0.47
WATERCOURSE	85	16.10	1.41	3.74		3.92	0.008556	1.89	8.52	5.99	0.5
WATERCOURSE	(36	20.20	1.41	3.96		4.17	0.009131	2.04	9.88	6.50	0.53
WATERCOURSE	36	41.20	1.41	4.77		5.06	0,008927	2.48	19.13	23.34	0.55
WATERCOURSE	120	12.20	1.56	3.31		3.51	0.010618	1.97	6.21	5.06	0.57
WATERCOURSE	.20	16.10	1.56	3.49		3.75	0.012791	2.26	7.12	5.36	0.63
WATERCOURSE	120	20.20	1.56	3.67		3.98	0.014164	2.49	8.12	5.67	0.66
WATERCOURSE	120	41.20	1.56	4.04	4.04	4.84	0.029171	3.98	10.48	6.84	0.97
WATERCOURSE	105	12.20	1.49	3.13		3.28	0.007503	1.68	7.27	6.15	0.49
WATERCOURSE	105.	16.10	1.49	3.22		3.44	0.010657	2.05	7.84	6.34	0.59
WATERCOURSE	105	20.20	1.49	3.33		3.62	0.013289	2.36	8.55	6.57	0.66
WATERCOURSE	105	41.20	1.49	3.98		4.47	0.016191	3.10	13.54	11.77	0.75
WATERCOURSE	90) 5	12.20	0.94	3.10		3.18	0.003383	1.27	9.61	5.90	0.32
WATERCOURSE	90	16.10	0.94	3.17		3.30	0.005263	1.61	10.02	5.98	0.40
WATERCOURSE	90;	20.20	0.94	3.25		3.44	0.007251	1.92	10.53	6.09	0.47
WATERCOURSE	90 5	41.20	0.94	3.72		4.19	0.015120	3.04	13.61	7.87	0.68
WATERCOURSE	75	12.20	1.05	3.10		3.14	0.001513	0.91	13.43	8.72	0.23
WATERCOURSE	75	16.10	1.05	3.16		3.23	0.002331	1.15	14.04	8.85	0.29
WATERCOURSE	76	20.20	1.05	3.25		3.35	0.003162	1.36	14.81	9.02	0.34
WATERCOURSE	75	41.20	1.05	3.78		3.99	0.005538	2.04	21.59	21.57	0.46
	104 (125 (1-4)) 11 - 41	1							10.05		
WATERCOURSE	60 and 10	12.20	1.44	3.02		3.09	0.003669	1.20	10.25	10.06	0.37
WATERCOURSE	60	16.10	1.44	3.04		3.16	0.006080	1.56	10.41	10.08	0.48
WATERCOURSE	60	20.20	1.44	3.06		3.24	0.008993	1.92	10.63	10.26	0.59
WATERCOURSE	60	41.20	1.44	3.28	3.22	3.80	0.020410	3.22	13.12	12.17	0.91
		2									
WATERCOURSE!	45	12.20	1.11	3.03		3.05	0.000613	0.68	21.21	19.96	0,16
WATERCOURSE	45	16.10	1.11	3.06		3.09	0.001003	0.88	21.74	20.24	0.21
WATERCOURSE	45	20.20	1.11	3.09		3.14	0.001454	1.07	22.45	20.62	0,25
WATERCOURSE	45	41.20	1.11	3.43		3.55	0.002870	1.69	30.26	27.31	0.37
		ê									
WATERCOURSEA	307.1	12.20	1.12	3.00		3.04	0.001449	0.84	14.60	11.27	0.23
WATERCOURSE	30	16.10	1.12	3.00		3.06	0.002522	1.10	14.60	11.27	0.31
WATERCOURSE	30	20.20	1.12	3,00		3.10	0.003968	1.38	14.61	11.27	0.39
WATERCOURSE	30	41.20	1.12	3.00		3.41	0.016568	2.82	14.58	11.27	0.79
Call of the second second	a punte de la						0.000000				0.15
WATERCOURSE	b 15 c = € (a)	12.20	1.15	3.00		3.02	0.000555	0.58	20.94	13.93	0.15
WATERCOURSE	15 3	16.10	1.15	3.00		3.03	0.000966	0.77	20.94	13.93	0.20
WATERCOURSE	15 3 22	20.20	1.15	3.00		3.05	0.001521	0.96	20.94	13.93	0.25
WATERCOURSE	15	41.20	1.15	3.00		3,20	0.006324	1.97	20.94	13.93	0.51
	$(p, q_{n}) \in \mathcal{O}_{n}$						0.0001110			00.00	
WATERCOURSE	02.5	12.20	1.14	3.00	1.73	3.01	0.000413	0.41	29.75	28.88	0.13
WATERCOURSE	0	16.10	1.14	3.00	1.83	3.01	0.000719	0.54	29.75	28.88	0.1/
WATERCOURSE	4 04 6 10 1	20.20	1.14	3.00	1.92	3.02	0.001132	0.68	29.75	28.68	0.21
WATERCOURSE	0	41.20	1.14	3.00	2.33	3.10	0.004709	1.38	29.75	28.88	0.44

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HEC-RAS Plan: Ult Culv US River: WATERCOURSE Reach: WATERCOURSE

	Reach	River Sta	. O lotal	, Minichi E	West Star	CHUW/S.	EG EW	1 E.G. SM.	Varenni	Flow Area	Top Width	Froude # Chil:
				(ii)	60)	(m),	(iii)	(m/m)	(m/s)	(m2)	(iii) - (iii)	536 / A. R. P.
	WATERCOURSE	013	11.20	7.25	9.05		9.09	0.004467	1.11	15.77	27.80	0.32
	WATERCOURSE	at is	14.80	7.25	9.19		9.24	0.004269	1.14	20.02	29.78	0.32
	WATERCOURSE	66	20.10	7.25	9.37		9.41	0.004213	1.20	25.45	32.13	0.32
	WATERCOURSE	86	37.60	7.25	9.80		9.85	0.004034	1.32	40.06	36.92	0.32
	WATERCOURSE	600	11.20	7.03	8.85	••••••••••••••••••••••••••••••••••••••	8.98	0.011569	1.73	8.81	18.76	0.50
	WATERCOURSE	600	14.80	7.03	9.02		9.14	0.010307	1.73	12.27	20.69	0.48
	WATERCOURSE	0.00	20.10	7.03	9.21		9.32	0.009595	1.77	16.17	20.91	0.47
	WATERCOURSE	600	37.60	7.03	9,62		9.76	0.010267	2.04	25.05	24.68	0.49
	WATERCOURSE	585	11.20	7.04	8.74		8.84	0.007029	1.48	10.35	20.58	0.40
	WATERCOURSE	585	14.80	7.04	8.90		9.00	0.006922	1.58	14.41	28.95	0.41
	WATERCOURSE	685	20.10	7.04	9.10		9.20	0.006331	1.64	21.11	39.02	0.40
_	WATERCOURSE	685	37.60	7.04	9.56		9.63	0.004364	1.59	41.14	43.85	0.34
		La editoria	44.00	0.01			0.74	0.010448	4.60		42.07	
	WATERCOURSE	570 a c deta	11.20	6.91	0.00		0.71	0.0110440	1.60	10.07	13.3/	0.50
	WATERCOURSE	57.0	14.80		0.75		0.07	0.011032	1.74	10.62	15.19	0.52
	WATERCOURSE	57/0	20.10	0.91	0.81		0.6	0.012101	1.83	10.02		0.55
	WATERCOURSE	57/0	37,60	0.91	8.29		9.01	0.014659	2.39		21.04	0.63
			44.00	07.0	0.55		9.50	0.004027	0.04	14.22	20.70	
	WATERCOURSE	566	11.20	6.70	6.55		8.09	0.004027	0.84	14.32	29.70	0.32
_	WATERCOURSE	1000	14.00	6.70	9.01		8.05	0.003430	1.00	25.81	33.21	0.30
	WATERCOURSE	000	20.10	0.10	0.01		0.00	0.003123	4.00	30 63	34.95	0.29
	WATERCOURSE	790	37.00	6.70	8,01			0.000200	1.41		G2.#D	0.31
		(5/0)	11 20	6.74	8.51		8 54	0.003264	0.97	18 98	30.24	0.28
	WATERCOURSE	040	14.80	6 74	8.69		8 71	0.002825	0.99	24.62	34.01	0.26
	WATERCOURSE	Self.	20.10	6.74	8.87		8.90	0.002740	1.05	31.33	38.01	0.26
	WATERCOORSEA	540	37.60	6.74	9.28		9.32	0.002884	1.25	48.42	44.14	0.28
	WATERCOURSE	540										
	WATERCOURSE	1000 (1000) (1000) 1000 (1000)	11.20	6.53	8.29		8.44	0.010365	1.82	8.01	15.20	0.51
	WATERCOURSE	625	14.80	6.53	8.46		8.62	0.010070	1.95	11.00	19.98	0.51
	WATERCOOKSE	525	20.10	6.53	8.68		8.82	0.008319	1.94	17.20	33.08	0.48
	WATERCOORDE	525	37.60	6.53	9.14		9.25	0.006286	1.98	33.80	40.08	0.43
	MATERCOOKSE							·····				
	WATERCOURSE	510) 7 M	11.20	6.48	8.11		8.27	0.012203	1.89	7.40	12.75	0.55
	WATERCOUNCE	510	14.80	6.48	8.27		8.45	0.012299	2.06	9.71	16.23	0.56
	WATERCOORSE	510	20.10	6.48	8.46		8.66	0.011991	2.22	13.39	21.23	0.57
	WATERCOURSE	510 510 57	37.60	6.48	8.92		9.12	0.010206	2.44	25.87	32.67	0.55
	WATERCOOKSE											
	WATERCOURSE	495	11.20	6.28	8.04		8.13	0.005634	1.33	10.63	18.30	0.39
	WATERCOURSE	495	14.80	6.28	8.22		8.31	0.005251	1.41	14.08	20.29	0.38
	WATERCOURSE	495	20.10	6.28	8.43		8.52	0.005123	1.52	18.45	22.55	0.39
	WATERCOURSE	495	37.60	6.28	8.86		8.99	0.005904	1.92	29.54	29.80	0.43
		No.										
	WATERCOURSE	480.	11.20	6.04	7.92		8.03	0.007347	1.53	9.18	16.02	0.42
	WATERCOURSE	480	14.80	6.04	8.10		8.22	0.007097	1.64	12.92	23.58	0.42
	WATERCOURSE	480	20.10	6.04	8.33	·	8.44	0.006106	1.67	18.84	27.76	0.40
	WATERCOURSE	480	37.60	6.04	8.78		8.90	0.005844	1.90	32.92	34.72	0.41
	Contraction of the local distance of the loc	Contraction (
-	WATERCOURSE	1465.	11.20	5.66	7.89		7.94	0.003059	1.03	13.19	17.93	0.28
	WATERCOURSE	465	14.80	5.66	8.07		8,13	0.003237	1.12	16.52	19.14	0.30
	WATERCOURSE	465 7 7	20.10	5.66	8.29		8.36	0.003398	1.22	20.94	20.65	0.31
	WATERCOURSE	465	37.60	5.66	8.71		8.82	0.004630	1.61	30.20	23.86	0.37
			¥					0.000004			00.00	
	WATERCOURSE	450.	11.20	5.69	7.87		7.90	0.002204	0.92	21.03	35.56	0.24
	WATERCOURSE	450	14.80	5.69	8.06		8.08	0.001939	0.93	28.20	41.03	0.23
	WATERCOURSE	450	20.10	5.69	8.29		8.31	0.001762	0.96	38.96	51.63	0.22
	WATERCOURSE	450	37.60	5.69	8.73		8.75	0.001661	1.07	63.46	59.24	
							7.04	0.005057		40.54	17.00	0.25
	WATERCOURSE	485	11.20	5.66	7.79		7.04	0.005657	1.20	12.04	19.62	0.34
	WATERCOURSE	435	14.80	5.66	7.98		8.03	0.005322	1.31	10.90	10.02	0.34
_	WATERCOURSE	435	20.10	5.66	8.20		8.20	0.005145	1.30	20.30	20.12	0.54
	WATERCOURSE	435	37.60	5.66	8.60		0.70	106600.0	1.72	20.90		0.40
		6.24			7 14		7 67	0.026086	2 20	5 09	8 57	0.65
	WATERCOURSE	1420	11.20	5.65	1.44		7.07	0.020000	2.23	7 97	0.02	0.68
	WATERCOURSE	- 129	14.80	5.65	1.08		06.1 0 0 0	0.020202	2.00	8 07	10 77	0.30
	WATERCOURSE	420	20.10	5.65	1.15		60.0 + a a	0.000424	2.00	20.05	26.98	0.73
	WATERGOURSE	420	37.60	5.05	0.31		0.01		<u> </u>	20,00	20.00	
		105	11 20	6 36	6.87	6.87	7.23	0.031685	2.73	4.88	8.68	0.83
	WATERCOURSE		11.20	5.35	7.07	7 07	7 42	0.027564	2.84	6.80	11.30	0.80
	WATERCOURSE	TAVOLENT	14.00	0.00	7.07	7.07	7.14	0.025414	2.01	0.42	14.11	0.70

HEC-RAS Plan: Ult Culv US River: WATERCOURSE Reach: WATERCOURSE (Continued)

	River Stat		Min Ch El	WYS Elev	CHWSH	E.G. Elev	EGI SIODA	Well Chink	Flow Area	Topwykith	Froude # Ch
		#4 (m3/s) -	(m)	(m) ^v	(m):#**	(m)	(m/m)	(m/s) .	(m2).	(11)	
	WATERCOURSE 400	14.80	5.26	5.96	6.31	7.11	0.126663	4.76	3.11	4.93	1.92
	WATERCOURSE 400	20.10	5.26	6.17	6.53	7.34	0.096496	4.79	4.19	5.22	1.71
	WATERCOURSE 100	37.60	5.26	6.94	7.12	7.91	0.043725	4.37	8.60	6.24	1.19
_	WATERCOURSE 399	11.20	4.76	5.26	5.64	6,61	0.212374	5.15	2.17	4.67	2.41
	WATERCOURSE 399	14.80	4.76	5.38	5.81	6.89	0.189554	5.45	2.72	4.82	2.32
	WATERCOURSE \$99	20.10	4.76	5.55	6.03	7.16	0.154393	5.63	3.57	5.05	2.14
	WATERCOURSE \$99	37.60	4.76	7.43		7.73	0.008799	2.44	15.43	7.56	0.54
_	WATERCOURSE \$20	11.20	4.71	6.05		6.20	0.008241	1.70	6.57	5.79	0.51
	WATERCOURSE 390	14.80	4.71	6.27		6.45	0.008644	1.88	7.88	6.08	0.53
	WATERCOURSE 1890	20.10	4.71	6.55		6.77	0.009139	2.09	9.62	6.45	0.55
	WATERCOURSE 1390	37.60	4.71	7.33		7.65	0.009324	2.49	15.10	7.50	0.56
	WATERCOURSE 876	11.20	4.63	5.89		6.06	0.010130	1.83	6.11	5.68	0.56
	WATERCOURSE 375	14.80	4.63	6,10		6.31	0.010605	2.02	7.32	5.96	0.58
	WATERCOURSE B76	20.10	4.63	6.36		6.62	0.011215	2.25	8.94	6.31	0.60
	WATERCOURSE 375	37.60	4.63	7.14		7.50	0.010882	2.64	14.26	7.35	0.60
	WATERCOURSE S60	11.20	4.55	5.43	5.43	5.82	0.033634	2.78	4.03	5.17	1.00
	WATERCOURSE 360	14.80	4.55	5,60	5.60	6.06	0.032853	3.01	4.92	5.40	1.00
	WATERCOURSE 7 360 4	20.10	4.55	5.85	5.82	6.36	0.029398	3.17	6.34	5.74	0.96
	WATERCOURSE 960	37.60	4.55	6.86		7.30	0.014572	2.94	12.81	7.08	0.70
	Station Server and Server 1995										
	WATERCOURSE 359	11.20	4.05	5.50		5.62	0.006341	1.55	7.21	5.94	0.45
	WATERCOURSE 359	14.80	4.05	5.75		5.89	0.006529	1.70	8.71	6.26	0.46
	WATERCOURSE 959	20.10	4.05	6.07		6.25	0.006594	1.86	10.83	6.70	0.47
	WATERCOURSE 359	37.60	4.05	7.00		7.23	0.006192	2.14	17.57	7.93	0.46
_	WATERCOURSE 345	11.20	3.97	5.38		5.53	0.007774	1.68	6.67	5.42	0.48
	WATERCOURSE 345	14.80	3.97	5.62		5.79	0.008253	1.86	7.96	5.65	0.50
	WATERCOURSE 345	20.10	3.97	5.93		6.14	0.008679	2.06	9.75	5.96	0.51
	WATERCOURSE 345	37.60	3.97	6.82		7.12	0.008658	2.43	15.48	6.85	0.52
-	· · · · · · · · · · · · · · · · · · ·										
	WATERCOURSE 330	11.20	3.89	5.25		5.40	0.008924	1.76	6.35	5.36	0.52
	WATERCOURSE - 330	14.80	3.89	5.47		5.66	0.009516	1.96	7.56	5.58	0.54
	WATERCOURSE 380	20.10	3.89	5.76		6.00	0.010034	2.17	9.25	5.87	0.55
~	WATERCOURSES 3301	37.60	3.89	6.67		6.99	0.009473	2.51	14.97	6.78	0.54
	WATERCOURSE 315 TANK	11.20	3.80	5.12		5.27	0.008805	1.74	6.42	5.76	0.53
	WATERCOURSE 315	14.80	3.80	5.33		5.52	0.009189	1.92	/./1	6.05	0.54
	WATERCOURSE 315	20.10	3.80	5.63		5.86	0.009334	2.10	9.55	6.44	0.55
	WATERCOURSE	37.60	3.80	6.57		6.85	0.007703	2.32	16.20	7.70	0.51
	经合理法 在自己的 机合理 机合理 化						0.010770				
	WATERCOURSE 300	11.20	3.71	4.95		5.13	0.010770	1.87	5.98	5.65	0.58
_	WATERCOURSE 300	14.80	3.71	5.16		5.37	0.011219	2.00	7.10	5.93	0.60
	WATERCOURSE 300	20.10	3.71	5.44		5.70	0.011227	2.20	45.00	7.66	0.00
	WATERCOURSE 300	37.60	3.71	0.45		0.73	0.007992	2.30	10.90	7.00	0.52
		44.00	2.04		4.40	4 9 9	0.033557	2.78	4.03		1.00
_	WATERCOURSEA 285 March	11.20	3.01	4.49	4.40	4.00	0.000002	2.10	4.03	5.17	1.00
	WATERCOURSE 285	14.60	3.01	4.00 E 14	4.00	5.12	0.032703	2.00	7.70	6.40	0.74
	WATERCOURSE 27 285 CARS	20.10	3.01	6 32		6.61	0.010370	2.01	15.78	7.62	0.53
	WATERCOURSE 285			0.02							
		11 20	- 311	4 45		4.51	0.003235	1.14	9.82	8.68	0.34
	WATERCOURSE: 204 204	14.90	3 11	4.40		4.81	0.002722	1 17	12.66	9.31	0.32
	WATERCOORSE 284	20.10	3.11	5 34		5.40	0.001767	1 10	18.33	10.45	0.26
	WATERCOURSE 284	20.10		6.47		6.55	0.001370	1 20	31.36	12 00	0.24
_	WATERCOURSE 204						0.001010				
		11 20	3.05	4 33	3.84	4 45	0.006664	1.54	7.25	7.71	0.47
	WATERCOORSE 2700	14.80	3.05	4.66	3.99	4 78	0.005505	1.57	9.43	8.14	0.43
	WATERCOURSE 270	20.10	3.05	5.26	4 19	5 36	0.003476	1.45	13.83	8.94	0.35
	WATERCOURSE 2/User 10	20.10	3.05	6.43	4.13 A 7A	6.50 6.52	0.001833	1.35	27.77	10.00	0.26
	WATERCOURSE 210										
	ALL STREAM STREET	Culver									{
	WATERCOURSE 240	Cuiven									
	TO A CALL AND A DAY OF A DAY	11 20	2 80	4 17	3.62	4.31	0.008276	1.68	6.68	6.07	0.46
	WATERCOURSE	14 80	2.00	4 46	3.78	4.63	0.008223	1.82	8.15	6.11	0.45
	WATERCOURSE	20.10	2.00	4.97	4.00	5.07	0.008184	1.97	10.18	6.16	0.44
	WATERCOURSE 222	37 60	2.00	6.24	4 62	6 40	0.004121	1.79	21.02	6.30	0.31
	WALERCOURSE										
_	THE REPORT OF THE PARTY OF	11 20	2 70	4 18	3.55	4.30	0.003541	1.49	7.54	6.77	0.40
	WATERCOURSE	11.20	2.19	4.10	3 70	4.62	0.003241	1.62	9 15	6.81	0.40

HEC-RAS Plan: Ult Culv US River: WATERCOURSE Reach: WATERCOURSE (Continued)

	Reach Reach River Sta		Minichel	WWS Elev	Cill Wis:	E.G. Elevi	E.G. Slope	Vel Chnill	LETOW Area	Top Width	ElDude # Ch
		(m3/s)	(m)	(ji)	(m)		·····(m/m)·····	(៣/ភ)	(m2)	(m)	
	WATERCOURSE 214	Culvert									
					· · · · · · · · · · · · · · · · · · ·						
	WATERCOURSE 207	12.20	2.70	4.05	3.51	4.19	0.004802	1.68	7.24	7.02	0.46
	WATERCOURSE 207	16.10	2.70	4.30	3.67	4.48	0.004668	1.87	8.63	7.10	0.47
	WATERCOURSE 207	20.20	2.70	4.53	3.83	4.75	0.004678	2.05	9.88	7.18	0.48
	WATERCOURSE 207	41.20	2.70	5.32	4.51	5.76	0.005870	2.91	14.15	7.42	0.57
						i					
	WATERCOURSE (60	12.20	1.77	3.59		3.82	0.013850	2.13	5.74	5.40	0.66
	WATEROOURSE 150	16.10	1.77	3.82		4.09	0.013974	2.28	7.05	5.97	0.67
	WATTERCOURSE 160	20.20	1.77	4.04		4.33	0.013741	2.40	8.41	6.50	0.67
	WATERCOURSE. 150	41.20	1.77	4.84		. 5.25	0.012440	2.85	14.90	12.90	0.68
	The second second second second										
	WATERCOURSE 185	12.20	1.41	3.48		3.63	0.008185	1.74	7.01	5.37	0.49
_	WATERCOURSE	16.10	1.41	3.69		3.89	0.009361	1.96	8.23	5.88	0.53
	WATERCOURSE 1105	20.20	1.41	3.91		4.14	0.010014	2.12	9.54	6.38	0.55
	WATERCOURSE 185	41.20	1.41	4.76		5.06	0.008943	2.48	19.04	22.02	0.56
	WATERCOURSE 120	12.20	1.56	3.22		3.45	0.012947	2.12	5.77	4.92	0.62
	WATERCOURSE! 120	16.10	1.56	3.35		3.67	0.016914	2.51	6.42	5.13	0.72
	WATERCOURSE 120	20.20	1.56	3.49		3.90	0.019787	2.82	7.17	5.37	0.78
	WATERCOURSER 120 + 121	41.20	1.56	4.11	4.04	4.85	0.025520	3.81	11.04	9.97	0.91
_	The second second second second						·	····			······
	WATERCOURSE 105	12.20	1.49	3.13		3.28	0.007496	1.68	7.28	6.15	0.49
	WATERCOURSE: 105	16.10	1.49	3.23		3.44	0.010643	2.05	7.85	6.34	0.59
	WATERCOURSE 405	20.20	1.49	3.33		3.62	0.013267	2.36	8.55	6.57	0.66
	WATERCOURSE 105	41.20	1.49	3.98		4.47	0.016194	3.10	13.54	11.77	0.75
	The second states of the second second										· · ·
	WATERCOURSE 90	12.20	0.94	3.10		3.18	0.003380	1.27	9.62	5.90	0.32
	WATERCOURSE 90	16.10	0.94	3.17		3.30	0.005258	1.61	10.03	5.98	0.40
	WATERCOURSE 90	20.20	0.94	3.25		3.44	0.007240	1.92	10.54	6.09	0.47
	WATERCOURSE 90	41.20	0.94	3.72		4.19	0.015124	3.04	13.61	7.87	0,68
								·			
	WATERCOURSE 751	12.20	1.05	3.10		3.14	0.001512	0.91	13.44	8.72	0.23
	WATERCOURSE 75	16.10	1.05	3.17		3.23	0.002329	1.15	14.05	8.85	0.29
	WATERCOURSE 75	20.20	1.05	3.25		3.35	0.003157	1.36	14.82	9.02	0.34
	WATERCOURSE 1751	41.20	1.05	3.78		3.99	0.005540	2.04	21.59	21.57	0.46
					<u>, , , , , , , , , , , , , , , , , , , </u>						
	WATERCOURSE	12.20	1.44	3.02		3.09	0.003662	1.20	10.25	10.06	0.37
-	WATERCOURSE 60	16.10	1.44	3.04		3.16	0.006062	1.56	10.42	10.09	0.48
	WATERCOURSED 80	20.20	1.44	3.06		3.24	0.008942	1.91	10.65	10.28	0.59
	WATERCOURSE 60	41.20	1.44	3.28	3.22	3.80	0.020223	3.21	13.16	12.20	0.90
-	WATERCOURSE 45	12.20	1.11	3.03		3.05	0.000612	0.68	21.22	19.97	0.16
	WATERCOURSE 45	16.10	1.11	3.06		3.09	0.001001	0.88	21.76	20.26	0.21
	WATERCOURSE 45	20.20	1.11	3.10		3.14	0.001448	1.07	22.49	20.64	0.25
	WATERCOURSE 45	41.20	1.11	3.44		3.55	0.002853	1.68	30.34	27.35	0.37
_											·····
	WATERCOURSE 30	12.20	1.12	3.00		3.04	0.001447	0.84	14.60	11.27	0.23
	WATERCOURSE	16.10	1.12	3.00		3.06	0.002516	1.10	14.62	11.28	0.31
	WATERCOURSE 30	20.20	1.12	3.00		3.10	0.003950	1.38	14.63	11.29	0.39
	WATERCOURSE 3012	41.20	1.12	3.01		3.41	0.016188	2.80	14.71	11.32	0.78
	WATERCOURSE \$ 15	12.20	1.15	3.00		3.02	0.000554	0.58	20.94	13.93	0.15
	WATERCOURSE	16.10	1.15	3.00		3.03	0.000964	0.77	20.96	13.93	0.20
	WATERCOURSE 15	20.20	1.15	3.00		3.05	0.001514	0.96	20.97	13.93	0.25
	MATERCOURSE 24 15	41.20	1.15	3.01		3.21	0.006174	1.95	21.11	13.95	0.51
								·······			
	MATERCOURSE IN A STATE	12.20	1.14	3.00	1.73	3.01	0.000413	0.41	29.75	28.88	0.13
_		16.10	1.14	3.00	1.83	3.01	0.000719	0.54	29.75	28.88	0.17
	MATCH COURSE IN A STATE	20.20	1 14	3.00	1.93	3.02	0.001132	0.68	29.75	28.88	0.21
	MATCHCOURSE AND AND AND	41 20	1 14	3 00	2.33	3.10	0.004709	1.38	29.75	28.88	0.44
	WYAIERUUURDE SHAUKAMAN AN	71.40	1.14	0.00	2.00						

APPENDIX D – RESULTS OF SENSITIVITY TESTING

Patterson Britton & Partners

HEC-RAS Plan: Sens, 1.2x Q River: WATERCOURSE Reach: WATERCOURSE

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	en Reach	River Star	- Qirotal	Mincher:	WS Elev.	offlws,	EG Elév	EG Slope	Vekchal	Flow Area	i Top With .	Froude & Chil
			(m3/s))	(ñ)	(m)	(0)	(m) î	(in/m)	(m/s)	(112)	(iii)	S. Charles
	WATERCOURSE	616 D. A. M.	24.12	7.25	9.55		9.59	0.003334	1.13	31.44	33.59	0.29
_	WATERCOURSE	600	24.12	7.03	9.44		9.52	0.006696	1.57	23.53	30.67	0.39
	WATERCOURSE	585	24.12	7.04	9.27		9.31	0.002931	1.19	41.54	73.33	0.27
	WATERCOURSE	570_	24.12	6.91	9.05		9.20	0.011832	1.99	15.93	18.84	0.55
	WATERCOURSE	555	24.12	6.70	9.04		9.09	0.002959	1.04	31.63	41.94	0.29
	WATERCOURSE	1640	24.12	6.74	9.02		9.05	0.002531	1.07	37.05	41.11	0.26
	WATERCOURSE	625	24.12	6.53	8.86		8.97	0.006175	1.79	23.43	35.87	0.42
•	WATERCOURSE	510	24.12	6.48	8.68		8.84	0.008858	2.09	18.67	27.39	0.50
	WATERCOURSE	495	24.12	6.28	8.57		8.67	0.004935	1.59	21.79	24.13	0.38
_	WATERCOURSE	480	24.12	6.04	8.48		8.58	0.005500	1.67	23.22	30.11	0.39
	WATERCOURSE	465	24.12	5.66	8.42		8.49	0.003669	1.31	23.54	21.49	0.32
	WATERCOURSE	3450	24.12	5.69	8.40		8.43	0.001882	1.03	48.58	74.83	0,23
	WATERCOURSE	1496	24.12	5.66	8.31		8.38	0.005672	1.46	22.50	20.81	0.36
-	WATERCOURSE	3209	24.12	5.05	8.00	7.40	8.19	0.033842	2.09	12.74	20.31	0.79
	WATERCOURSE	405	24.12	5.35	7.63	7.40	7.80	0.007326	1.99	19.34	21.69	0.44
	WATERCOURSE	400	24.12	5.20	0.20	6.00	7.44	0.095661	5.06	4.//	5.36	1.71
	WATERCOURSEL	399	24.12	4.70	5.00	0.10	7.41	0.140762	3.00	4.12	5.20	2.10
	WATERCOURSE	075	24.12	4./1	6.56		1.00	0.009201	2.20	10.80	0.12	0.55
	WATERCOURGE	270	24.12	4.05	6 20		6.62	0.011183	2.30	8 41	6.0	0.01
	WATERCOORSE	- 550	24.12	4.55	6 35		6.54	0.018142	1.89	12 76	7.07	0.79
	WATERCOORDET	245	24.12	3.97	6.00		6.44	0.007942	2 10	11.51	6.25	0.49
	WATERCOOKSE	330	24.12	3.89	6.08		6.32	0.008619	2.16	11 17	6 19	0.43
	WATERCOURSE	315	24.12	3.80	5.98		6,19	0.007342	2.03	11.90	6.91	0.49
	WATERCOURSE	300	24.12	3.71	5.87		6.08	0.007594	2.05	11.75	6.86	0.50
	WATERCOURSE	285 (24)	24.12	3.61	5.75		5.97	0.007789	2.07	11.64	6.86	0.51
_	WATERCOURSE	284	24.12	3.11	5.87		5.92	0.001181	1.00	24.16	11.51	0.22
	WATERCOURSE	270	24.12	3.05	5.81	4.33	5.89	0.002320	1.32	18.29	9.67	0.29
	WATERCOURSE	245 4	Culvert									
	WATERCOURSE	1 222 - 5 6	24.12	2.80	5.38	4.16	5.56	0.006395	1.89	12.78	6.22	0.38
	WATERCOURSE	221,	24.12	2.79	5.40	4,06	5.54	0.002044	1.71	14.09	6.93	0.34
	WATERCOURSE	214 101	Culvert									
	WATERCOURSE	207	24.24	2.70	4.80	3.97	5.03	0.004268	2.14	11.33	7.26	0.47
	WATERCOURSE	1. 150	24.24	1.77	4.27		4.57	0.012571	2.43	9.98	7.07	0.65
_	WATERCOURSE	135	24.24	1.41	4.15		4,39	0.009299	2.17	11.22	7.74	0.54
	WATERCOURSE	120,	24.24	1.56	3.83		4.19	0.014953	2.67	9.11	6.42	0.69
	WATERCOURSE	105	24.24	1.49	3.48		3.81	0.014192	2.54	9.55	6.88	0.69
	WATERCOURSE	90	24.24	0.94	3.38		3.61	0.008588	2.14	11.32	6.25	0.51
	WATERCOURSE	a 75 contration	24.24	1.05	3.36		3,48	0.003819	1.54	15.79	9.22	0.37
	WATERCOURSE	60.4.5	24.24	1.44	3.09		3.34	0.011831	2.23	10.95	10.53	0.68
	WATERCOURSE	45 %	24.24	1.11	3.14		3.21	0.001872	1.24	23.46	21.15	0.29
	WATERCOURSE	30	24.24	1.12	3.00		3.14	0.005671	1.65	14.65	11.29	0.46
	WATERCOURSE	15 12 201	24.24	1.15	3.00		3.07	0.002173	1.15	20.99	13.93	0.30
	WATERCOURSE	, 0 , 1 , 1 ,	24.24	1.14	3.00	2.01	3.03	0.001630	0.81	29,75	28.88	0.26

HEC-RAS Plan: Sens, n+0.02 River: WATERCOURSE Reach: WATERCOURSE

	Reach	River Star	G Total	Minichiel	WS Env	Cilliws.	ELGIELOV	Els slope	Yest Sidin	Flow/Area	Top Wittin	Froude # Chl
			(m3/s)	(m)	(m):	(0)	(ii))	(17)(11)	(iiia)	(62)	(iii)	
	WATERCOURSE	615	20.10	7.25	9.54		9.56	0.004001	0.92	30.90	33.37	0.24
	WATERCOURSER	600	20.10	7.03	9.42		9.48	0.008132	1.30	23.11	30.41	0.33
	WATERCOURSE	585	20.10	7.04	9.27		9.30	0.003358	0.95	41.57	73.36	0.22
	WATERCOURSE	570	20.10	6.91	9.09		9.18	0.012434	1.55	16.76	19.36	0.42
	WATERCOURSE	555	20.10	6.70	9.04		9.07	0.003539	0.85	31.61	41.93	0,24
	WATERCOURSE	540	20.10	6.74	9.01		9.03	0.002915	0.86	36.58	40.87	0.21
	WATERCOURSE	525	20.10	6.53	8.88		8.94	0.006866	1.43	23.95	36.09	0.33
	WATERCOURSEA	510	20.10	6.48	8.71		8.81	0.009617	1.65	19.49	28.36	0.39
	WATERCOURSE	495	20.10	6.28	8.59		8.66	0.005493	1.27	22.37	24.40	0.30
	WATERCOURSE	480 .	20.10	6.04	8.50		8.56	0.006126	1,33	23.77	30.39	0.31
	WATERCOURSE	465	20.10	5.66	8.42		8.47	0.004375	1.07	23.56	21.49	0.26
	WATERCOURSE	450.	20.10	5.69	8.39		8.40	0.002281	0.85	47.24	73.34	0.19
	WATERCOURSE	485	20.10	5.66	8.29		8.34	0.006723	1.19	22.23	20.73	0.29
_	WATERCOURSE	420	20.10	5.65	7.98		8.12	0.042684	1.74	12.39	19.93	0.66
	WATERCOURSE	405	20.10	5.35	7.62	7.27	7.76	0.014902	1.99	15.23	18.92	0.47
	WATERCOURSE	400	20.10	5.26	6.25	6.53	7.22	0.144782	4.37	4.60	5.32	1.50
	WATERCOURSE	399.4	20.10	4.76	6.96		7.10	0.009684	1.67	12.03	6.94	0.40
	WATERCOURSE	390	20.10	4./1	6.86		7.01	0.010426	1.72	11./1	6.87	0.42
	WATERCOURSE	375	20.10	4.63	6.67		6.84	0.012627	1.84	10.92	6.72	0.46
	WATERCOURSE	360 (1.5. SP)	20.10	4.55	6.39		6.61	0.017928	2.09	9.62	6.45	0.55
	WATERCOURSE	359	20.10	4.05	0.46		6.57	0.007101	1.49	13.48	7.21	0.35
	WATERCOURSE	345	20.10	3.97	0.31		0.40	0.009345	1.00	14.14	6.33	0.38
	WATERCOURSE	10 330 Reise Bright	20.10	3.09	6.13		6.15	0.010521	1.75	12.11	6.20	0.41
	WATERCOURSES	31604	20.10	3.00	6.01		6.00	0.010645	1.00	11.62	6.85	0.40
	WATERCOOKSE	9 300 a	20.10	3.71	5.60		5.83	0.012301	1.10	11.02	6 74	0.42
_	WATERCOURSE	403,3102	20.10	3.01	5.00		5.00	0.012001	0.88	22.80	11 27	0.45
	WATERCOURSE	204	20.10	3.05	5.68	4 19	5.75	0.003698	1.16	17.27	9.51	0.26
	WATERCOURSE	046	Culvert			4.10					0.01	0.20
	WATERCOURSE	240 - 240	20 10	2.80	5 32	4.00	5.45	0.009251	1.61	12.49	6.22	0.33
-	WATERCOURSE	222	20.10	2.00	5.33	3.91	5.44	0.003026	1.46	13.73	6.92	0.29
	WATERCOURSE	244 1220	Culvert									
	INATERCOURSE!	207	20.20	2.70	4.93	3.83	5.07	0.004793	1.68	12.00	7.30	0.36
	MATERCOURSE	150 - 0.0	20.20	1.77	4.39		4.56	0.013820	1.87	10.81	7.35	0.49
	MATERCOURSE	135206.1.2	20.20	1.41	4.21		4.37	0.011306	1.74	11.72	8.33	0.43
	WATERCOURSE	1201-111	20.20	1.56	3.91		4.14	0.017335	2.11	9.64	6.59	0.53
	WATERCOURSE	105	20.20	1.49	3.64		3.82	0.014416	· 1.90	10.65	7.20	0.50
	WATERCOURSE	4 90 Sale Sale	20.20	0.94	3.48		3.62	0.010167	1.69	11.93	6.36	0.39
	WATERCOURSE	75	20.20	1.05	3.40		3.48	0.004860	1.25	16.18	9.30	0.30
	WATERCOURSE	60	20.20	1.44	3.19		3.34	0.012079	1.70	12.09	11.42	0.49
	WATERCOURSE	45	20.20	1,11	3.18		3.22	0.002357	1.00	24.20	21.52	0.23
	WATERCOURSE	30	20.20	1.12	3.06		3.15	0.006958	1.33	15.24	11.54	0.37
-	WATERCOURSE	15	20.20	1.15	3.02		3.07	0.002859	0.95	21.23	13.96	0.25
	WATERCOURSE	0	20.20	1.14	3.00	1.92	3.02	0.002219	0.68	29.75	28.88	0.21

END